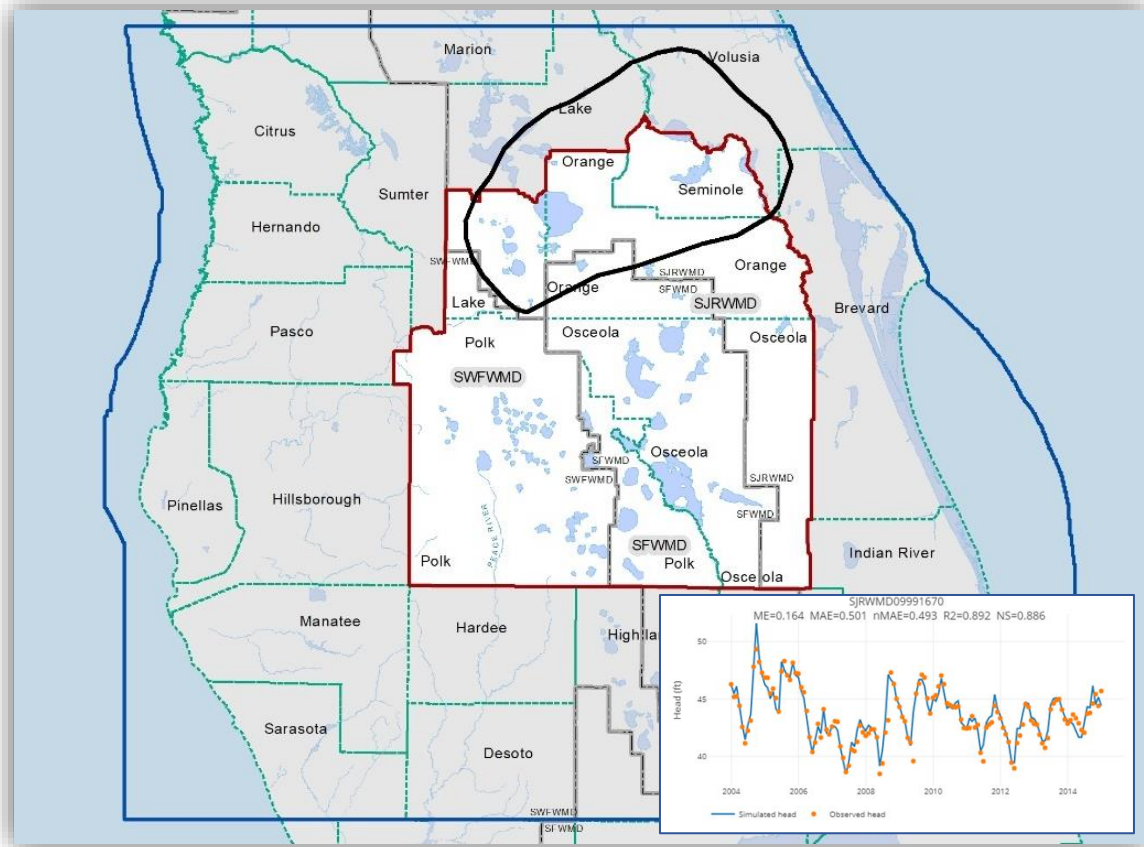


East-Central Florida Transient Expanded (ECFTX) V2.0 Model Report



March 2022

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by

Fatih Gordu, PE
Lanie Sisco
St. Johns River Water Management District

Ron Basso, PG
Hua Zhang, PhD, PG
Jason Patterson, PG
Southwest Florida Water Management District

Peter Kwiatkowski, PG
Anushi Obeysekera
South Florida Water Management District

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ACRONYMS AND ABBREVIATIONS

Aphpz	Avon Park high permeability zone
APPZ	Avon Park permeable zone
APT	aquifer performance test
cfs	cubic feet per second
CFWI	Central Florida Water Initiative
CUP	consumptive use permit
DRN	Drain package
DRT	Drain Return package
DWRM	Districtwide Regulatory Model
dy ⁻¹	ft per day per ft
ECFT	East-Central Florida Transient model
ECFTX	East-Central Florida Transient Expanded Model
EFH	equivalent freshwater head
ET	evapotranspiration
FAS	Floridan aquifer system
ft/d	feet per day
ft ² /d	Feet squared per day
GHB	General Head Boundary package
gpd	gallons per day
HAT	Hydrologic Analysis Team
IAS	intermediate aquifer system
ICU	intermediate confining unit
K	hydraulic conductivity
Kh	horizontal hydraulic conductivity
Kv	vertical hydraulic conductivity
LFA	Lower Floridan aquifer
LFA-upper	First subdivision of the LFA – upper permeable zone
LF-basal	Lower Floridan aquifer – basal permeable zone
MAE	mean absolute error
MCU	middle confining unit
MCU_I	first component of the MCU
MCU_II	second component of the MCU
ME	mean error
mg/l	Milligrams per liter
MGD	million gallons per day
MODFLOW-2005	USGS Modular Groundwater Flow Model 2005

MSR	mean-square residual
NAVD88	North American Vertical Datum of 1988
NGVD29	National Geodetic Vertical Datum of 1929
NS	Nash-Sutcliffe coefficient
OCAPpz	Ocala-Avon Park low-permeability zone
OMR	overall mean residual
PZ	Permeable zone
R ²	coefficient of determination
RMS	root mean square
RMSR	root-mean-square-residual
RWSP	Regional Water Supply Plan
SAS	Surficial aquifer system
SFWMD	South Florida Water Management District
SJRWMD	St. Johns River Water Management District
SWFWMD	Southwest Florida Water Management District
UFA	Upper Floridan aquifer
UFA-upper	Uppermost permeable zone of FAS
UPW	Upstream weighting
USGS	United States Geological Survey
USGS-ECFT model	USGS version of the ECFT model
Districts	water management districts
WUP	water use permit

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CHAPTER 1 – INTRODUCTION

1.1 Background

The East Central Florida Transient Expanded (ECFTX) model was developed in 2020 to support central Florida regional water supply planning and used as a primary tool to estimate groundwater availability and assess water supply and management strategies in the Central Florida Water Initiative (CFWI) planning region (CFWI, 2020). The ECFTX model domain covers about a 23,800-square-mile area of central Florida, including the entire CFWI area and extends from central Volusia County (to the north) to the Charlotte-Desoto County line (to the south) and from the Atlantic Ocean (to the east) to the Gulf of Mexico (to the west) (Figure 1.1). The model has 603 rows and 740 columns with a uniform grid spacing of 1,250 feet (CFWI HAT, 2020).

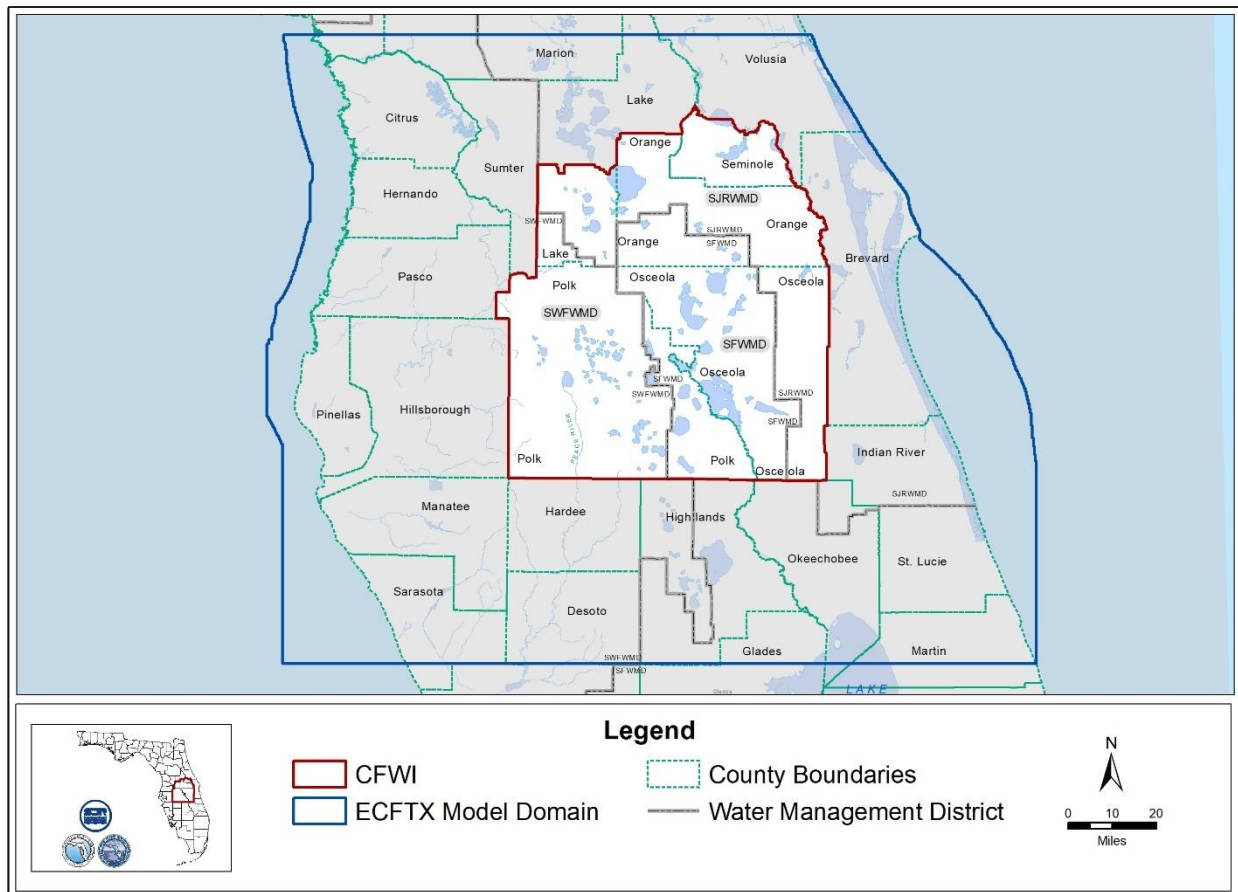


Figure 1.1. ECFTX Model Domain

The ECFTX model is a fully three-dimensional groundwater flow model using the MODFLOW NWT (Niswonger, et al., 2011) computer code. All elevation data for the model are in a vertical datum of NAVD 88. In general, for those areas of the model where chloride concentrations exceed 5,000 mg/l (or 10,000 mg/l total dissolved solids), the layers were inactivated, and general head

boundaries were set along the edge of the active areas (CFWI HAT, 2020). The model consists of 11 hydrostratigraphic layers — layer 1 is the Surficial aquifer (SA), layer 2 is the Intermediate Aquifer System/Confining Unit (IAS/ICU), layers 3–5 are the Upper Floridan aquifer (UFA), layers 6–8 are Middle Confining Unit (MCU I and II), and layers 9–11 are the Lower Floridan aquifer (LFA) (Figure 1.2).

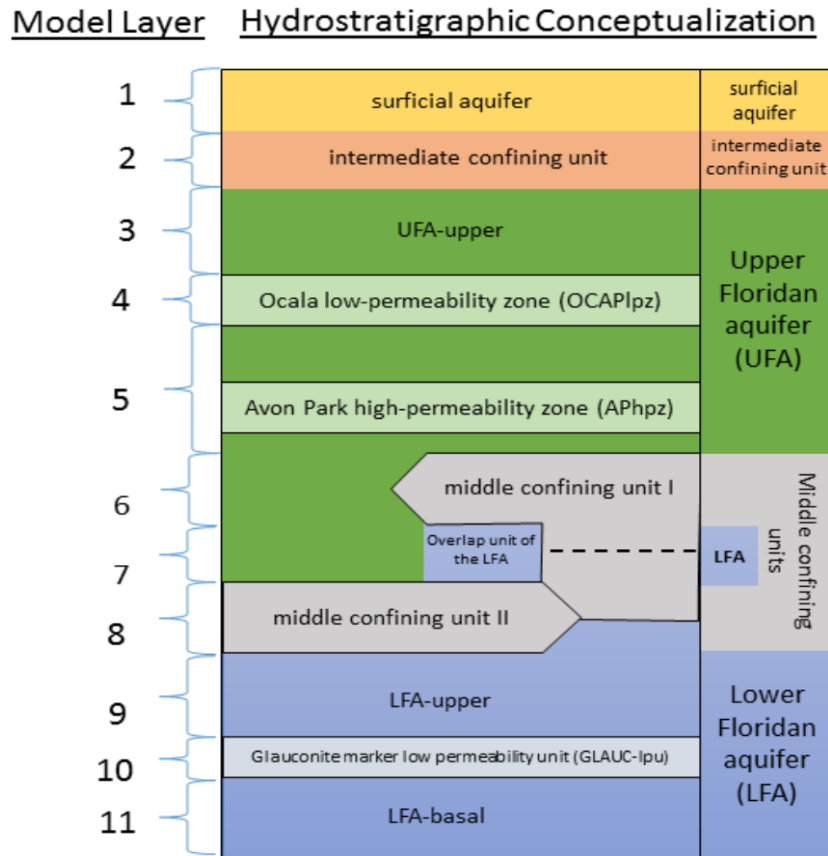


Figure 1.2. ECFTX Model layers (CFWI HAT, 2020).

The ECFTX is a transient model simulating monthly groundwater flows and levels from 2004 through 2014 with an average 2003 steady-state condition serving as the initial conditions. It was calibrated to match observed flows and levels in 2003 (annual averages) and 2004 through 2012 (monthly averages). The years 2013–2014 were used as the verification period.

1.2 Objectives

The main purpose of the ECFTX model was to support water supply planning decisions. To make the model a more suitable tool for regulatory decisions and improve the model performance in the areas where critical minimum flows and levels (MFL) water bodies are located, a groundwater modeling team from three districts (SJRWMD, SWFWMD and SFWMD) reviewed the model and identified an area within the CFWI portion of the domain where the original calibration could be improved. This area primarily included the Wekiva River springs groundwater contributing basin and Seminole County, shown in Figure 1.3 as circled areas.

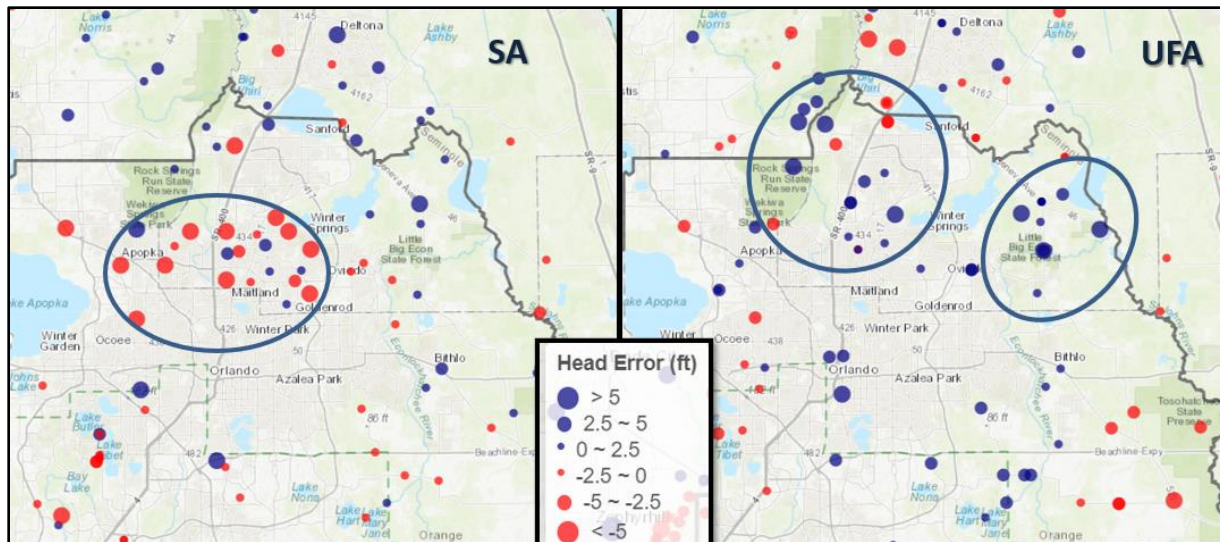


Figure 1.3 Areas of concern

As a result of more thorough review of the local-scale data in these areas, the team also identified opportunities for refinements to the following:

- Spring pool elevations
- Wekiva River stages
- Groundwater level targets near Wekiwa Springs
- Layering of pumping wells

To facilitate the recalibration effort, a local groundwater basin (focus area) was delineated using the Wekiva River groundwater contributing basin and the USGS May/July 2010 UFA potentiometric surface (Figure 1.4). The recalibration effort was conducted only in the focus area with a goal to improve the model's ability to better match observed water levels and spring flows. In addition, the horizontal hydraulic conductivity (K_h) for the IAS/ICU (layer 2) in the Southern Water Use Caution Area (SWUCA) of SWFWMD outside the focus area was modified to improve accuracy of the model conceptualization, model convergence, and run time (Appendix A).

This report describes the model updates, recalibration approach and results, and a sensitivity analysis to better understand the influence of recharge on model calibration. The original and recalibrated models are referred as ECFTX v1.0 and ECFTX v2.0, respectively, in this document.

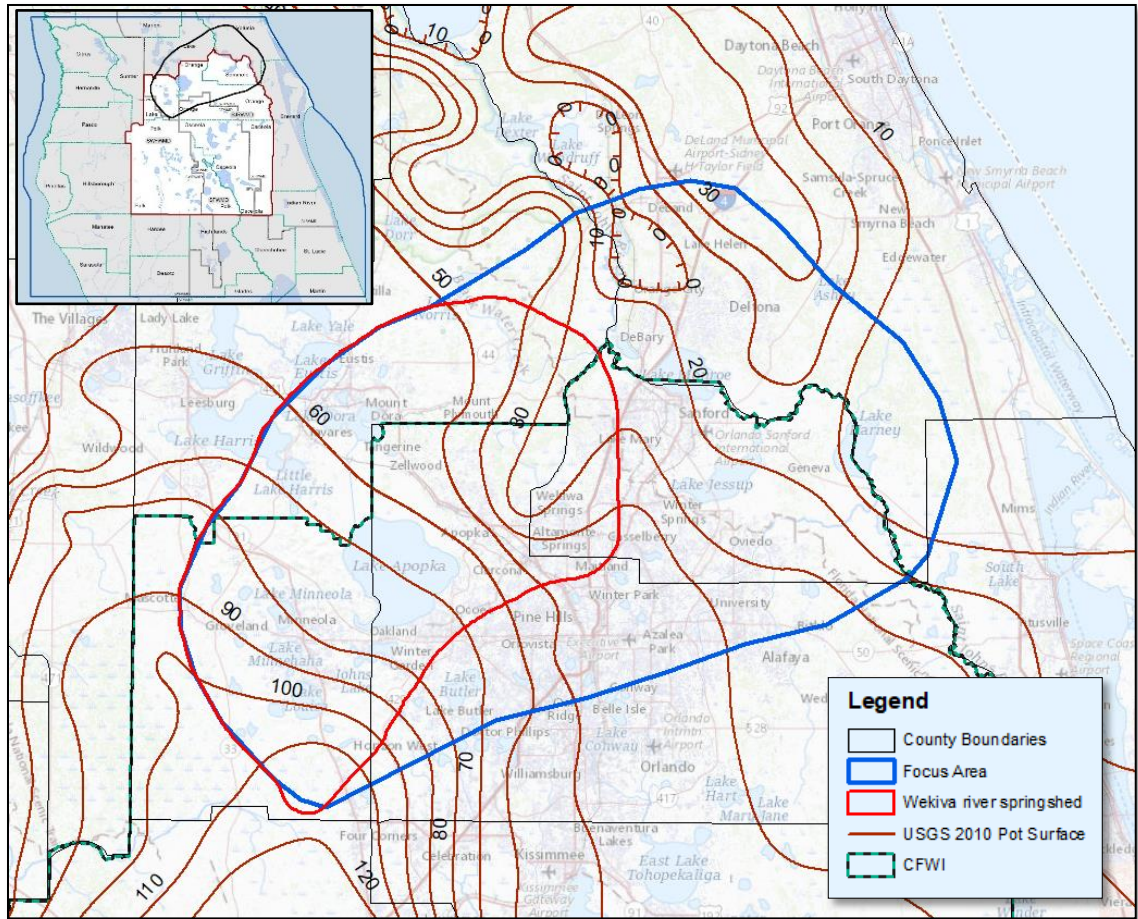


Figure 1.4 Focus Area

CHAPTER 2 – MODEL UPDATES

ECFTX model updates, generally limited to the focus area as shown in Figure 1.4, included modifications to spring pool elevations, river stages, and groundwater level targets. Minor adjustments were also made to recharge, maximum saturated evapotranspiration (MSET), and groundwater withdrawals. Changes to the model outside of the focus area included updates to layer 2 horizontal hydraulic conductivity (Kh) in the region where the Intermediate aquifer system (IAS) is present.

2.1 Spring pool elevations

A review of the springs in the model indicated the pool elevations assigned to several springs within or near the focus area needed to be modified due to an older vertical elevation datum, feet NGVD29, previously used. Therefore, pool elevations were updated to reflect the appropriate vertical datum, feet NAVD88, for the model simulation period of 2003 to 2014. Figure 2.1 shows the locations of the 12 springs where the pool elevations were revised in the model and Table 2.1 includes the change in updated 2003 to 2014 average pool elevation at each spring. Of the springs with revised pool elevations, Alexander Spring was the only spring categorized as a magnitude 1 spring (with a discharge at or greater than 100 cfs). The remaining revised springs were of magnitudes 2 or 3 and were mostly within the Wekiva River springshed, apart from Gemini Springs and Green Spring, which were outside of the springshed but within the focus area (Figure 2.1).

Table 2.1. The revised springs

Name	ID	Magnitude	ECFTX V1.0	ECFTX V2.0
			2003 to 2014 Average Pool Elevation (Feet, NAVD88)	2003 to 2014 Average Pool Elevation (Feet, NAVD88)
Sanlando Springs	1736	2	26.20	25.59
Palm Springs	1456	3	22.47	21.43
Starbuck Spring	1916	2	21.02	19.97
Miami Spring	4109	3	15.21	14.20
Wekiwa Spring	2353	2	13.29	12.17
Holiday Spring	841	3	65.03	67.03
Bugg Spring	256	2	60.51	61.67
Rock Springs	1624	2	26.26	25.16
Green Spring	730	3	11.34	10.40
Gemini Springs #2	4163	2	2.61	1.68
Gemini Springs #1	4161	2	2.61	1.23
Alexander Spring	16	1	2.95	9.41

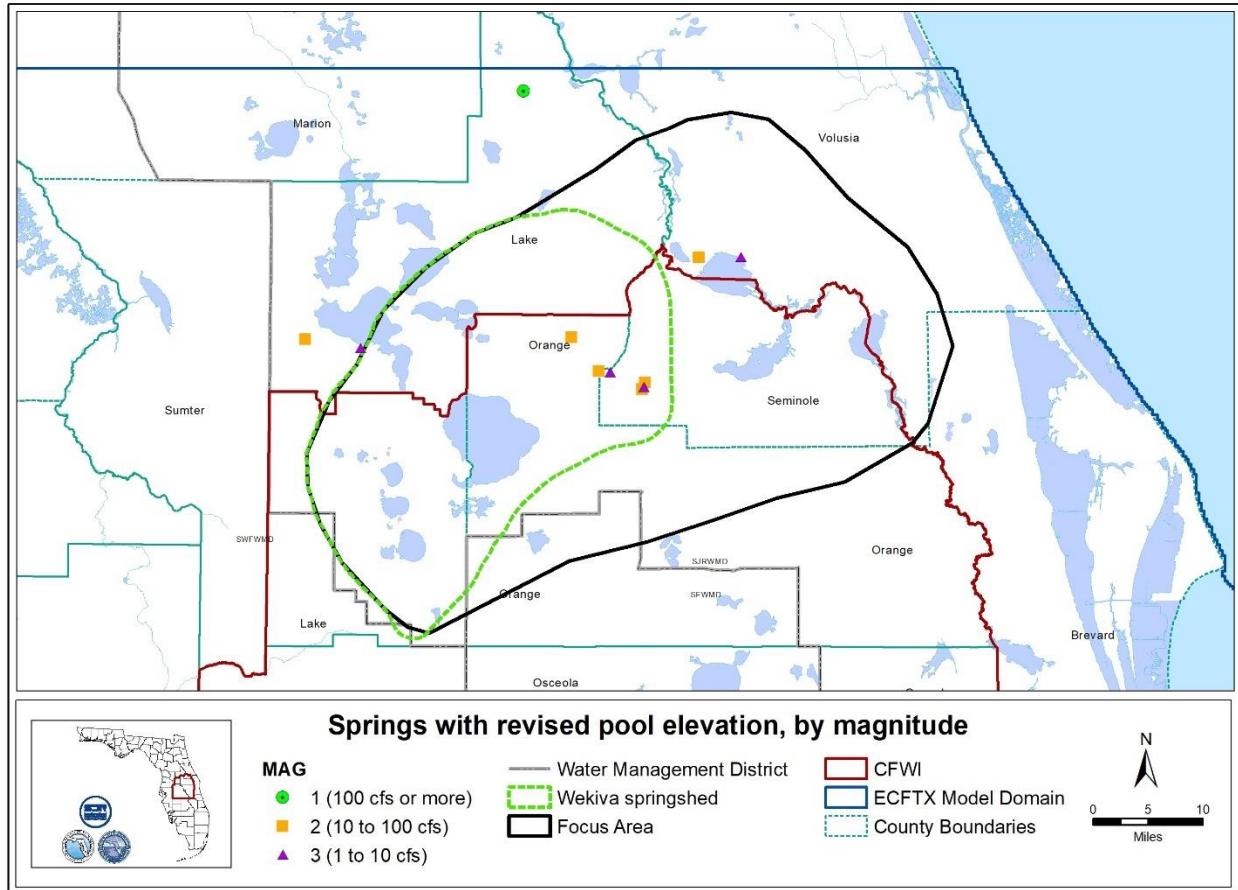


Figure 2.1. Spring locations with revised pool elevations

2.2 Wekiva River stages

A comparison of modeled Wekiva River stages with the observed data, where available, indicated the modeled stages needed to be modified at several locations in the river, likely due to data error or the use of an old vertical elevation datum. The simulated stages from the Wekiva River HEC-RAS model (SJRWMD, 2019) were used to update river stages at cell locations representing the Wekiva River in the model for the period of 2003 through 2014. Linear interpolation was used to estimate the stages in between locations where HEC-RAS model data were available. At river boundary cells where the stages were adjusted, the river bottom elevations were also adjusted to maintain the depth assigned to each cell in ECFTX V1.0. Figure 2.2 shows the river boundary cells representing the portion of the Wekiva River where stage was adjusted.

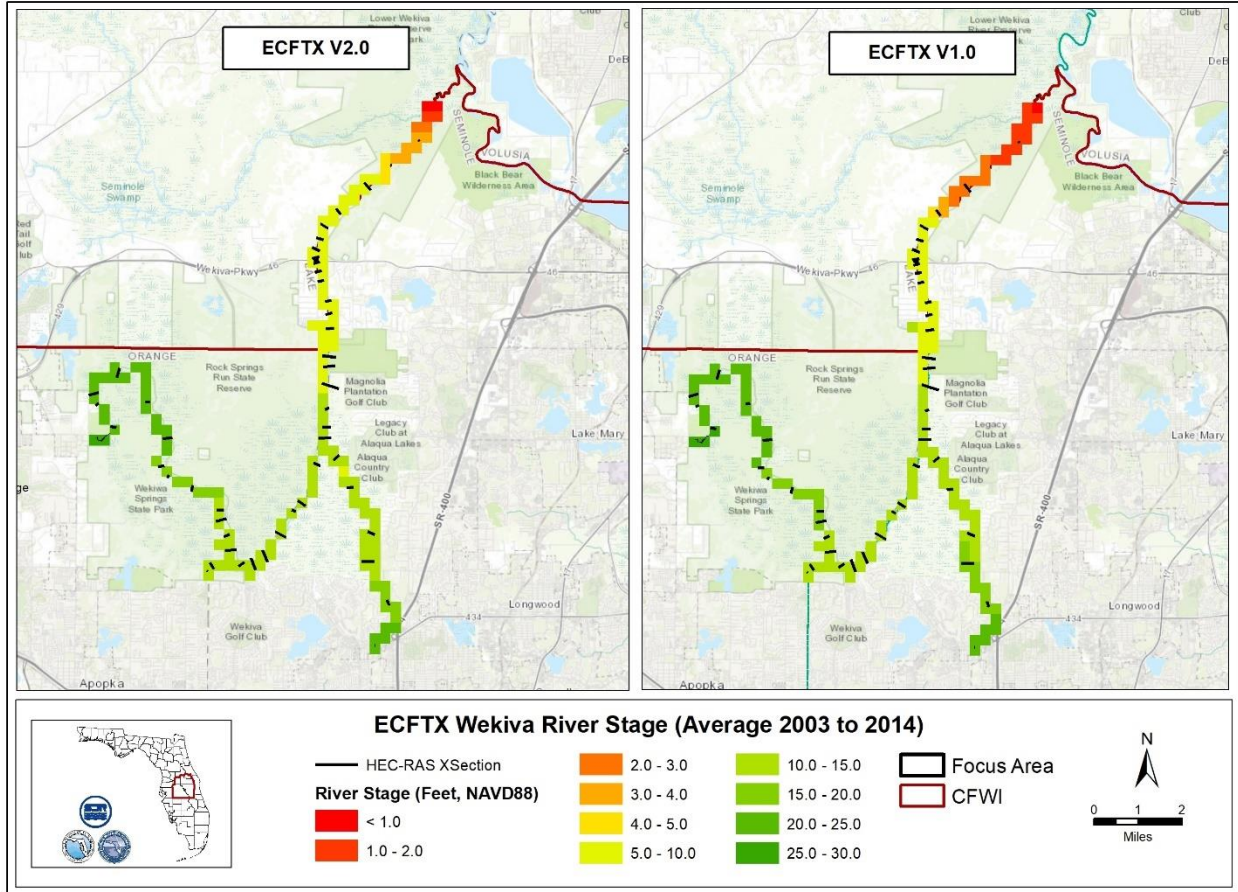


Figure 2.2. Wekiva River cell locations where the river stage and bottom elevation were updated in the model. The 2003 to 2014 revised stage in ECFTX V2.0 is shown on the left while the stage in ECFTX V1.0 is shown on the right for comparison.

2.3 Groundwater level targets

Groundwater level targets in the focus area were reviewed for location accuracy and observation values. Monitoring wells OR0547 and OR0548 are near Wekiwa Springs (Figure 2.3). OR0548 is open to the shallow part of the UFA (model layer 3) and OR0547 is open to MCU_1 (model layer 6). The vertical head difference between these wells exceeds 20 feet. Our review of the head observation (HOB) package indicated OR0548 was not utilized in the ECFTX v1.0 model calibration and OR0547 well was assigned to the wrong model layer, layer 5. The HOB package was modified by assigning OR0547 well to layer 6 and OR0548 well to layer 3. Additionally, the model grid cell assigned to surficial aquifer monitoring well OR0894 near Prevatt Lake (Figure 2.3) was shifted 1 grid cell east (row 118 and column 382) to avoid the OR0894 grid cell intersecting with a river boundary cell in the model (row 118 and column 381). This prevented the simulated groundwater levels at the OR0894 grid cell from being strongly influenced by the specified stage in the river cell.

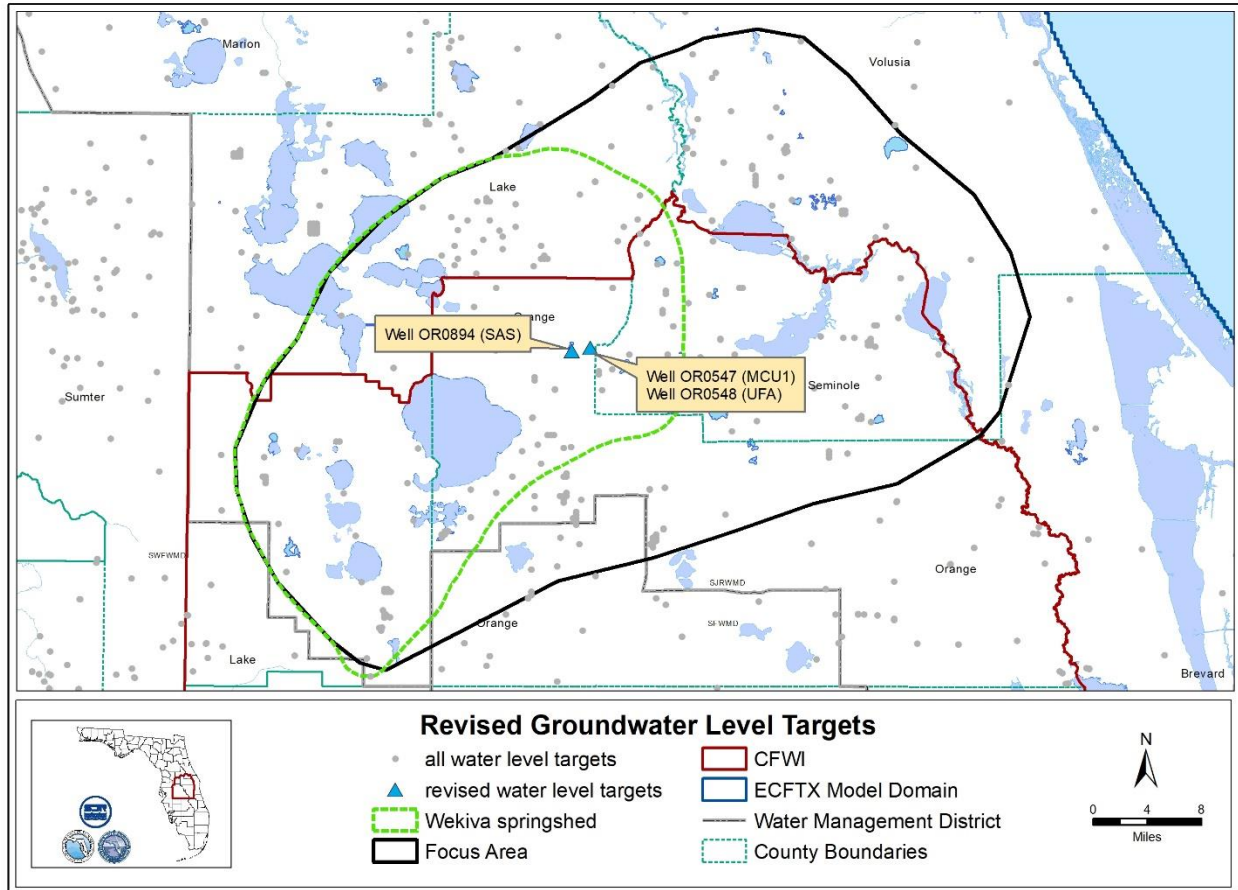


Figure 2.3. Locations of revised groundwater level calibration targets in the model.

2.4 Groundwater withdrawals

A review of groundwater withdrawals in the ECFTX model revealed several consumptive use permits (CUPs) in SJRWMD required updated layer assignments due to inaccurate well construction information used to assign the model layer in the MODFLOW well package. Thus, the model layers in the well package were adjusted based on the updated well construction data for the following CUPs: City of Casselberry (CUP 8284), City of Altamonte Springs (CUP 8372) and the City of Eustis (CUP 84879). Review of well construction information at well SJ_8284_15422 indicated that this well was constructed deeper than originally modelled and was open to layers 5 through 9. Therefore, fluxes previously assigned to layers 3 and 4 at this well in ECFTX v1.0 were redistributed to layers 5-9. Review of well construction information at wells SJ_8284_15427, SJ_8284_15428, SJ_8372_15672, SJ_8372_19978 and SJ_8372_19979 indicated that fluxes previously assigned to layers 3-5 in ECFTX v1.0 needed to be redistributed to layers 7-9. Review of well construction information at SJ_84879_34862 indicated that this well is open to the basal UFA (layer 5) and not the LFA as modeled in ECFTX v1.0; therefore, the model layer was adjusted.

2.5 Recharge and Maximum Saturated ET rates

Recharge and maximum saturated ET rates were set to zero at the grid cells representing the Wekiva River in layer 1 of the model, shown in Figure 2.4, because the specified river stages in

the model already accounted for the influences of actual recharge and ET on water levels. The Wekiva River Basin was the primary focus for recalibration; therefore, this change was only applied at the grid cells representing the Wekiva River. Recharge and ET in the model domain outside of the Wekiva River remained at rates assigned to ECFTX V1.0 (CFWI HAT, 2020). It should also be noted that this adjustment was made for more accurately calculating baseflows within the Wekiva River Basin and had little or no effect on model results since river stages were specified in the model.

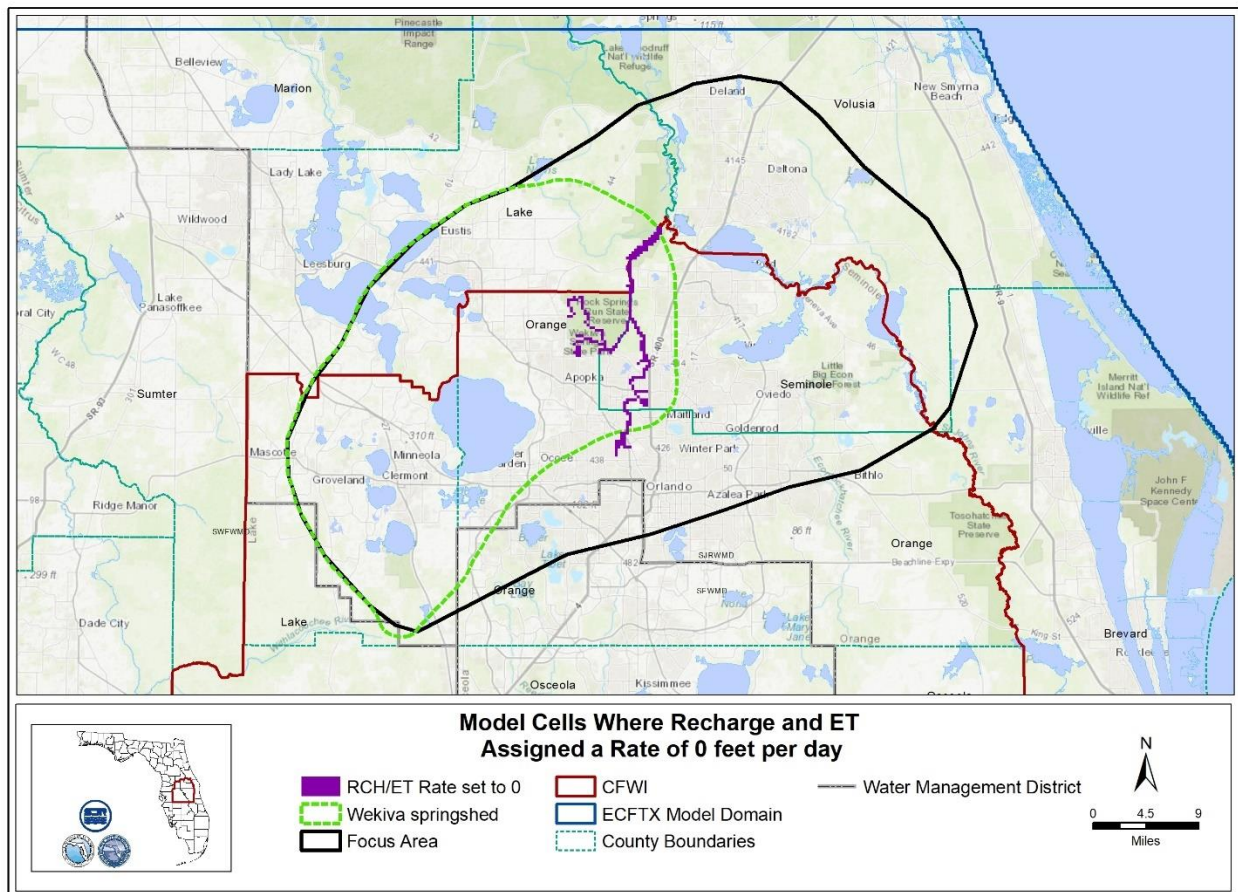


Figure 2.4. Locations where the recharge and ET rate in the model were assigned a value of 0 feet per day for all stress periods.

2.6 Outside Focus Area updates

The Hawthorn aquifer system (HAS) (also more generally referred to as the intermediate aquifer system) exists within an approximate 5,000-square-mile area of DeSoto, Sarasota, Hardee, Manatee, and parts of Charlotte, Hillsborough, Highlands counties, and in the southwest portion of Polk County within the CFWI region. Two main water-producing aquifers exist within the HAS: the Upper Arcadia aquifer and Lower Arcadia aquifer. The ECFTX v1.0 treated the HAS as a confining unit and did not simulate the individual aquifers within the HAS. As a result, assignment of low hydraulic conductivity values to the areas where the HAS existed has resulted in convergence issues in the model due to the presence of pumping wells in these aquifers. Although the convergence issue has not affected the results significantly (see Appendix A), it has considerably increased model run time. Therefore, as part of this update, the horizontal hydraulic conductivity (Kh) for layer 2 was increased so that the model becomes more conceptually accurate by simulating horizontal flow within the intermediate aquifer system and consequently improving model convergence and run time. Vertical hydraulic conductivity values were unchanged in layer 2 and overall vertical leakage from the surficial aquifer to the UFA through layer 2 largely remained the same as ECFTX v.1.0. Appendix A presents details of this update.

CHAPTER 3 – MODEL RECALIBRATION

3.1 Calibration Approach

After the model was updated, model parameters were recalibrated to improve the model performance within the focus area. No changes were made to the parameters outside the focus area as part of recalibration. Because the focus area was identified as a local groundwater basin with limited flow exchange across the lateral boundary, the modifications to the model parameters within the focus area were assumed to minimally affect the groundwater levels and flows outside the focus area. Model testing after recalibration generally confirmed this understanding.

As recommended by peer reviewers of ECFTX 1.0 (Andersen, et al. 2020), the automated parameter estimation tool (PEST) by Dougherty (2014) was used for recalibration. The following parameters were adjusted using PEST:

- Hydraulic conductivities of all layers
- Spring conductances
- River conductances along Wekiva River

Initial parameter values were obtained from the ECFTX v1.0 model. The upper and lower bounds of the pilot points, utilized for adjustment of hydraulic conductivities, were set so that the UFA and LFA transmissivities and ICU and MCU leakance values were maintained within the values consistent with the known hydrogeology of the area. All observations utilized for PEST calibration were located within the focus area. The following observations were employed for PEST calibration:

- Groundwater levels for all layers
- Groundwater level differences between UFA and SA and between LFA and UFA
- Spring flows
- Baseflows within Wekiva River Basin

The model was recalibrated using a four-step approach as follows:

1. PEST optimization was first conducted on a steady-state model representing the average 2003-2014 condition.
2. Once a steady-state calibration was satisfactory, a transient model was run with the updated parameters.
3. Storage coefficients were adjusted as needed.
4. Steps 1 through 3 were repeated until a satisfactory transient calibration was achieved.

The calibration criteria set for ECFTX v1.0 were used to evaluate model performance. The recalibration was performed to ensure that the modeled groundwater levels and spring flows matched observed ones closely within the focus area and the modeled baseflows were within the range of estimated baseflows at critical gages in Wekiva River basin. The remaining simulated baseflows within the focus area were reviewed qualitatively to ensure that the ECFTX v2.0 performed in a similar manner as ECFTX v1.0. Simulated groundwater level contours were

compared with potentiometric surface maps to further assess the model's ability to adequately match the configuration of the UFA flow field and groundwater flow direction. Aquifer performance test (APT) and literature data were utilized qualitatively to evaluate the reasonableness of the aquifer parameters.

In addition to PEST calibration, the GHB conductances were manually adjusted along the eastern boundary to ensure the fluxes across that boundary were similar to the ECFTX v1.0 fluxes. No recharge adjustment was made. However, a recharge sensitivity analysis, described in detail later in this report, was performed to better understand the effect of recharge on model parameterization. Upon review and testing, storage coefficients were left unchanged from the original ECFTX v1.0 values. After the recalibration was finalized, model-wide calibration statistics were also reviewed to ensure there was no degradation in model performance outside the focus area.

3.2 Transient Calibration Model Results

Monthly average aquifer water levels, springflow and baseflow estimates, developed for the calibration of ECFTX V1.0, were utilized in this recalibration effort as calibration targets to assess model calibration metrics. These included average monthly water levels from observation wells from the SA (layer 1), UFA (layers 3–5), and the LFA (layers 9–11). Additional information regarding the observation data utilized to calibrate the model can be found in Chapter 5 of the ECFTX V1.0 documentation (CFWI HAT, 2020). In addition, vertical head differences (VHDs) between the SA and the UFA and between the UFA and the LFA were introduced as new quantitative targets.

The ECFTX v1.0 calibration criteria were also used in this recalibration effort and included: 1) a mean error for SA, UFA, and LFA aquifer heads from all wells of less than one foot, 2) a root mean squared error of less than 5 feet from all wells within each aquifer, and a mean absolute error within 5% of the total head elevation range for each aquifer. Total modeled spring flows had to be within 10% of the estimated/measured flows. Mean simulated discharge at each magnitude 1 and 2 spring with observed records also individually had to be within 10% of the observed flows. Specific to the CFWI area, 50% of the mean absolute simulated head residuals for all wells in the SA, UFA, and LFA had to be within 2.5 feet of observed and 80% of the mean absolute simulated head residuals for all wells in the SA, UFA, and LFA were required to be within 5 feet of observed values (CFWI HAT, 2020).

Model statistics for observation wells are presented in this section for three geographic areas: 1) ECFTX model domain, 2) CFWI area and 3) focus area, shown in Figures 1.1 and 1.4. For assessing improvement in model prediction performance, model statistics for all calibration target groups were compared to ECFTX v1.0.

3.2.1 Groundwater levels

Transient model calibration statistics were computed for the target wells in the SA, UFA and LFA within the focus area (Table 3.1), CFWI area (Table 3.2) and ECFTX model domain (Table 3.3). The spatial distributions of mean error, expressed as the simulated minus observed water level, for the target wells for the SA, UFA, and LFA in the focus area are shown in Figures 3.1 through

3.3. The calibration period mean simulated versus observed water levels for the SA, UFA, and LFA targets in the focus area are compared between version 1.0 and 2.0 of the models in Figures 3.4 through 3.6. Figures 3.8 through 3.13 show individual simulated versus observed water level hydrographs at selected wells within the focus area (Figure 3.7). Figure 3.14 shows the location of 50 vertical head difference targets located in the focus area that were used to calibrate the model. The calibration period mean simulated versus observed vertical head differences in the recalibration focus area are compared between version 1.0 and 2.0 of the models in Figure 3.15. The 2003 to 2014 average flooded depth in layer 1 is compared in Figure 3.16. Appendix B includes graphs of simulated versus observed water levels for the calibration period at each target well grouped by major aquifer within the focus area.

Table 3.1. Transient model calibration period statistics of the target monitoring wells in the focus area.

	Focus Area – V1.0			Focus Area – V2.0		
	SA	UFA	LFA	SA	UFA	LFA
Residual Mean	-0.72	0.42	1.96	-0.10	-0.25	-0.02
Error Standard Dev	5.04	4.44	1.51	2.19	1.85	0.42
5% Observation Range	6.86	5.73	2.5	6.86	5.73	2.50
Absolute Residual Mean	3.68	3.39	2.11	1.90	1.60	0.82
Error sum of squares	1789	1670	65	331	293	2
RMS Error	5.05	4.43	2.43	2.17	1.86	0.4
Minimum Residual	-13.12	-9.63	-0.12	-7.71	-8.00	-1.1
Maximum Residual	18.47	22.07	4.54	9.37	5.11	0.47
# Observations	70	85	11	70	85	11
% MAE < 2.5 ft	57%	48%	64%	87%	89%	100%
% MAE < 5.0 ft	74%	82%	100%	94%	96%	100%
R ² > 0.4	81%	94%	100%	84%	94%	100%

All values in feet except as noted. Calibration period is 2004 to 2012. Mean error is expressed as simulated minus observed.

Table 3.2. Transient model calibration period statistics of the target monitoring wells in the CFWI area.

	CFWI Area – V1.0			CFWI Area – V2.0		
	SA	UFA	LFA	SA	UFA	LFA
Residual Mean	-0.64	0.34	1.23	-0.42	-0.06	-0.16
Error Standard Dev	3.47	3.75	2.68	2.86	3.18	2.46
5% Observation Range	8.6	6.2	2.62	8.60	6.20	2.62
Absolute Residual Mean	2.61	3.24	2.48	2.23	2.64	1.80
Error sum of squares	3442	2729	202	2299	1956	140
RMS Error	3.53	3.75	2.9	2.88	3.18	2.42
Minimum Residual	-16.51	-11.93	-5.46	-16.52	-11.98	-7.16
Maximum Residual	13.29	10.11	5.73	13.28	10.03	4.82
# Observations	277	194	24	277	194	24
% MAE < 2.5 ft	71%	52%	58%	77%	67%	83%
% MAE < 5.0 ft	87%	85%	88%	92%	89%	92%
R ² > 0.4	78%	96%	92%	79%	96%	92%

All values in feet except as noted. Calibration period is 2004 to 2012. Mean error is expressed as simulated minus observed.

Table 3.3. Transient model calibration period statistics of the target monitoring wells in the ECFTX model domain.

	ECFTX Model Domain – V1.0			ECFTX Model Domain – V2.0		
	SA	UFA	LFA	SA	UFA	LFA
Residual Mean	-0.46	0.46	0.46	-0.43	0.39	-0.65
Error Standard Dev	4.24	4.7	3.33	4.09	4.58	2.96
5% Observation Range	8.97	7.59	2.79	8.97	7.59	2.79
Absolute Residual Mean	2.83	3.78	2.65	2.72	3.62	2.11
Error sum of squares	18156	20666	329	16878	19599	266
RMS Error	4.27	4.72	3.31	4.11	4.60	2.98
Minimum Residual	-31.65	-22.1	-10.19	-31.69	-22.10	-10.15
Maximum Residual	21.15	19.14	5.73	21.15	19.14	4.82
# Observations	997	928	30	997	928	30
% MAE < 2.5 ft	68%	48%	60%	70%	53%	80%
% MAE < 5.0 ft	88%	76%	87%	89%	77%	90%
R ² > 0.4	78%	93%	93%	78%	93%	93%

All values in feet except as noted. Calibration period is 2004 to 2012. Mean error is expressed as simulated minus observed.

3.2.2 Spring flow

Simulation period model statistics for 17 magnitude 1 and 2 springs are included in Table 3.4. The observed and simulated flux in Table 3.4 is computed as the average for the 2003 to 2014 simulation period. The spatial distribution of the mean error for the 17 simulated springs is shown

in Figure 3.17 and a regression plot of simulated versus observed mean spring flow is shown in Figure 3.18. Simulated versus observed monthly flow hydrographs for Wekiwa and Rock springs are shown in Figure 3.19. For the calibration period from 2004 to 2012, mean simulated springflow in the model from all 158 springs was 2,104 cfs, while observed (estimated and measured) was 2,159 cfs, resulting in a mean error of 2.5%. Appendix C contains graphs of simulated versus observed springflow where it was continuously measured.

Table 3.4. Model simulation period statistics of the magnitude 1 and 2 target springs simulated in the ECFTX model.

Spring Name	Observed Flux (cfs)	ECFTX V1.0		ECFTX V2.0	
		Simulated flux (cfs)	% error	Simulated flux (cfs)	% error
Lithia Spring Major	34.7	33.2	-4.4%	33.1	-4.5%
Buckhorn Main Spring	12.2	12.1	-0.9%	12.1	-1.0%
Sulphur Spring	34.7	35.4	2.0%	35.4	2.0%
Crystal Main Spring	45.5	46.4	2.0%	46.3	1.9%
Weeki Wachee Spring	160.4	167.3	4.4%	167.3	4.4%
Chassahowitzka Spring	59.6	59.3	-0.6%	59.3	-0.6%
Homosassa Spring	83.5	84.5	1.1%	84.5	1.2%
Gum Spring	63.8	64.8	1.5%	64.8	1.5%
Rainbow Spring*	71.3*	73.3	2.0%	73.3	2.0%
Apopka Spring	24.9	24.8	-0.1%	24.9	0.0%
Sanlando Springs	18.8	19.9	5.4%	18.9	0.1%
Starbuck Spring	12.1	12.6	3.9%	12.0	-0.5%
Wekiwa Spring	61.0	64.6	5.8%	61.3	0.4%
Bugg Spring	10.6	9.7	-8.2%	10.0	-5.2%
Rock Springs	54.9	51.6	-6.1%	54.7	-0.5%
Volusia Blue Spring	143.6	132.4	-7.9%	144.3	0.5%
Alexander Spring	100.1	98.9	-1.2%	99.8	-0.3%

*Observed flow reduced by 88% since only 12% of rainbow springshed included in active domain.

3.2.3 Baseflow

Calibration criterion for simulated baseflows was within an order of magnitude due to the variability of estimation methods for this more uncertain flow statistic. Information about the baseflow estimation methods used for the ECFTX model can be found in Chapter 4 of the ECFTX V1.0 report (CFWI HAT, 2020). A total of 18 USGS gages where baseflows were estimated are included in Table 3.5, alongside the minimum and maximum estimated baseflows. Gauge 02238000, Haynes Creek at Lisbon, was removed from the list because its flows have been regulated with multiple control structures. Wekiva River at Sanford gauge 02235000 was added to the list because it was one of the gauges used to set minimum flows and levels at Wekiva River. For the calibration period, from 2004 to 2012, mean simulated baseflow in the model from all 18 USGS gauges was 5,557 cfs, while the range of estimated flows varied between 2,391 and

9,998 cfs. A total of 15 out of 18 USGS gauges were within the range of estimated baseflows by baseflow separation methods (Figure 3.20).

Table 3.5. Simulated mean baseflow from 2004 to 2012 compared to estimated ranges using the USF method and USGS Groundwater Toolbox methods at 18 stations.

Gage	Station	Min (cfs)	Max (cfs)	Simulated (cfs)	
				V1.0	V2.0
02232400	St. Johns River nr Cocoa FL	221	928	293	293
02232500	St. Johns River nr Christmas FL	282	1085	384	383
02234000	St. Johns River above Lake Harney nr Geneva FL	441	1473	856	860
02236000	St. Johns River Near DeLand FL	389	3186	1768	1946
02235000	Wekiva River nr Sanford	184	254	278	247
02294650	Peace River at Bartow	21	125	77	78
02294898	Peace River at Fort Meade	19	144	129	129
02295637	Peace River at Zolfo Springs	80	350	331	334
02296750	Peace River at Arcadia	118	596	533	537
02298830	Myakka River nr Sarasota	14	150	50	50
02300500	Little Manatee River nr Wimauma	23	88	70	70
02301500	Alafia River at Lithia	53	189	110	111
02303000	Hillsborough River nr Zephyrhills	65	145	102	102
02310000	Anclote River nr Elfers	4	38	11	11
02312000	Withlacoochee River at Trilby	28	153	165	164
02312500	Withlacoochee River at Croom	56	211	190	195
02312762	Withlacoochee River nr Inverness	156	378	36	46
02313000	Withlacoochee River nr Holder	237	505	-8	1

3.2.4 Water budget

The simulated water budget including boundary condition inflows (Table 3.6) and outflows (Table 3.7) from the ECFTX v2.0 model by layer were prepared for the calibration period. The total flux (IN-OUT) balances to within less than 0.05 inches/year. Net fluxes for each major component of the water budget during the calibration period included recharge of 8.7 in/yr, GHB lateral flux of 0.9 in/yr into the model with constant head, well, river, springs, and drains having net outflow components of 2.3, 2.0, 0.8, 1.3, and 3.5 inches per year, respectively. A total net storage change of +0.4 in/yr occurred over the 2004–2012 period based on the model results. Net fluxes for each major component in ECFTX v1.0 included recharge of 8.7 in/yr, GHB lateral flux of 0.7 in/yr into the model with constant head, well, river, springs, and drains having net outflow components of 2.3, 2.0, 0.9, 1.3, and 3.4 inches per year, respectively. A total net storage change of +0.4 in/yr occurred over the 2004–2012 period based on the model results, which is unchanged from v2.0.

Table 3.3.6. Annual average boundary condition influx in the ECFTX transient model during the calibration period (2004-2012). Note units are in inches per year.

Layer	Constant Head in/yr	GHB in/yr	Well in/yr	River in/yr	Recharge in/yr	ET in/yr	Spring in/yr	Drain return in/yr	Drain in/yr	Storage in/yr
1	0.22	0.04	0.08	3.23	21.83	-	-	-	-	0.38

Layer	Constant Head in/yr	GHB in/yr	Well in/yr	River in/yr	Recharge in/yr	ET in/yr	Spring in/yr	Drain return in/yr	Drain in/yr	Storage in/yr
2	-	0.26	-	-	-	-	-	-	-	0.01
3	-	0.48	-	-	-	-	-	0.07	-	-
4	-	0.23	-	-	-	-	-	-	-	-
5	-	0.57	-	-	-	-	-	-	-	-
6	-	0.10	-	-	-	-	-	-	-	-
7	-	0.08	-	-	-	-	-	-	-	-
8	-	0.06	-	-	-	-	-	-	-	-
9	-	0.21	-	-	-	-	-	-	-	-
10	-	-	-	-	-	-	-	-	-	-
11	-	0.16	-	-	-	-	-	-	-	-
Total	0.22	2.19	0.08	3.23	21.83	0.00	-	0.07	-	0.39

Table 3.3.7. Annual average boundary condition outflux in the ECFTX transient model during the calibration period (2004-2012). Note: units are in inches per year.

Layer	Constant Head in/yr	GHB in/yr	Well in/yr	River in/yr	Recharge in/yr	ET in/yr	Spring in/yr	Drain return in/yr	Drain in/yr	Storage in/yr
1	-2.56	-0.02	-0.17	-4.03	-	-13.13	-	-	-3.61	-
2	-	-0.03	-0.02	-	-	-	-	-	-	-
3	-	-0.45	-0.64	-	-	-	-1.29	-	-	-
4	-	-0.11	-0.30	-	-	-	-	-	-	-
5	-	-0.06	-0.59	-	-	-	-0.01	-	-	-
6	-	-	-	-	-	-	-	-	-	-
7	-	-	-	-	-	-	-	-	-	-
8	-	-	-	-	-	-	-	-	-	-
9	-	-0.12	-0.31	-	-	-	-	-	-	-
10	-	-	-	-	-	-	-	-	-	-
11	-	-0.51	-	-	-	-	-	-	-	-
Total	-2.56	-1.30	-2.03	-4.03	-	-13.13	-1.30	-	-3.61	-

3.2.5 Aquifer and Confining Unit Properties

Hydraulic properties within the ECFTX model include hydraulic conductivity (both vertical and horizontal) and specific storage properties. During the calibration process, the initial estimates of hydraulic conductivity were adjusted within reasonable limits to improve the agreement between simulated and observed conditions while maintaining parameterization consistent with the conceptual model of the system. After testing different values of storage properties, the V1.0 model assigned values were left unchanged. The horizontal hydraulic conductivity distribution in model layer 1 is shown in Figure 3.21 and hydraulic conductivity maps for all model layers are included in Appendix D. Transmissivity, the product of the aquifer K and the saturated thickness expressed in feet squared per day (ft²/d), was computed for the UFA (layer 3 through 5) and the LFA (layer 9 through 11). UFA transmissivity is compared with APT results (Figure 3.22) and the September 2010 UFA potentiometric surface map (Figure 3.23). The potentiometric surface maps provide evidence of higher or lower transmissivity based on whether the gradient is flat or steep and was used as a qualitative guide to calibration. LFA transmissivity is compared with APT

results in Figure 3.24. The leakance coefficient, computed as the vertical hydraulic conductivity divided by the confining unit thickness and expressed in units of ft/d/ft (d^{-1}) was computed for layers 2, 6, 8 and layer 10 (Appendix D).

3.3 Recharge Sensitivity Analysis

Recharge was not adjusted during the recalibration of the model because it was originally estimated using a water-balance model and generally seemed to work well for ECFTX v1.0. The results presented above show that the model was able to be successfully recalibrated by adjusting aquifer parameters without modifying the recharge in the model. However, recharge, being a large water budget component, is one of the largest sources of uncertainty in the model due to lack of measured data. Therefore, a full PEST recalibration was also performed with a recharge rate reduction of 20% within the focus area to test the sensitivity of the model performance to recharge.

3.3.2 Methods

As described in Chapter 3.1 Calibration Approach, an initial steady-state model was developed for automated calibration representing the average (2003 to 2014) ECFTX V1.0 model. Within the initial model, recharge rates were not adjusted from ECFTX v1.0, apart from not assigning a value (0 feet per day) to grid cells representing the Wekiva River (Figure 2.4). As a sensitivity test, another model was developed in which the initial model recharge rates were reduced by 20% within the focus area. Except for recharge rates, both models were identical and were assigned the same initial parameter values and bounds in PEST. For each model, PEST was independently run. The same calibration approach described in section 3.1 was implemented.

3.3.3 Groundwater levels

Transient model calibration statistics were computed for the target monitoring wells in the SA, UFA, and LFA within the focus area (Table 3.8). The spatial distribution of mean error, expressed as the simulated minus observed water level, and observed versus simulated water levels for the target wells in the SA, UFA, and LFA in the recalibration focus area are compared in Appendix E.

Table 3.8. Transient model calibration period statistics of the target monitoring wells in the recalibration focus area with and without the recharge rate adjustment.

	Focus Area No recharge adjustment			Focus Area Recharge reduced by 20%		
	SA	UFA	LFA	SA	UFA	LFA
Residual Mean	-0.10	-0.25	-0.02	-0.41	-0.3	-0.18
Error Standard Dev	2.19	1.85	0.42	2.36	1.82	1.01
5% Observation Range	6.86	5.73	2.50	6.86	5.73	2.5
Absolute Residual Mean	1.90	1.60	0.82	1.93	1.52	0.91
Error sum of squares	331	293	2	398	287	11
RMS Error	2.17	1.86	0.4	2.38	1.84	0.98
Minimum Residual	-7.71	-8.00	-1.1	-9.7	-7.37	-2.92
Maximum Residual	9.37	5.11	0.47	8.21	4.67	1.03
# Observations	70	85	11	70	85	11
% MAE < 2.5 ft	87%	89%	100%	83%	88%	91%
% MAE < 5.0 ft	94%	96%	100%	94%	98%	100%
R ² > 0.4	84%	94%	100%	87%	96%	100%

All values in feet except as noted. Calibration period is 2004 to 2012. Mean error is expressed as simulated minus observed.

3.3.4 Spring flow

Simulation period model statistics for 17 magnitude 1 and 2 springs are included in Table 3.9. The observed and simulated flux in Table 3.9 is computed as the average for the 2003 to 2014 simulation period. A regression plot of simulated versus observed mean spring flow for the magnitude 1 and 2 springs is included in Appendix E.

Table 3.9. Transient model calibration statistics of the magnitude 1 and 2 target springs simulated in the ECCTX model.

Spring Name	Observed Flux (cfs)	Focus Area No recharge adjustment		Focus Area Recharge reduced by 20%	
		Simulated flux (cfs)	% error	Simulated flux (cfs)	% error
Lithia Spring Major	34.7	33.1	-4.5%	33.2	-4.4%
Buckhorn Main Spring	12.2	12.1	-1.0%	12.1	-1.0%
Sulphur Spring	33.7	35.4	2.0%	35.4	2.0%
Crystal Main Spring	45.5	46.3	1.9%	46.3	1.9%
Weeki Wachee Spring	160.4	167.3	4.4%	167.3	4.4%
Chassahowitzka Main Spring	59.6	59.3	-0.6%	59.3	-0.6%
Homosassa No. 1 Spring	83.5	84.5	1.2%	84.5	1.2%
Gum Spring Group	63.8	64.8	1.5%	64.8	1.5%
Rainbow Spring*	71.3*	73.3	2.0%	73.3	2.0%

Spring Name	Observed Flux (cfs)	Focus Area No recharge adjustment		Focus Area Recharge reduced by 20%	
		Simulated flux (cfs)	% error	Simulated flux (cfs)	% error
Apopka Spring	24.9	24.9	0.0%	24.7	-0.8%
Sanlando Springs	18.8	18.9	0.1%	18.5	-1.7%
Starbuck Spring	12.1	12.0	-0.5%	11.9	-1.9%
Wekiwa Spring	61.0	61.3	0.4%	61.1	0.1%
Bugg Spring	10.6	10.0	-5.2%	9.9	-7.0%
Rock Springs	54.9	54.7	-0.5%	53.1	-3.4%
Volusia Blue Spring	143.6	144.3	0.5%	144.4	0.6%
Alexander Spring	100.1	99.8	-0.3%	99.8	-0.3%

*Observed flow reduced by 88% since only 12% of springshed area represented in active domain

3.3.5 Baseflow

A total of 18 USGS gauges where baseflow was estimated are included in Table 3.10, alongside the minimum and maximum estimated baseflow. For both runs, with and without the recharge adjustment, a total of 15 out of 18 USGS gauges where baseflow was estimated were within the range of flows estimated baseflow.

Table 3.10. Simulated mean baseflow from 2004 to 2012 compared to estimated ranges using the USF method and USGS Groundwater Toolbox methods at 18 stations.

Gage	Station	Min (cfs)	Max (cfs)	Simulated (cfs)	
				No recharge adjustment	Recharge reduced by 20%
02232400	St. Johns River nr Cocoa FL	221	928	293	293
02232500	St. Johns River nr Christmas FL	282	1085	383	383
02234000	St. Johns River above Lake Harney nr Geneva FL	441	1473	860	784
02236000	St. Johns River Near DeLand FL	389	3186	1946	1730
02235000	Wekiva River nr Sanford	184	254	247	223
02294650	Peace River at Bartow	21	125	78	77
02294898	Peace River at Fort Meade	19	144	129	129
02295637	Peace River at Zolfo Springs	80	350	334	331
02296750	Peace River at Arcadia	118	596	537	532
02298830	Myakka River nr Sarasota	14	150	50	50
02300500	Little Manatee River nr Wimauma	23	88	70	70
02301500	Alafia River at Lithia	53	189	111	110
02303000	Hillsborough River nr Zephyrhills	65	145	102	102
02310000	Anclote River nr Elfers	4	38	11	11
02312000	Withlacoochee River at Trilby	28	153	164	164
02312500	Withlacoochee River at Croom	56	211	195	194

Gage	Station	Min (cfs)	Max (cfs)	Simulated (cfs)	
				No recharge adjustment	Recharge reduced by 20%
02312762	Withlacoochee River nr Inverness	156	378	46	42
02313000	Withlacoochee River nr Holder	237	505	1	-2

3.3.6 Aquifer and Confining Unit Properties

Horizontal hydraulic conductivity maps for all layers, leakance coefficient maps for layers 2, 6, 8, and 10, and UFA and LFA transmissivity maps for the recharge sensitivity simulations are included in Appendix E.

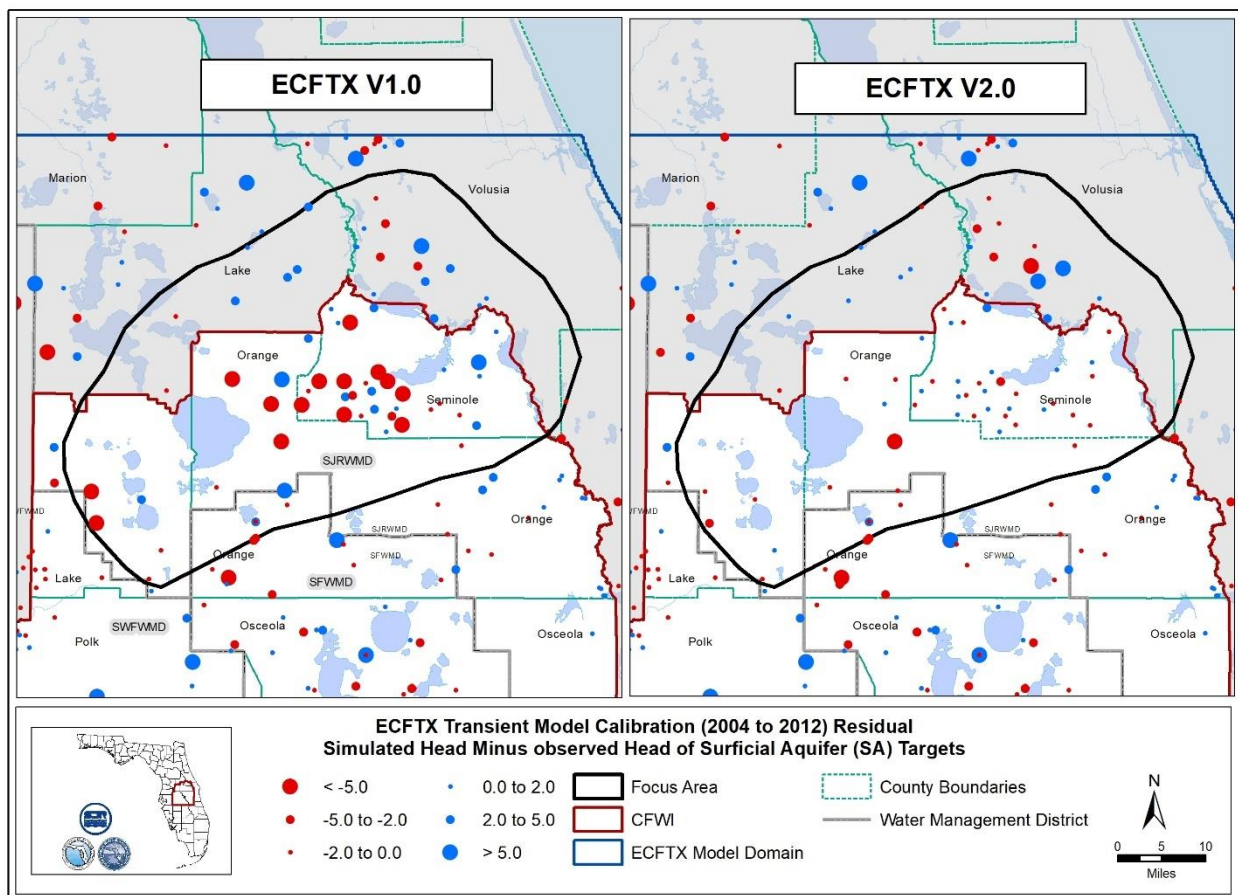


Figure 3.1. Spatial distribution of mean error for the SA targets within the focus area in the ECFTX transient model calibration.

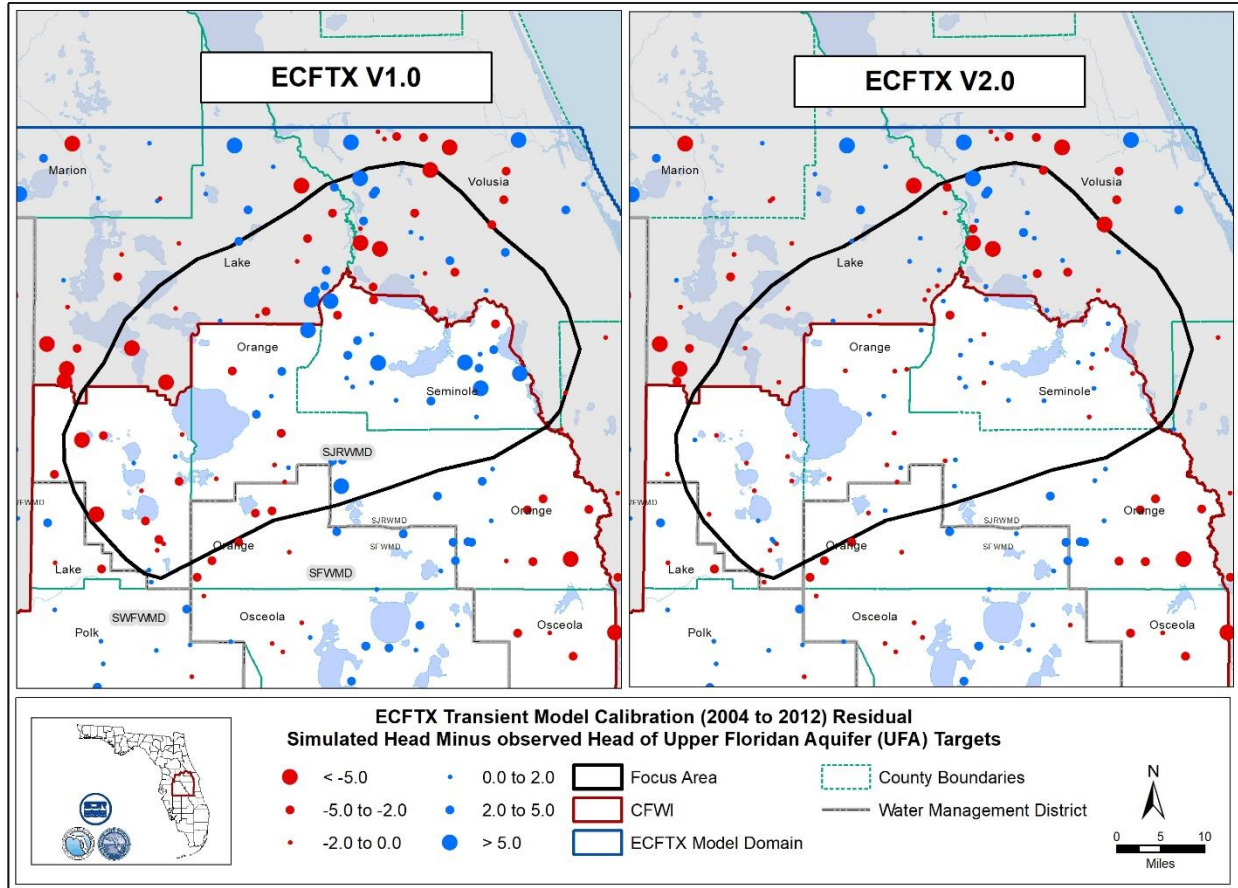


Figure 3.2. Spatial distribution of mean error for the UFA targets within the focus area in the ECFTX transient model calibration.

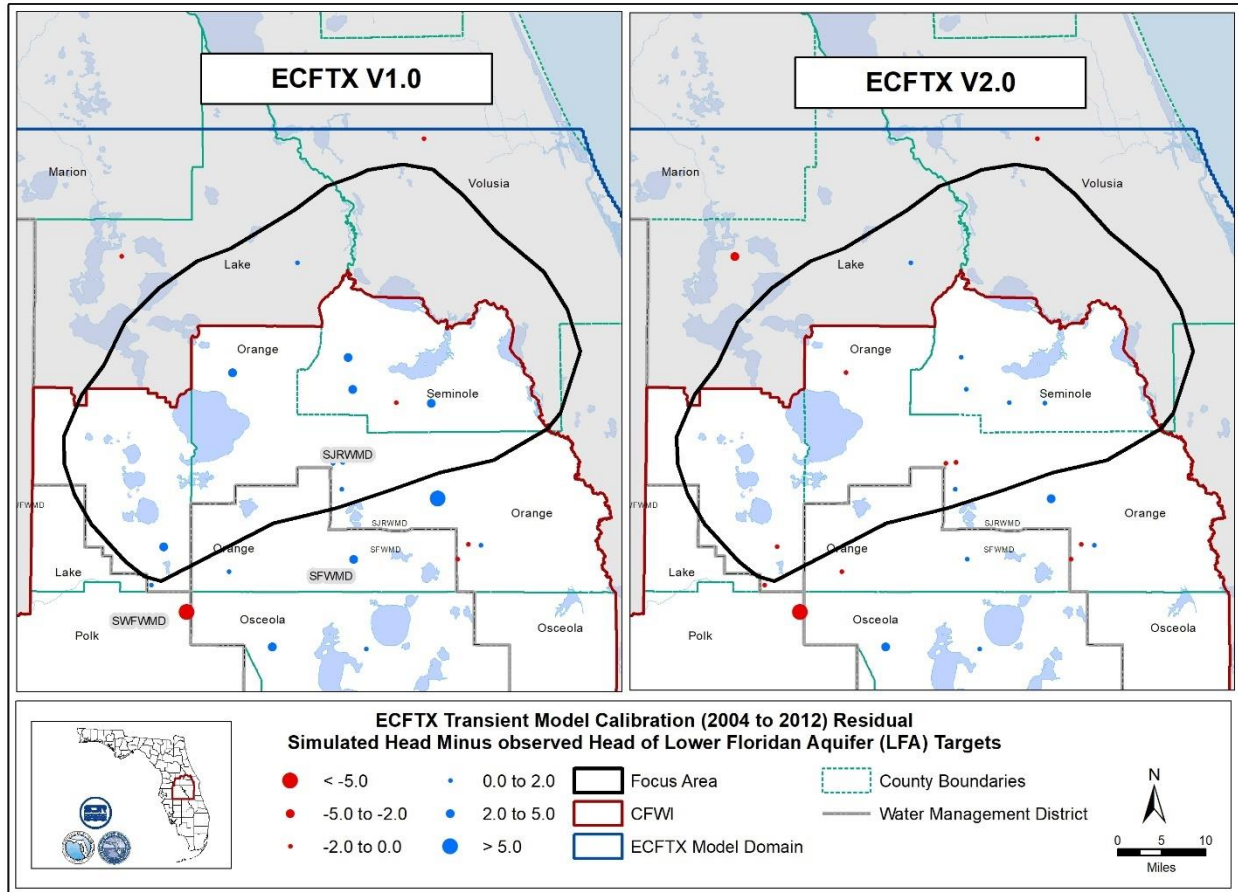


Figure 3.3. Spatial distribution of mean error for the LFA targets within the focus area in the ECFTX transient model calibration.

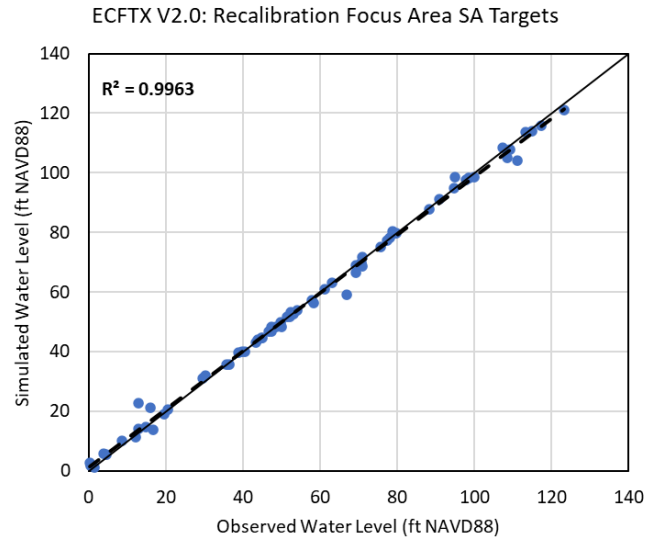
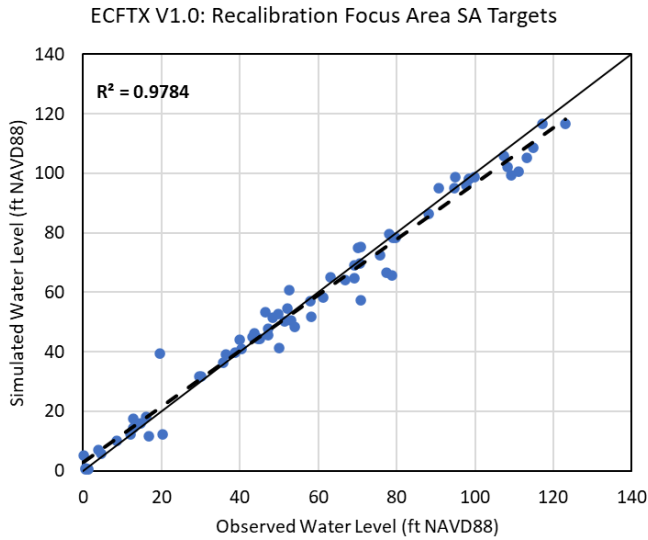


Figure 3.4. Mean simulated versus observed water levels for the SA within the focus area in the ECFTX transient model for V1.0 (left) and V2.0 (right). (Note: Solid line is 1:1 relation between simulated and observed water levels; dashed line is linear regression of simulated versus observed water levels from target wells)

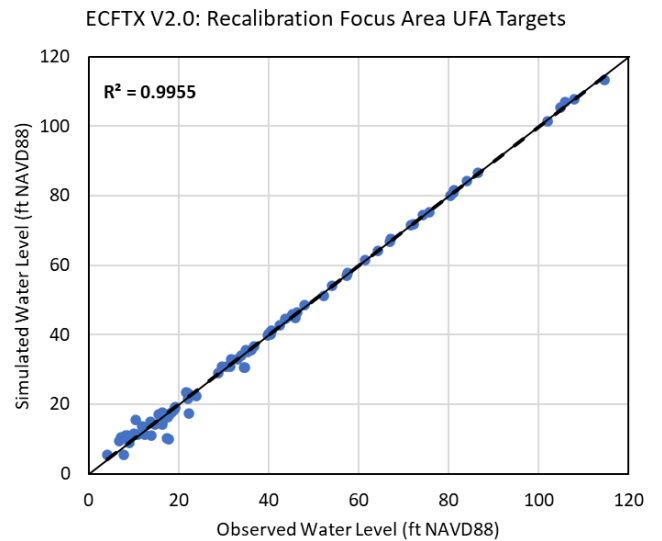
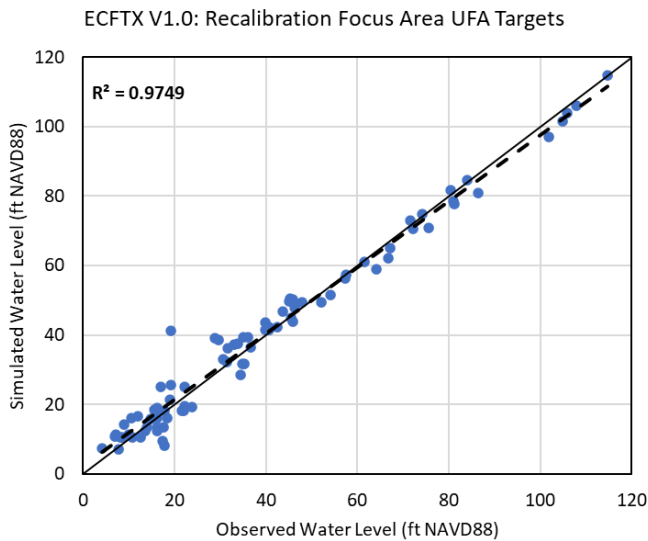


Figure 3.5. Mean simulated versus observed water levels for the UFA within the focus area in the ECFTX transient model for V1.0 (left) and V2.0 (right). (Note: Solid line is 1:1 relation between simulated and observed water levels; dashed line is linear regression of simulated versus observed water levels from target wells)

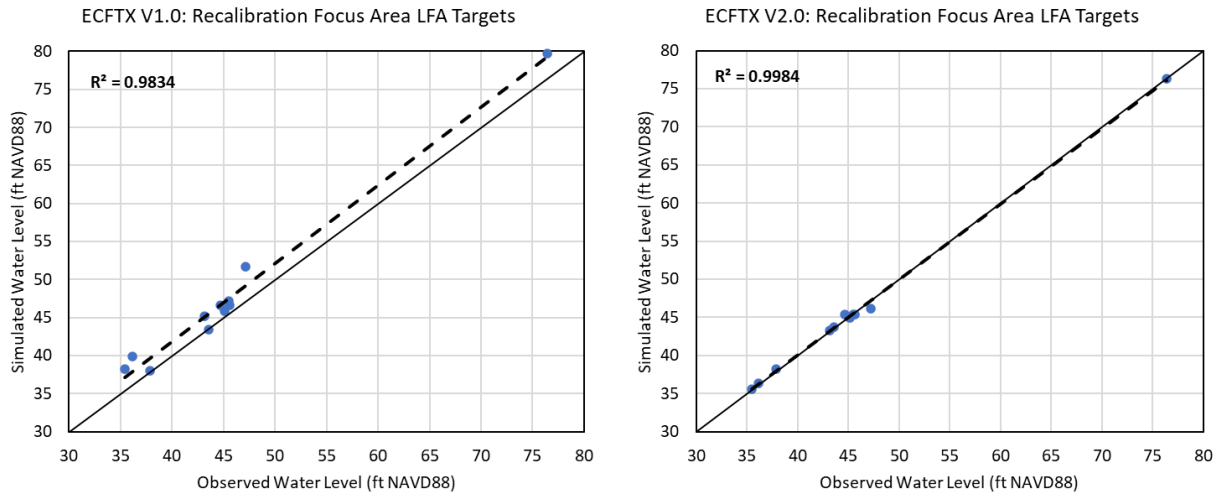


Figure 3.6. Mean simulated versus observed water levels for the LFA within the focus area in the ECFTX transient model for V1.0 (left) and V2.0 (right). (Note: Solid line is 1:1 relation between simulated and observed water levels; dashed line is linear regression of simulated versus observed water levels from target wells)

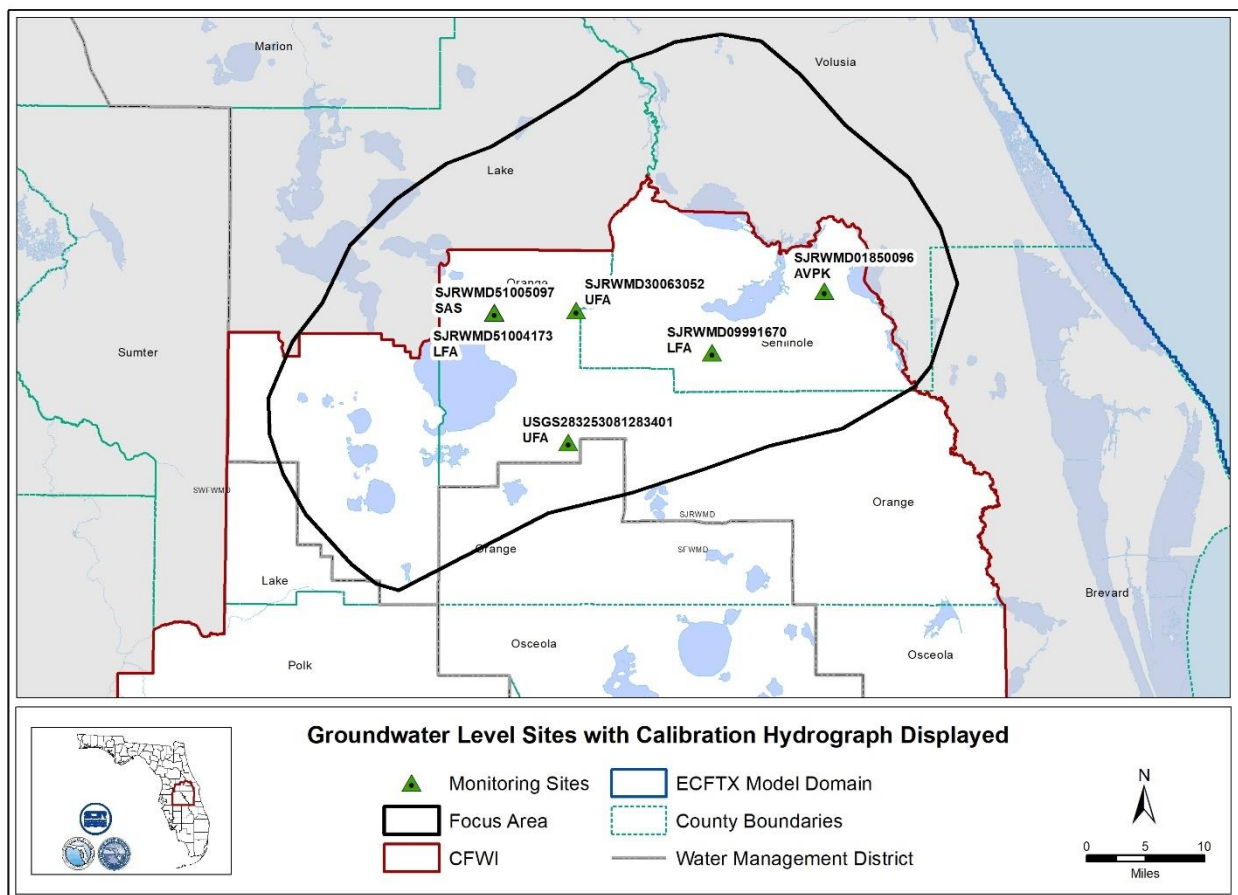


Figure 3.7. Location of hydrographs of selected simulated versus observed water levels for the SA, UFA, and LFA within the focus area.

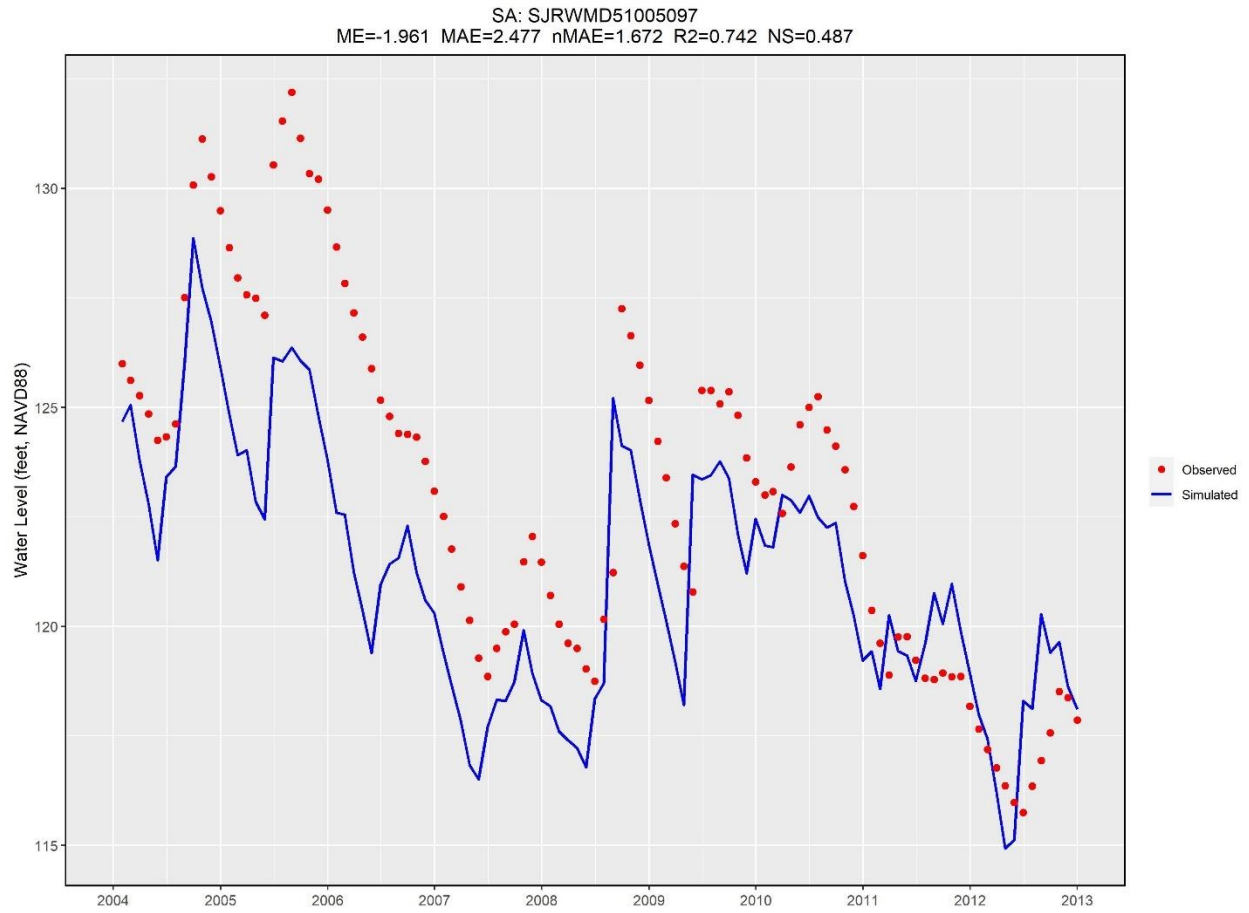


Figure 3.8. Simulated versus observed water levels for the SA monitor well OR0107 at Plymouth Tower.

UFA: SJRWMD30063052
ME=-0.247 MAE=0.464 nMAE=0.413 R2=0.663 NS=0.553

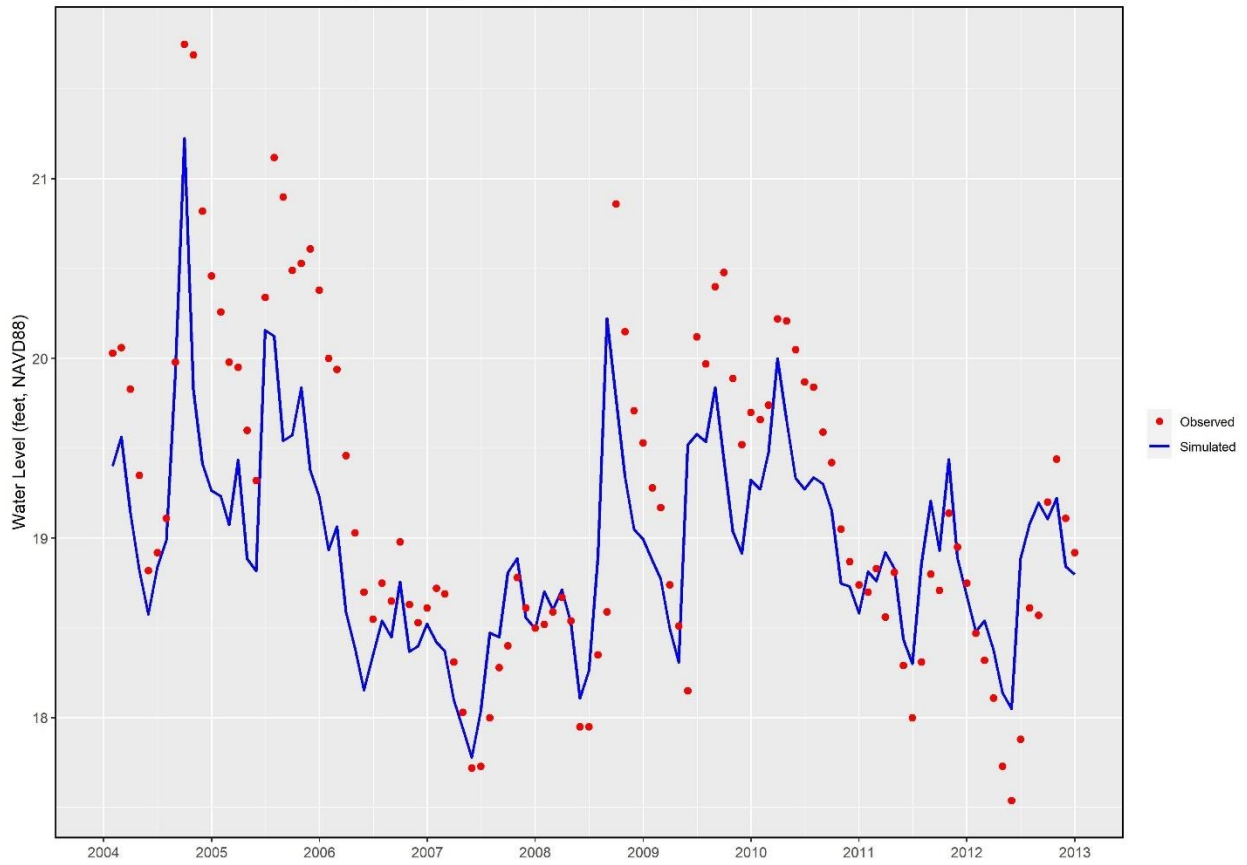


Figure 3.9. Simulated versus observed water levels for the UFA well OR0548 at Wekiwa Springs State Park.

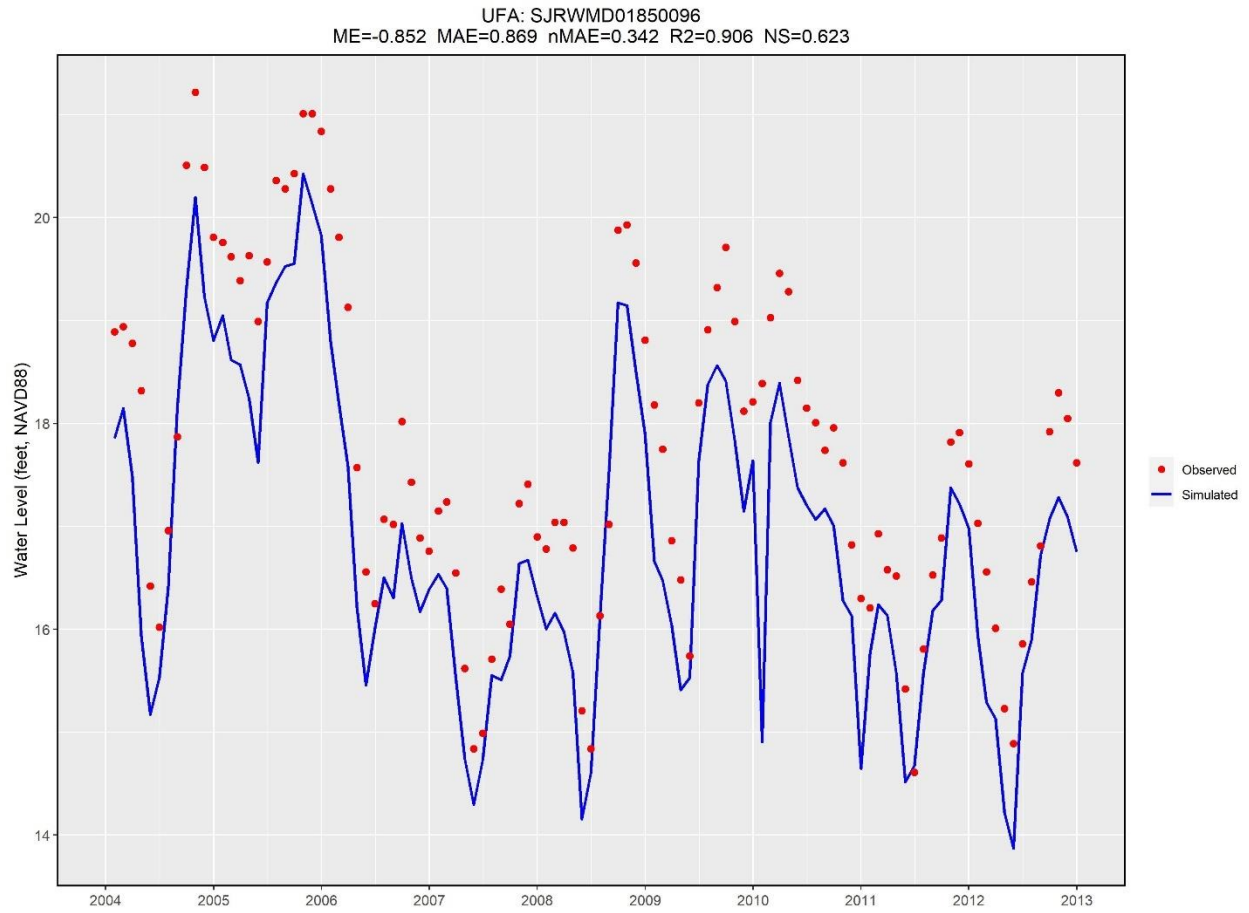


Figure 3.10. Simulated versus observed water levels for the UFA well S-1224 at Geneva Fire Station.

UFA: USGS283253081283401
ME=0.265 MAE=2.331 nMAE=2.359 R2=0.368 NS=0.078

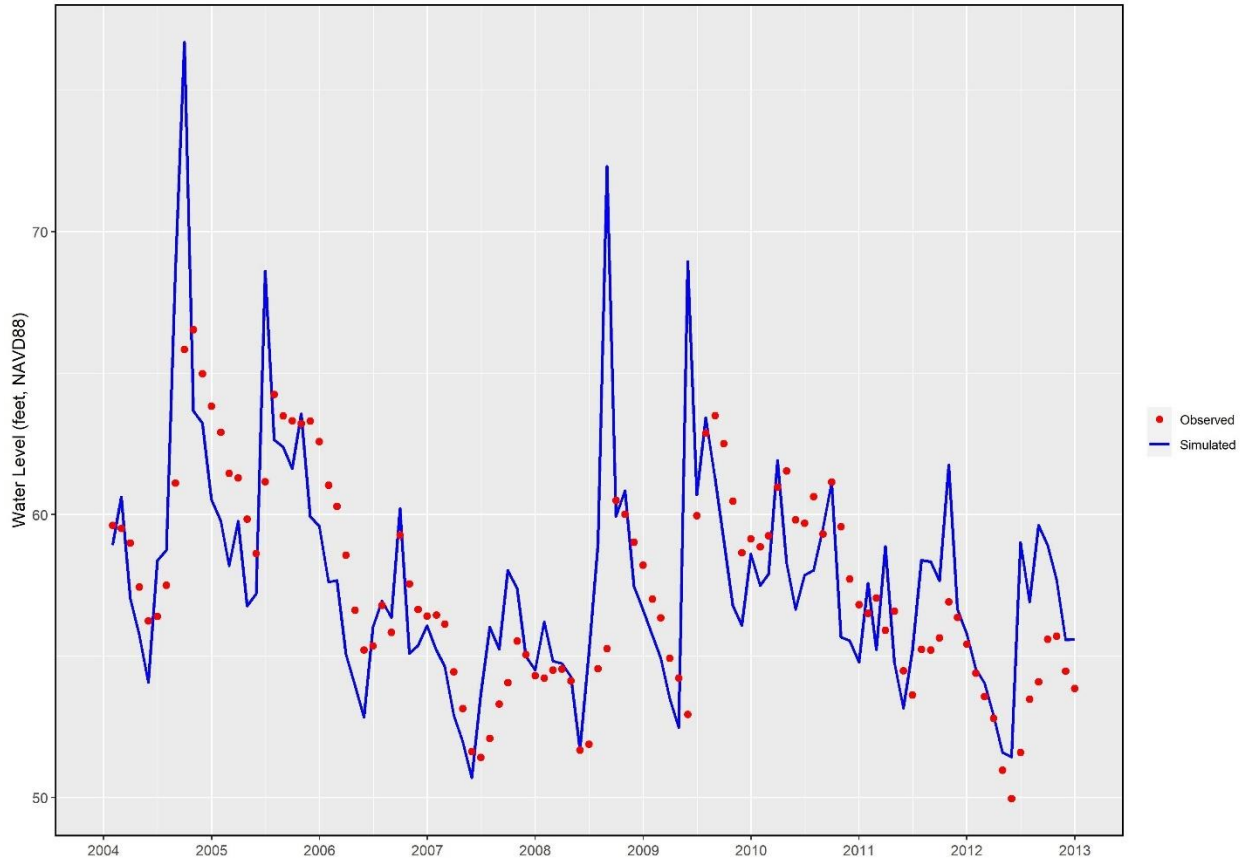


Figure 3.11. Simulated versus observed water levels for the UFA well OR-47 at Orlo Vista, FL.

LFA: SJRWMD51004173
ME=-1.096 MAE=1.44 nMAE=1.028 R2=0.767 NS=0.591

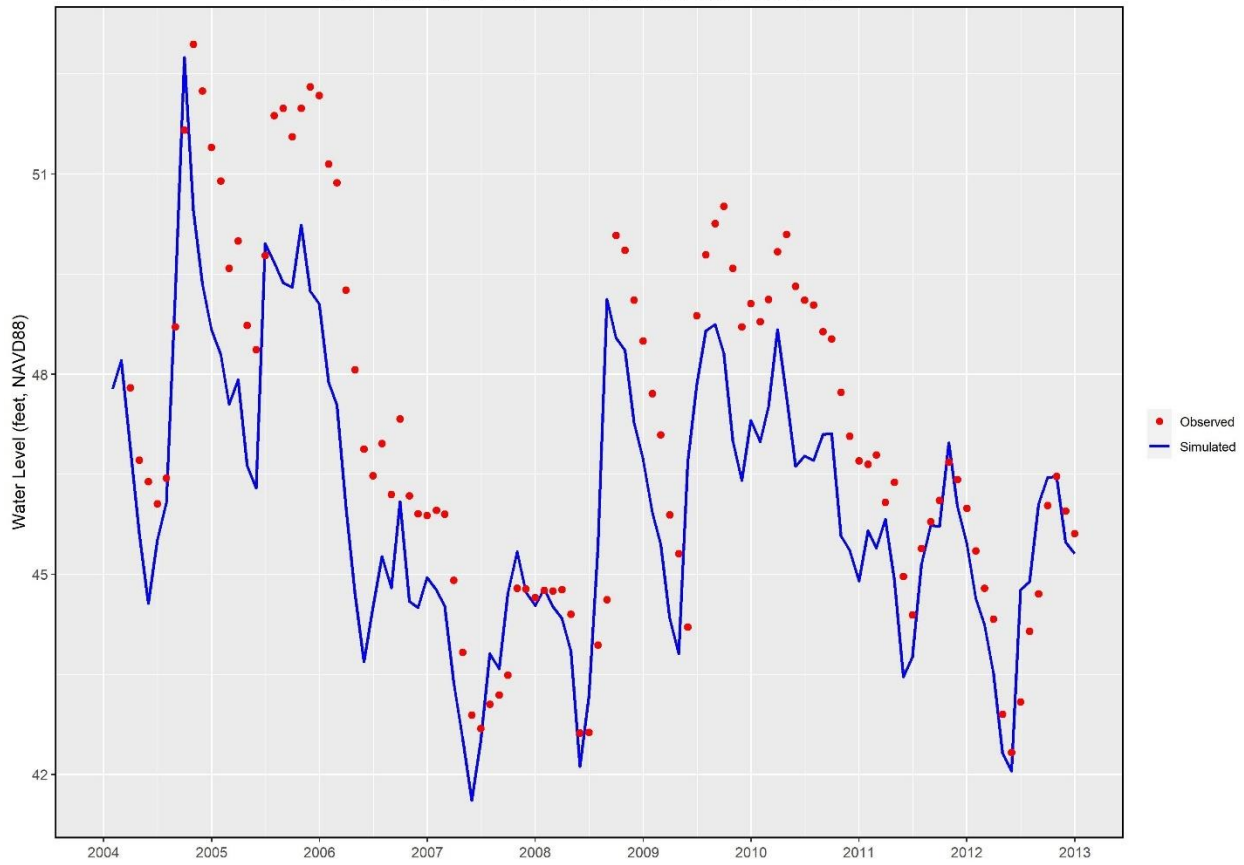


Figure 3.12. Simulated versus observed water levels for the LFA well OF794 at Plymouth Tower.

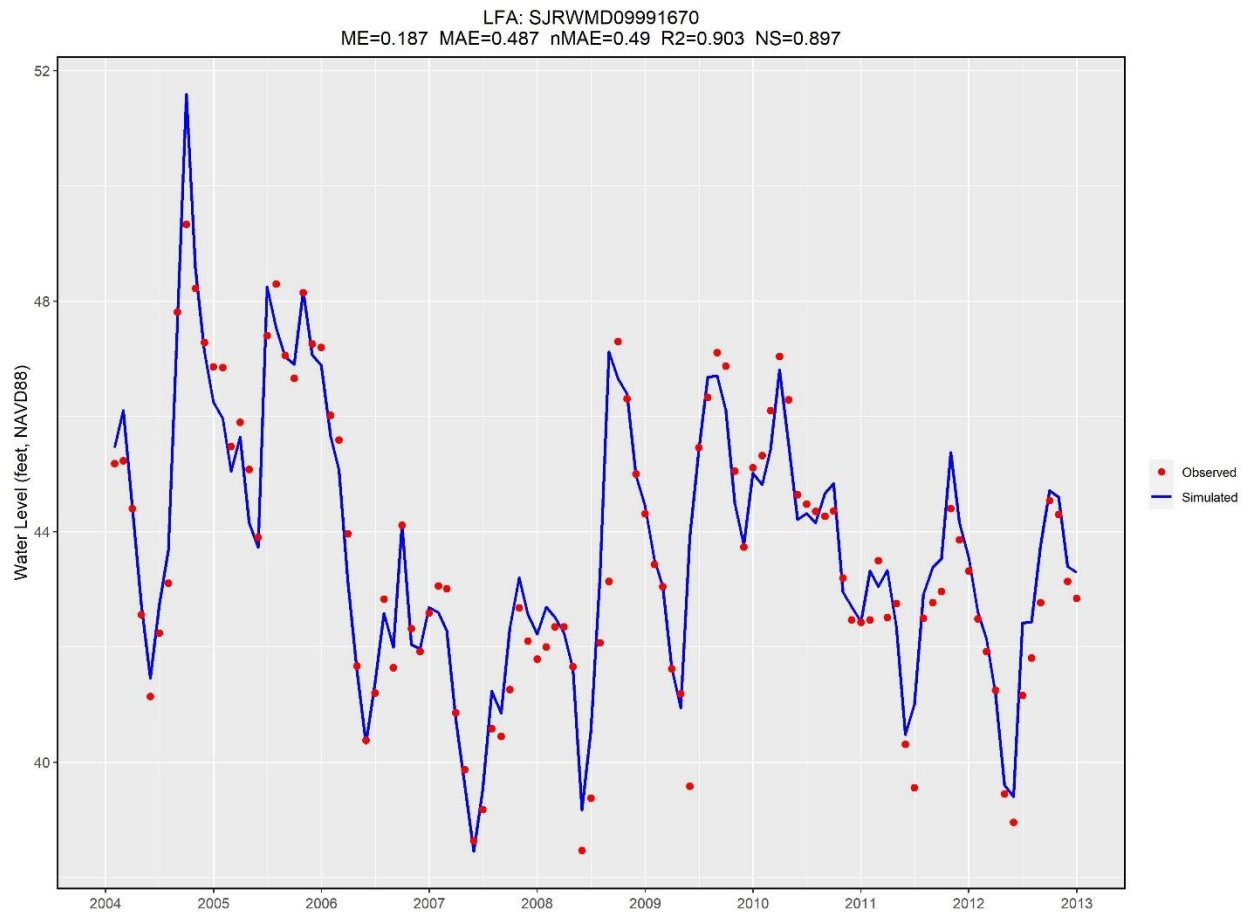


Figure 3.13. Simulated versus observed water levels for the LFA well S-1329 at Winter Springs at Casselberry.

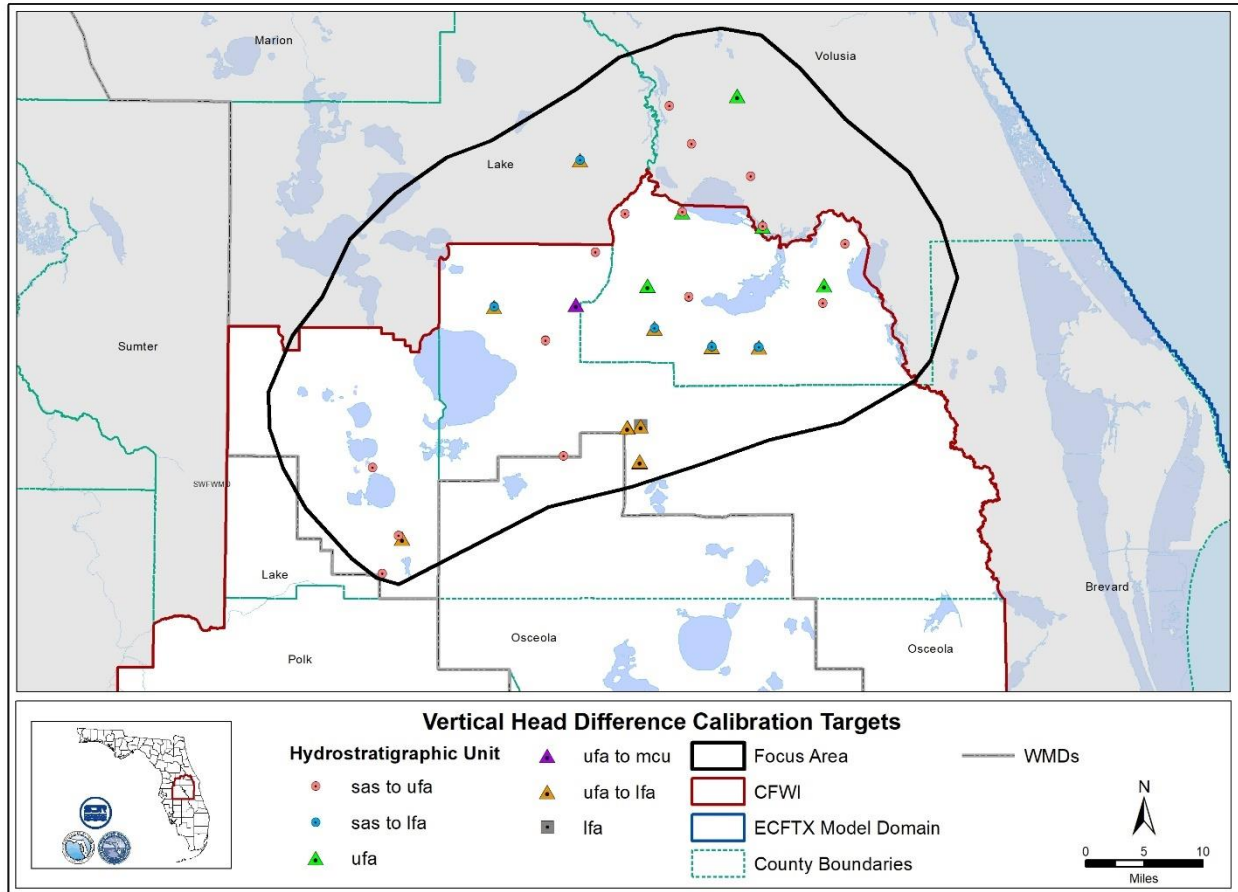
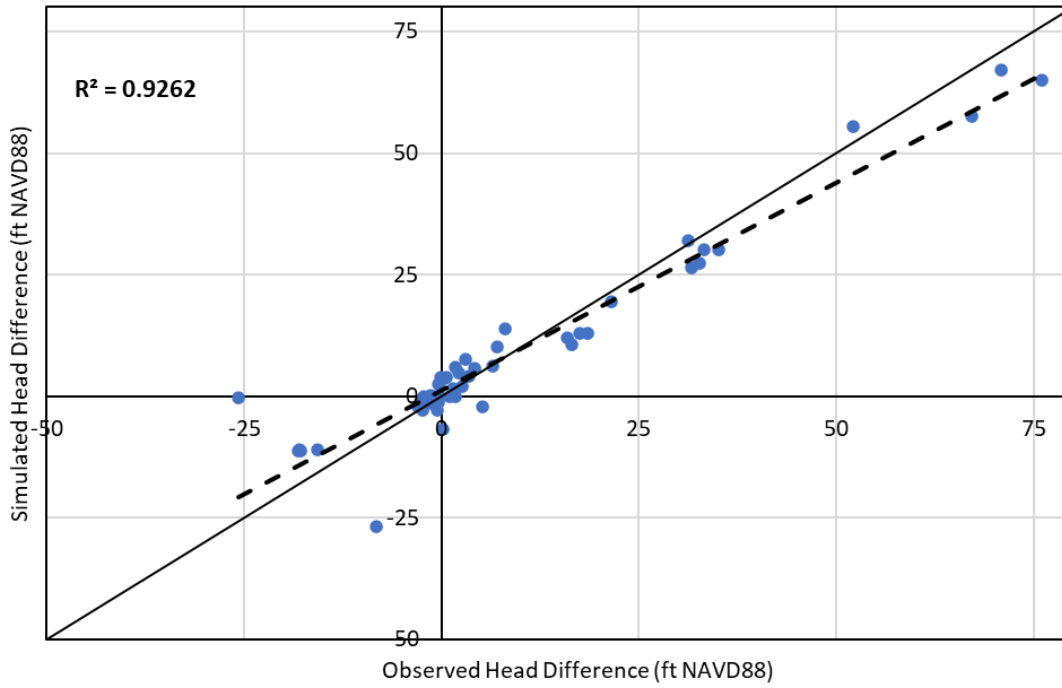


Figure 3.14. Location of vertical head difference targets in the focus area.

ECFTX V1.0: Focus Area Vertical head Difference Targets



ECFTX V2.0: Focus Area Vertical head Difference Targets

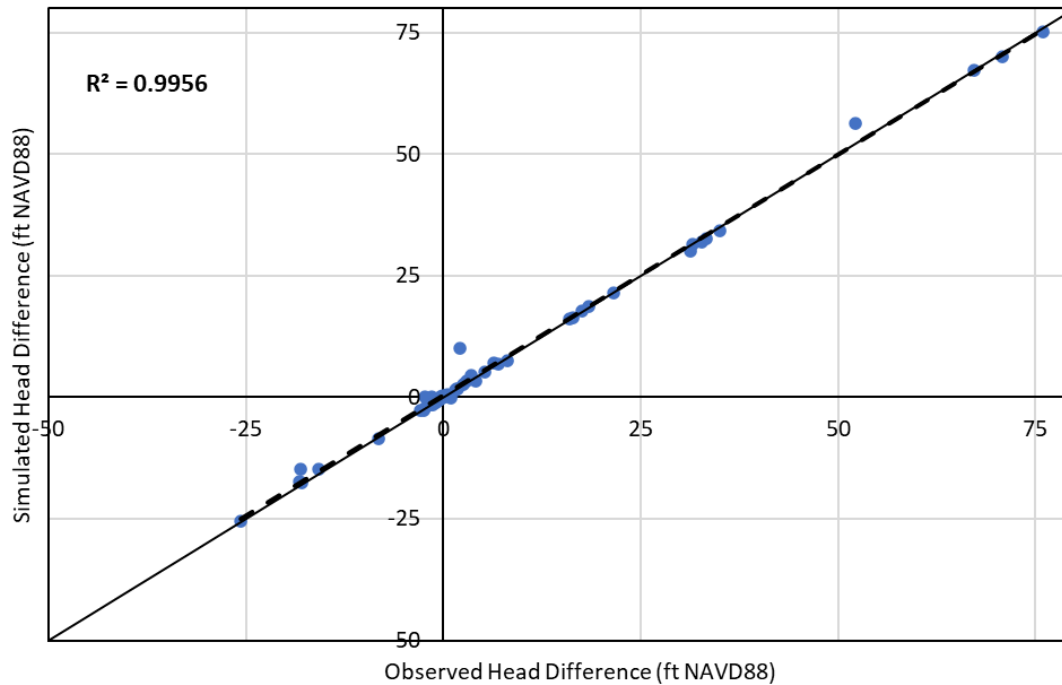


Figure 3.15. Mean simulated versus observed vertical head differences at targets within the focus area in the ECFTX transient model for V1.0 (top) and V2.0 (bottom). (Note: Solid line is 1:1 relation between simulated and observed head differences; dashed line is linear regression of simulated versus observed head differences).

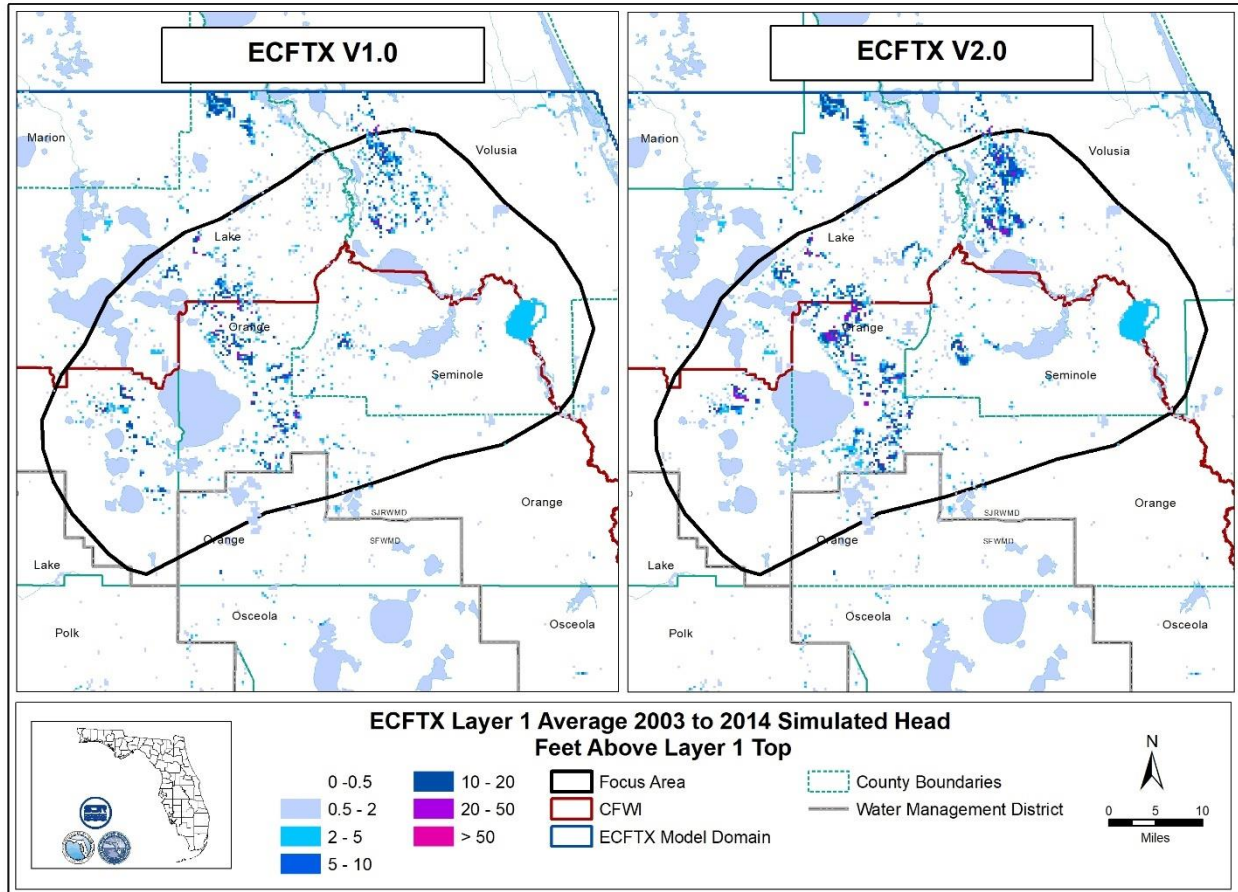


Figure 3.16. Simulated average 2003 to 2014 layer 1 flooded depth for ECFTX v1.0 (left) and ECFTX v2.0 (right).

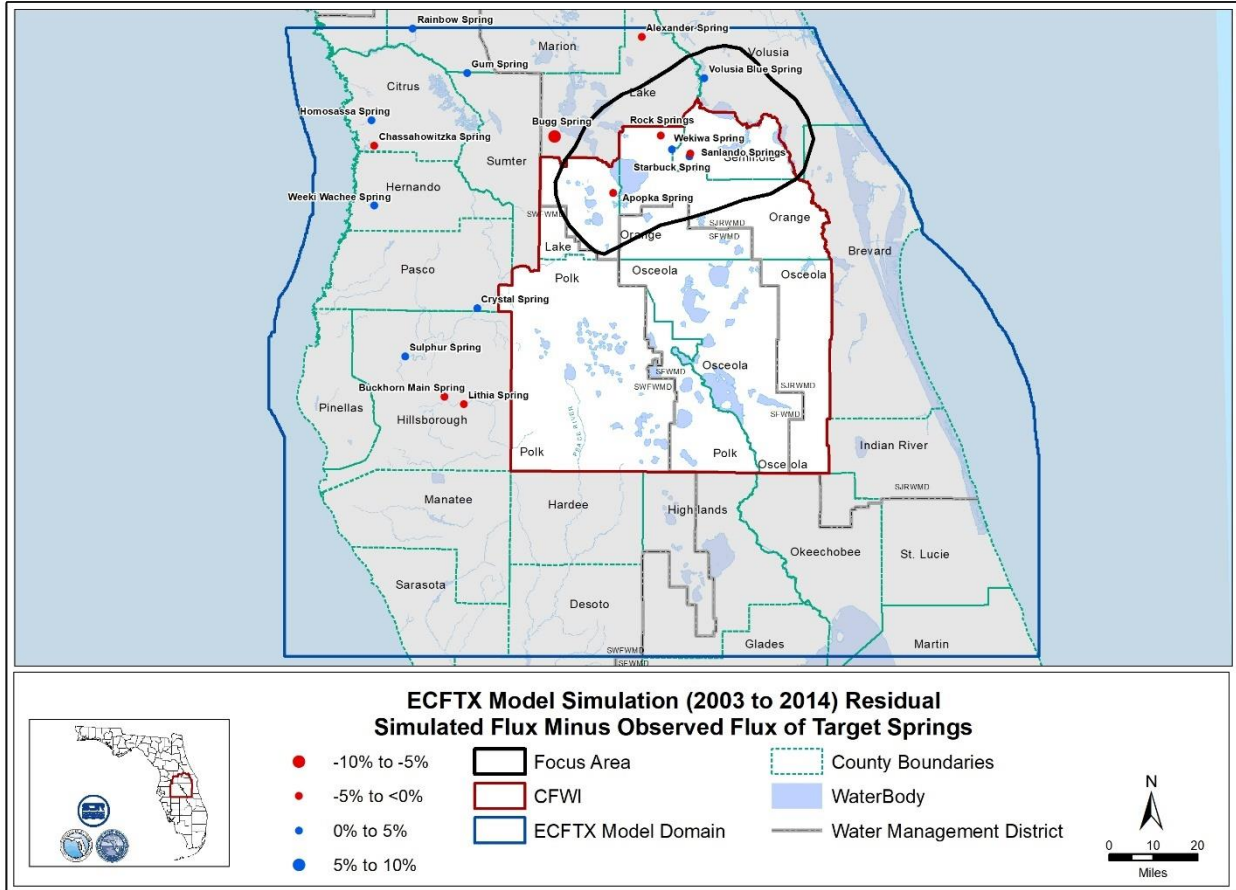


Figure 3.17. Spatial distribution of mean error for 17 magnitude 1 and 2 springs within the ECFTX model during calibration period. Blue indicates simulated flows higher than observed, red indicates simulated flows lower than observed.

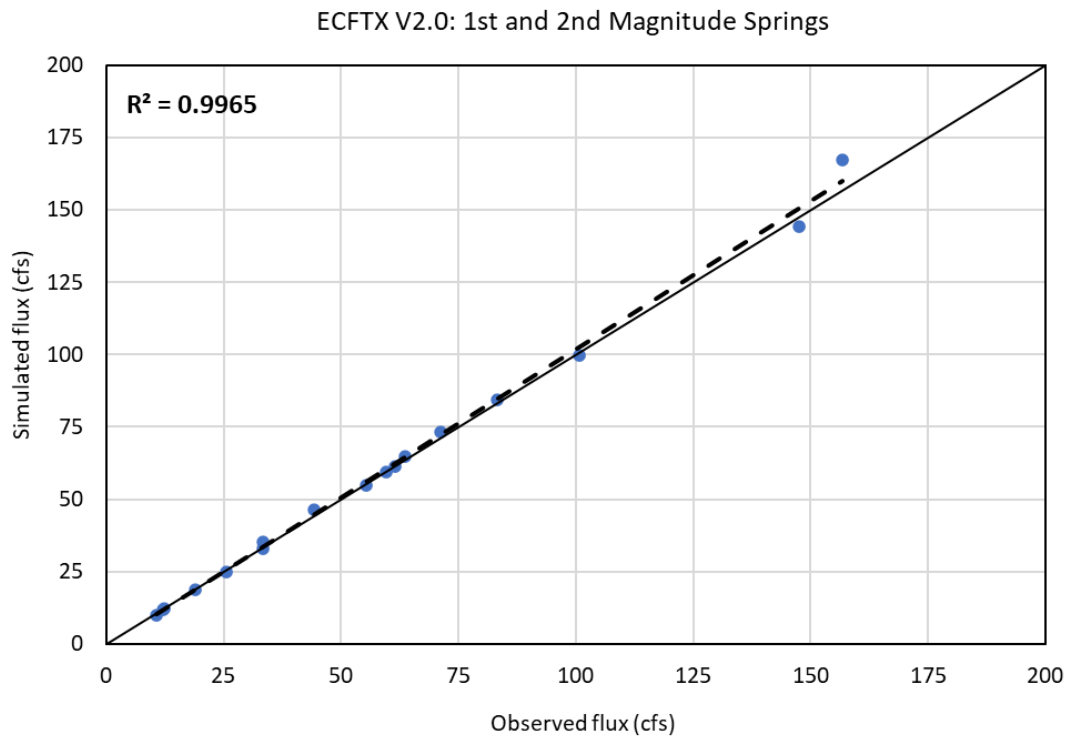
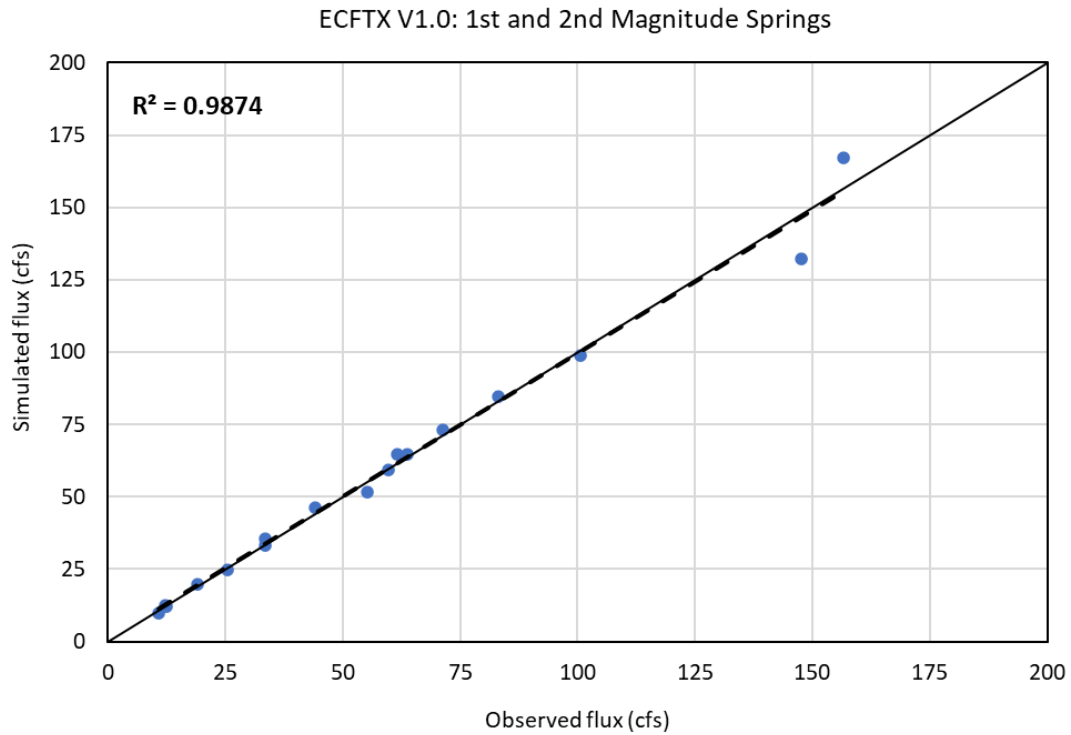


Figure 3.18. Mean simulated versus observed flow for magnitude 1 and 2 springs within the ECFTX model domain. Note: solid line is 1:1 relation between simulated and observed flow; dashed line is linear regression of simulated versus observed flow from 17 springs.

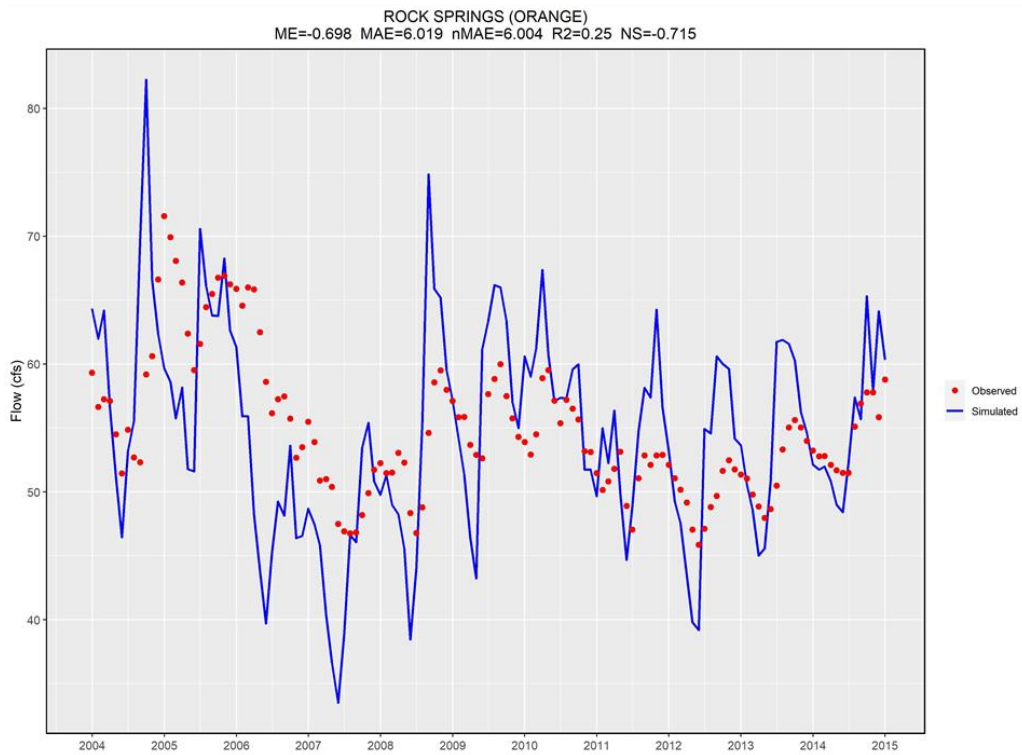
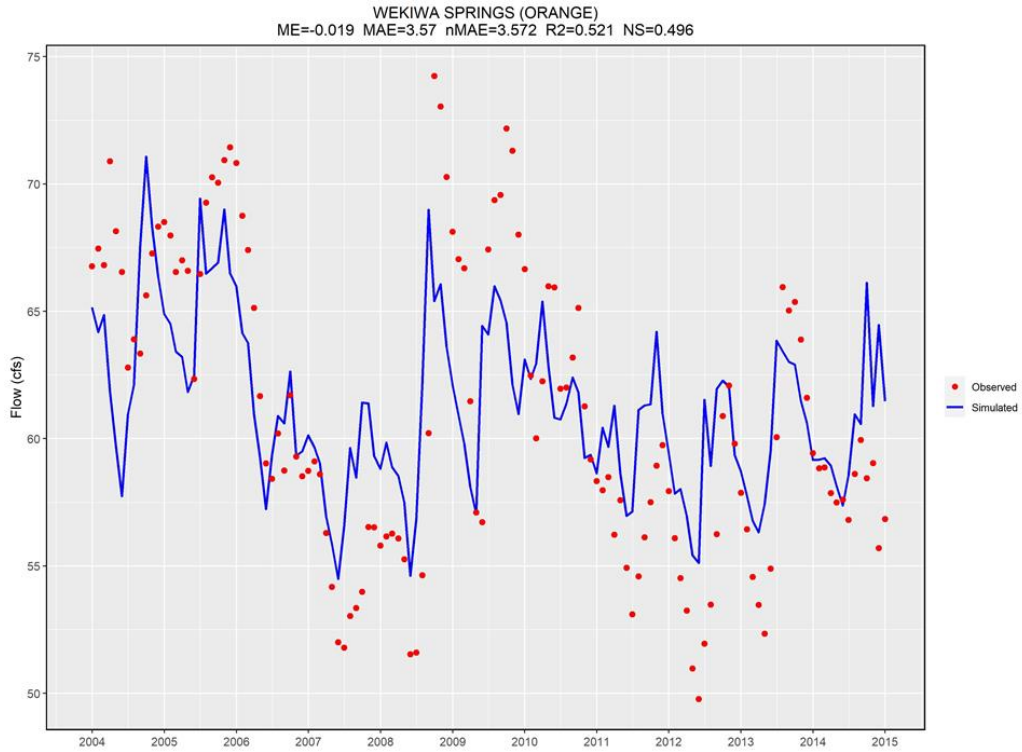


Figure 3.19. Simulated versus observed flows at Wekiwa (top) and Rock springs (bottom) within the ECFTX model.

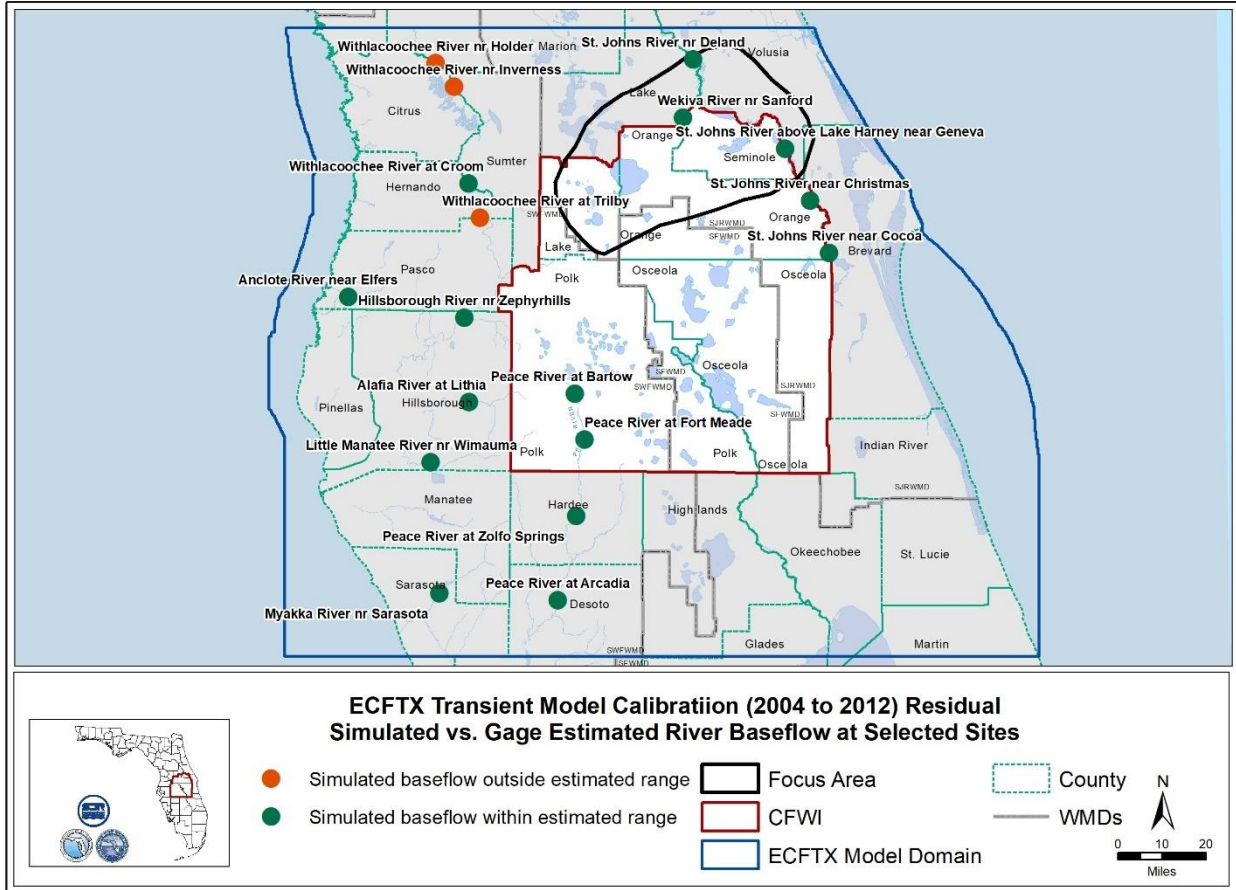


Figure 3.20. Spatial distribution of the USGS streamflow gages that were within or outside the baseflow estimation ranges within the ECFTX domain for the calibration period.

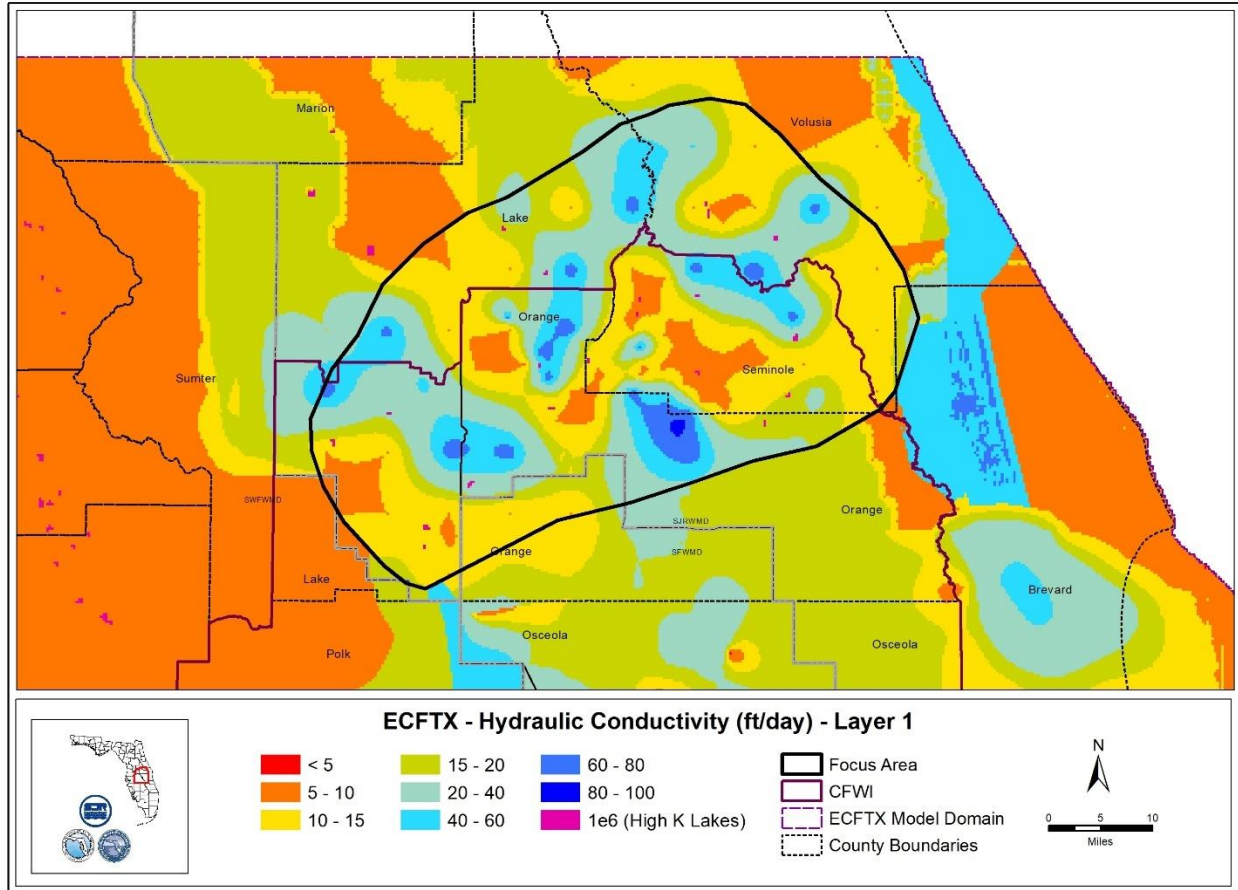


Figure 3.21. Hydraulic conductivity values in layer 1 in the focus area.

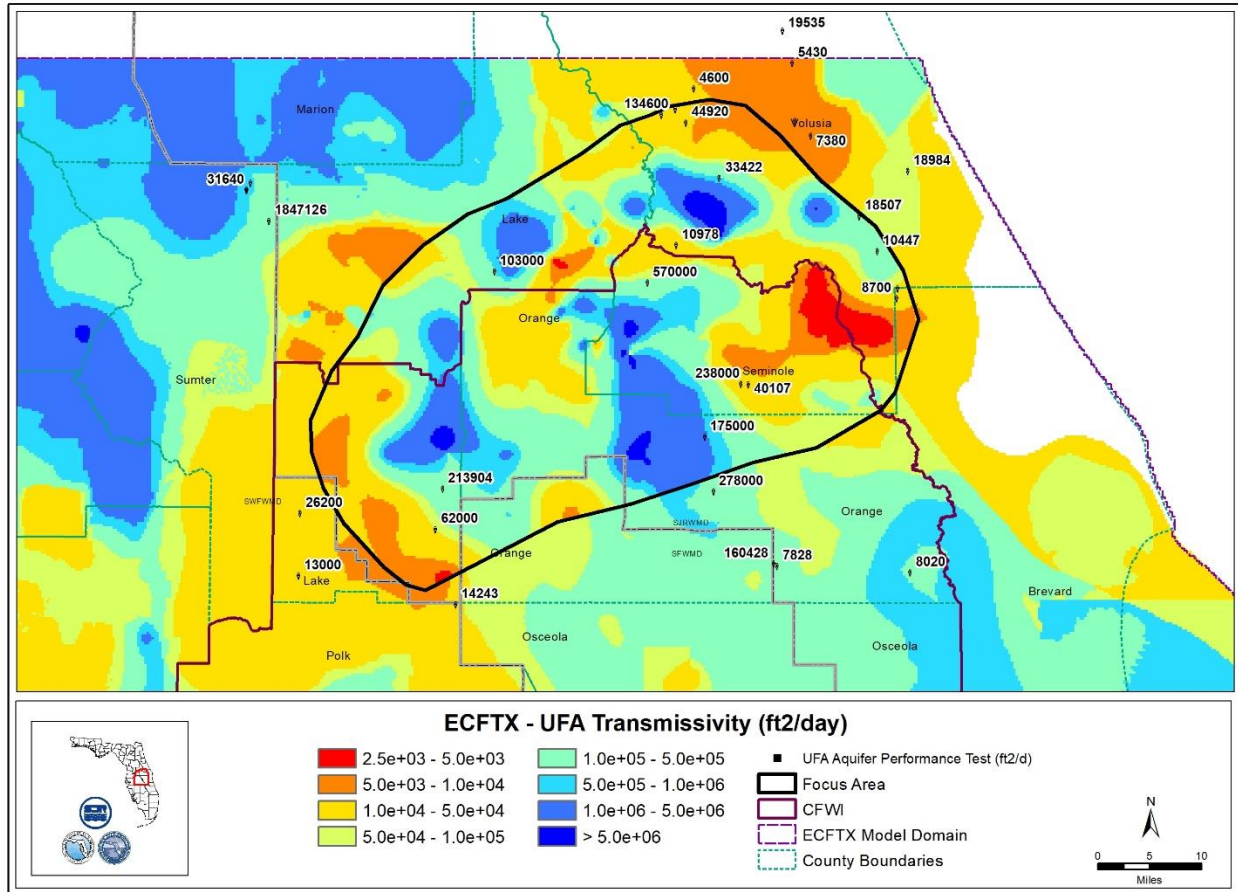


Figure 3.22. Transmissivity values for the UFA (layers 3–5) plotted with historical APT results.

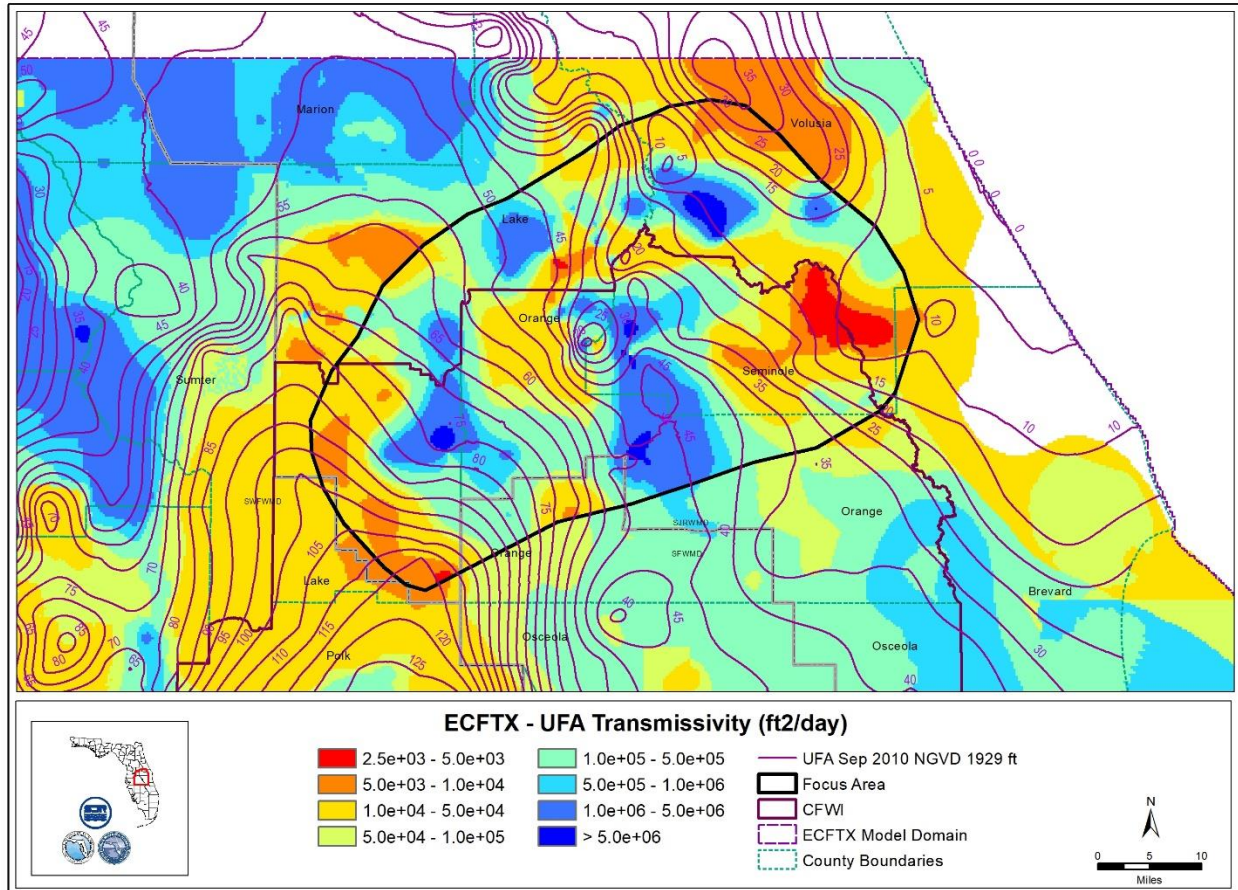


Figure 3.23. Transmissivity values for the UFA (layers 3–5) plotted with the September 2010 UFA potentiometric surface.

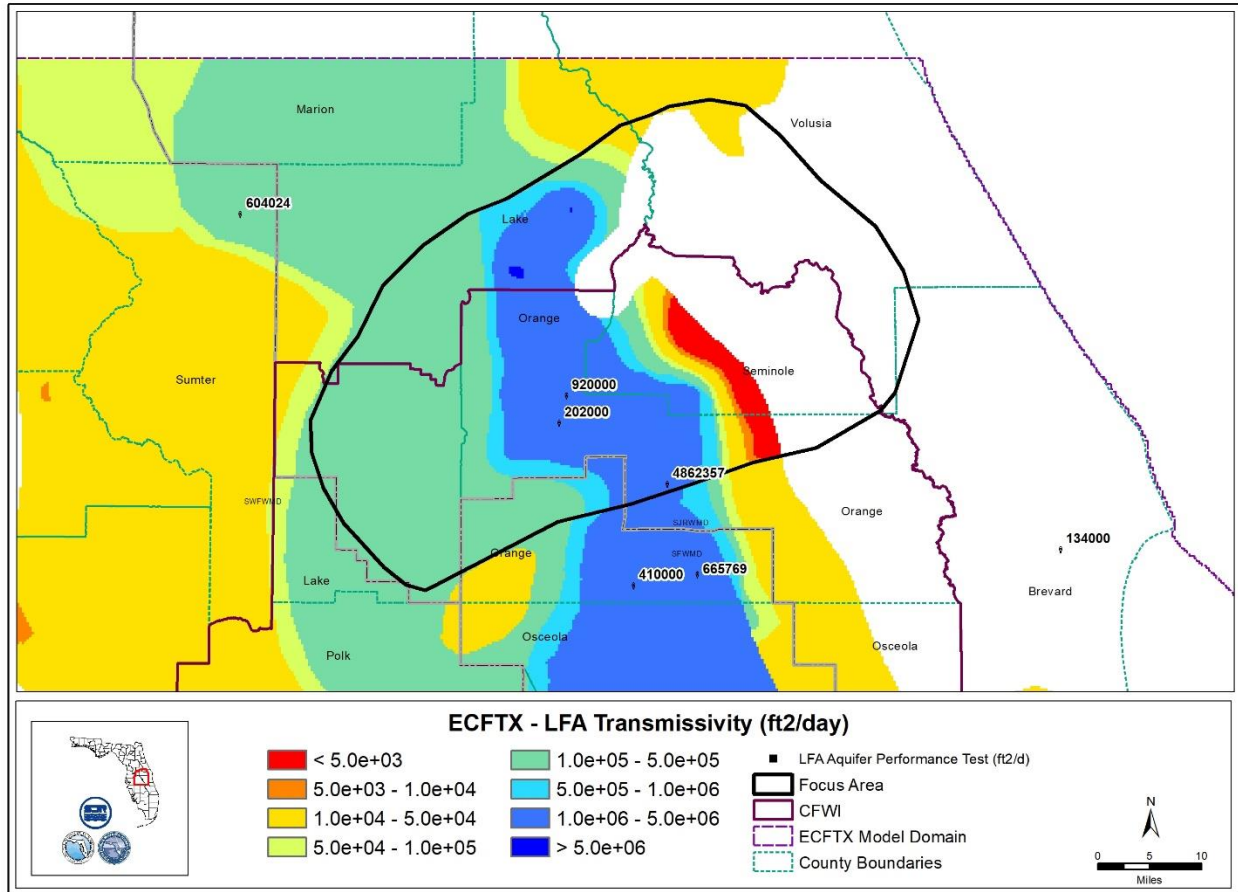


Figure 3.24. Transmissivity values for the LFA (layers 9–11) plotted with historical APT results.

CHAPTER 4 – DISCUSSION

The recalibration of the ECFTX model significantly improved the model performance in the focus area. Average RMS error in the focus area reduced from about 5.1 to 2.2 feet in the SA, from 4.4 feet to 1.9 feet in the UFA and from 2.4 to 0.4 feet in the LFA. In addition, the coefficient of determination (R-squared) values greater than 0.4 were generated by regressing all simulated versus observed water levels in ECFTX v2.0 and compared with those in ECFTX v1.0. These R-squared values were considered a measure of how well the model performed matching shorter-term transient response to dynamic stresses (CFWI HAT, 2020). Compared to ECFTX v1.0, ECFTX v2.0 had higher percentage of R-squared values greater than 0.4 for SA and the same for the UFA and LFA in the focus area. Although overall calibration has been improved significantly, water levels of a few target wells in CFWI were not matched as well as they were in ECFTX v1.0 but the difference was small (increase in MAE < 0.5 ft in the SA and <1.5 ft in the UFA). The maximum increase in MAE (approximately 1.3 ft) was noticed at a monitoring well in northeast Seminole County (SJRWMD ID: 30342858). It appears that the adjustment made to improve several wells near Wekiva River having very high MAEs in ECFTX v1.0 (Figure 3.2) degraded one well slightly.

The simulations of major springs were also improved as the average model error decreased from about 6% to less than 1% in Wekiwa and Rock springs. Moreover, VHDs between UFA and SA and between UFA and LFA were introduced as calibration targets which were not utilized quantitatively in the ECFTX v1.0 calibration. The VHDs are one of the primary indicators of the degree of confinement between two aquifers and helped us improve the model's ability to simulate degree of confinement in the region. This is important for accurately predicting the propagation of impacts of groundwater pumping in the UFA and LFA to lakes, rivers, and wetlands. While the calibration improved in the focus area, no significant changes occurred outside it as a result. The improvement in model-wide calibration performance reflects the improvement in focus area calibration performance.

We assessed the reasonableness of the updated simulated hydraulic conductivities by reviewing the transmissivity values of UFA and LFA with APT/literature data, spring locations (karst-dominated geology) and potentiometric surface contour gradients. The leakage values of the ICU and MCU were better represented based on VHDs and literature information.

Figure 4.1 shows the updated UFA transmissivity values with September 2010 potentiometric surface contours, the available APTs, and spring locations. Figure 4.2 shows the updated LFA transmissivity values with the available APTs, and spring locations. Small and large spacings between two contours of potentiometric surface are usually indications of low or high aquifer transmissivities respectively. As shown in Figure 4.1, the recalibrated parameter distribution is generally consistent with the contour spacing as high transmissivity areas usually coincide with the contours with large spacing (flat gradients) whereas low transmissivity areas usually coincide with contours with tight spacing (steep gradients). In addition, very high transmissivity values were assigned to the areas of springs and their vicinities, which is consistent with the fact that aquifers are expected to be highly transmissive in the vicinity of springs due to presence of conduits and large fractures. Although the recalibrated transmissivity values are similar to the APT-derived

values in most of the focus area, the transmissivity values in the model are much higher than the APT-derived values in northern Orange County; however, the horizontal hydraulic conductivity values shown in Figures 60 and 61 of the USGS-ECFT model report, corresponding to layers 3 and 5 of the model, seem to be similar to the calibrated hydraulic conductivities in those areas (Sepulveda et al, 2012). Hydraulic conductivity maps of each layer in the model are included in Appendix D. It should also be noted that APT values should be cautiously used for comparing with model parameters. APT values are usually derived from field tests using analytical solutions with limitations. The quality of the field tests and collected data would significantly affect the transmissivity values derived from the APTs. In addition, the APTs (mostly lasting less than 72 hours) usually do not sufficiently stress the aquifer more than a few miles so the derived transmissivity values may not represent large areas. Moreover, some of the APTs are based on only one pumping well (with no monitoring well nearby) and can produce highly questionable transmissivity estimates due to pumping well frictional effect mixing with aquifer water level change due to pumping.

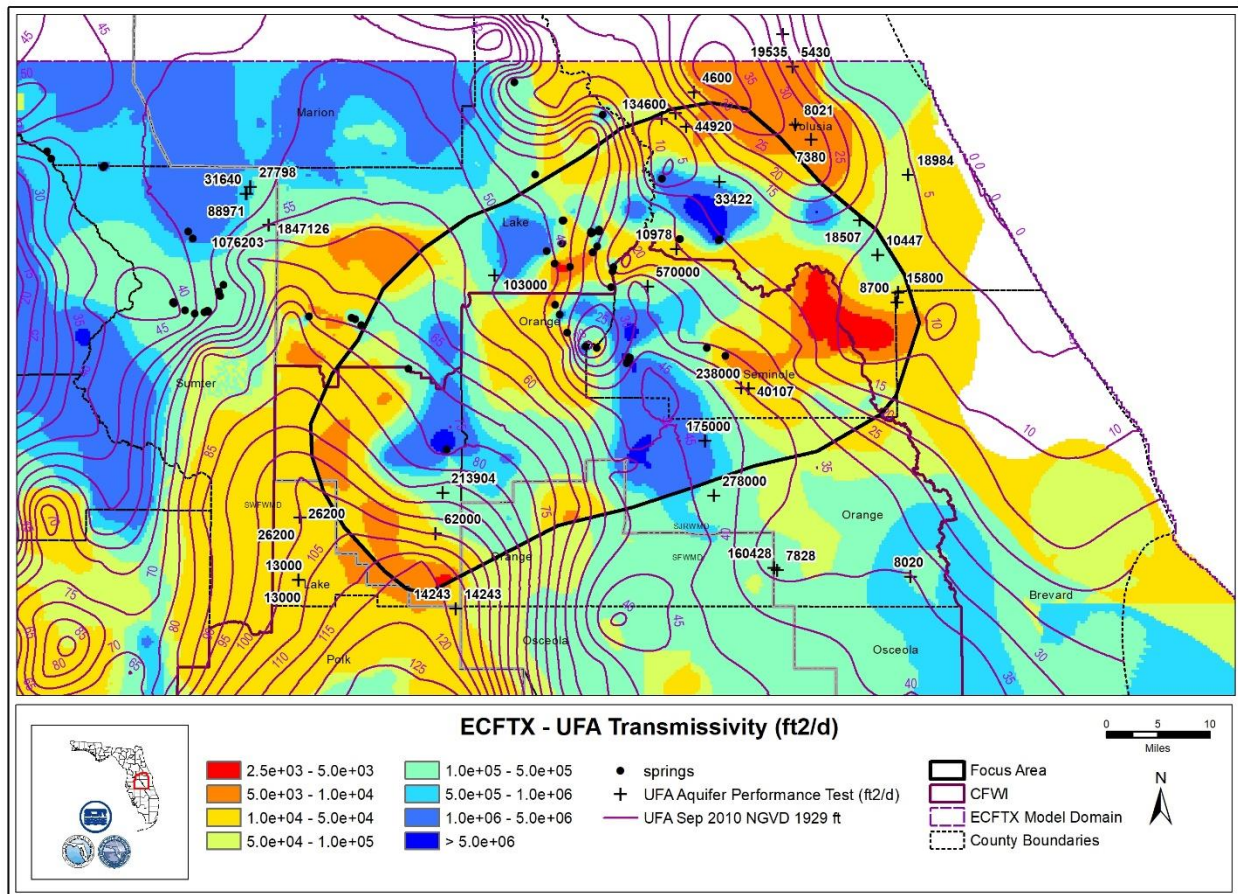


Figure 4.1. UFA Transmissivities with potentiometric surface, spring locations, and APTs.

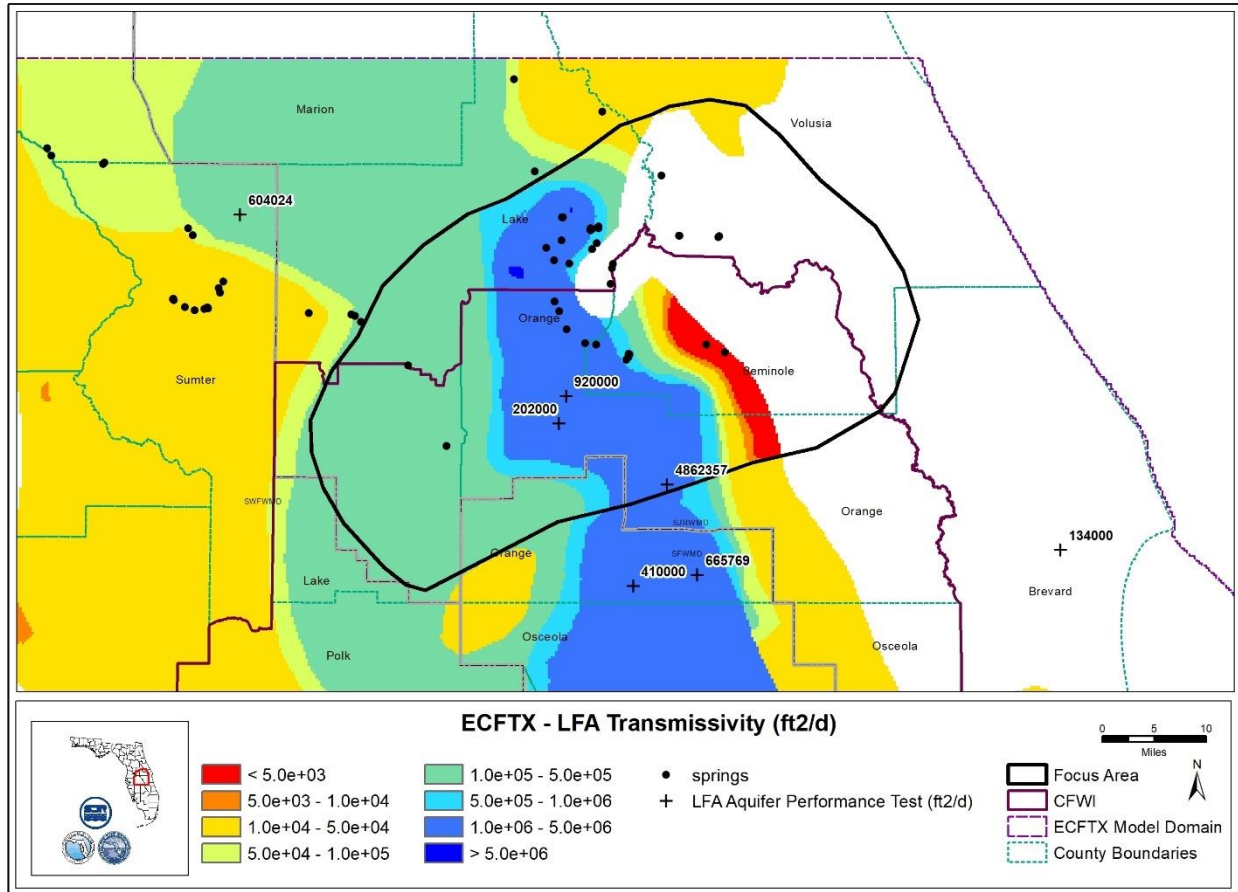


Figure 4.2. LFA Transmissivities with APTs.

Figure 4.3 shows the recalibrated leakance values in the UFA with VHDs between the SA and UFA. As expected, low leakance values are in the areas of large VHDs and high leakance values are in the areas of small VHDs, indicating the reasonableness of the leakance values in the ICU. Similarly Figure 4.4 shows consistency between the VHDs and leakance values in MCU 1.

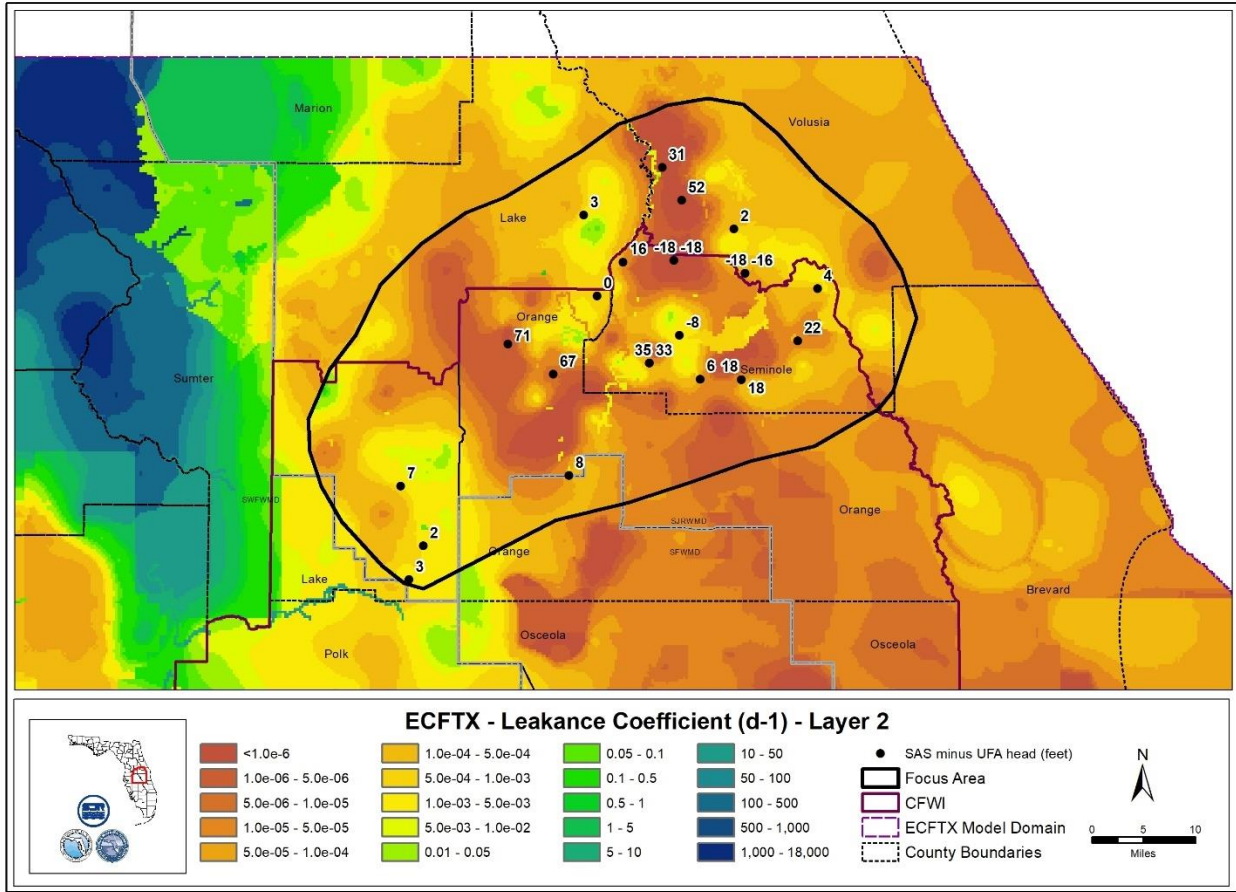


Figure 4.3. ICU Leakances with SA/UFA VHDs.

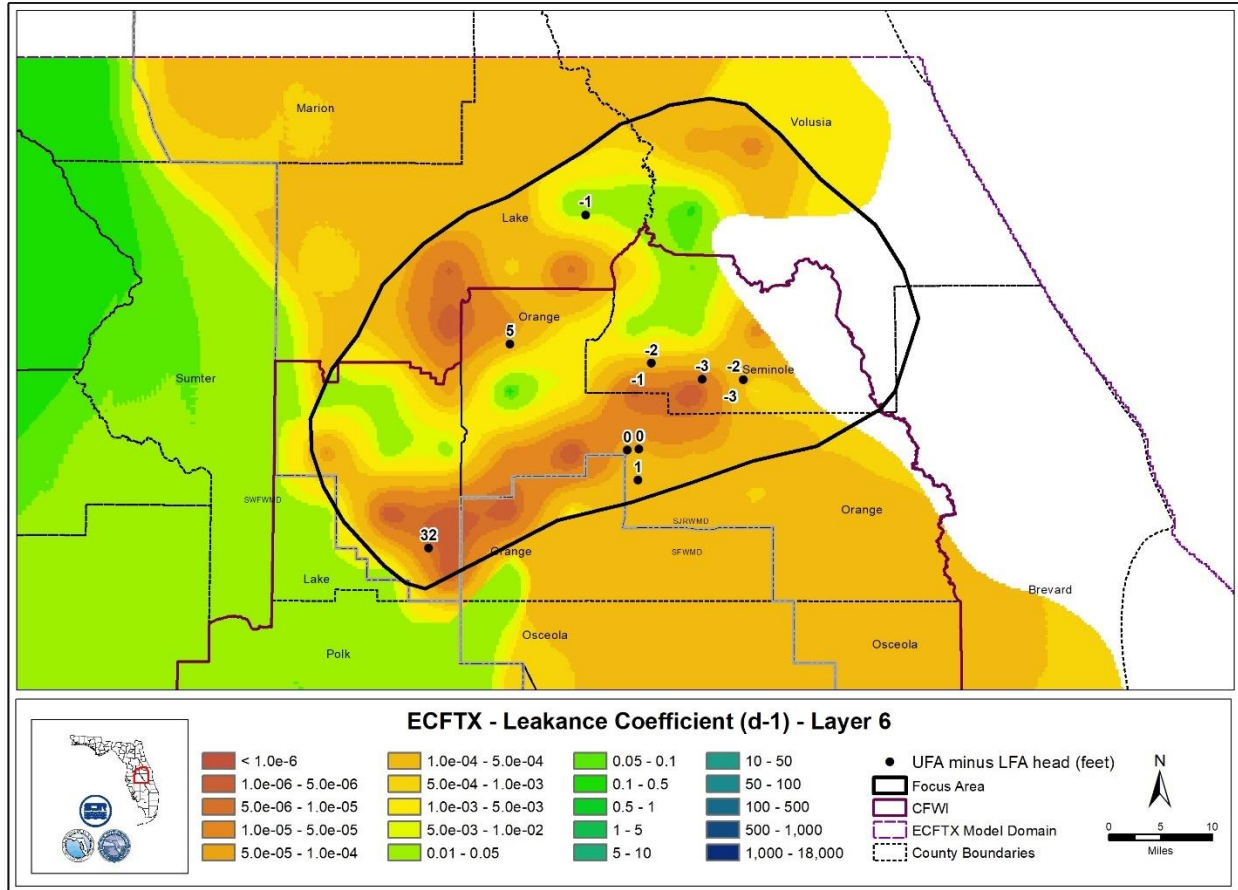


Figure 4.4. MCU_I Leakance distribution with UFA/LFA VHDs.

During the recalibration effort, the recharge used in ECFTX v1.0 was retained. However, because of uncertainties in recharge estimation, a recharge sensitivity calibration simulation was conducted to better understand the effect of recharge on model calibration. A full recalibration was performed using PEST by reducing the recharge by 20%. The purpose was to see if a good calibration (i.e., calibration statistics were similar to the ECFTX v2.0 calibration) could be achieved when the recharge was 20% less and what the parameter distribution would be, compared to the updated parameters. The results indicated that the calibration statistics of ECFTX v2.0 were similar to or better than those of the recalibrated model with 20% recharge adjustment, although a reasonably good calibration was achieved under a reduced recharge condition. However, parameter distribution of the sensitivity run would still be like ECFTX v2.0, providing us more confidence that the final parameters in ECFTX v2.0 were reasonable.

GHB fluxes along the eastern seawater/freshwater interface boundary were a significant concern during calibration of ECFTXv1.0. Large influxes from this boundary would result in an artificial source of freshwater that would tend to artificially mitigate drawdown impacts from wellfield withdrawals. Care was taken during recalibration to adjust GHB conductance to ensure that GHB fluxes in ECFTX v2.0 were similar to those of ECFTXv1.0

CHAPTER 5 – CONCLUSIONS

The ECFTX v2.0 model performance was considerably improved within the focus area including the Wekiva River springs groundwater contributing basin and Seminole County. Aquifer parameters were adjusted within a range consistent with the known hydrogeology in the region. Accordingly, the model-wide calibration performance was also improved as a result of the improvement in the focus area. Overall, this provides greater confidence that ECFTX v2.0 should be considered an appropriate tool for assisting regulatory decisions, minimum flows and minimum levels (MFL) evaluations, and future planning efforts.

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LIST OF APPENDICES

Appendix A. SWFWMD Layer 2 Hydraulic Conductivity Updates

Appendix B. Transient model calibration well hydrographs

Appendix C. Transient model calibration spring hydrographs

Appendix D. Aquifer and Confining Unit Properties

Appendix E. Recharge Sensitivity Results

Appendix A – East-Central Florida Transient Expanded (ECFTX) Model Horizontal Hydraulic Conductivity Modifications in Layer 2

TECHNICAL MEMORANDUM

TO: Central Florida Water Initiative (CFWI) Hydrologic Assessment Team (HAT)

FROM: Hua Zang, P.G., Ph.D., Environmental Flows and Levels Section
Jason G. Patterson, P.G., Environmental Flows and Levels Section

DATE: November 10, 2021

SUBJECT: East-Central Florida Transient Expanded (ECFTX) Model Horizontal Hydraulic Conductivity Modifications in Layer 2

The East Central Florida Transient Expanded (ECFTX) groundwater flow model and the subsequent model documentation report were completed by the Central Florida Water Initiative (CFWI) Hydrologic Assessment Team (HAT) in February 2020. During recent discussions, the Districts' members of HAT determined that it was necessary to modify the horizontal hydraulic conductivity (Kh) for layer 2, which represents the intermediate confining unit (ICU) within the ECFTX model. By increasing the layer 2 Kh, the model becomes more conceptually accurate by simulating horizontal flow within the Hawthorn aquifer system (HAS) (also more generally referred to as the intermediate aquifer system) and consequently improving model convergence and run time.

The HAS generally occurs as individual, thin, low permeability water bearing units within the intermediate confining unit, between the surficial aquifer (above) and the Upper Floridan aquifer system (below). The HAS exists within an approximate 5,000 square-mile area of DeSoto, Sarasota, Hardee, Manatee, and parts of Charlotte, Hillsborough, Highlands, and in the southwest portion of Polk County within the CFWI region. There are two main water producing aquifers within the HAS, the Upper Arcadia aquifer and Lower Arcadia aquifer. The lateral continuity and water-bearing potential of the zones within the HAS are highly variable due to a mixture of shell, sand, gravel, dolomite, and thin limestone beds that are interbedded within a clay matrix. This heterogeneous sequence often leads to low permeability of the water bearing zones and complicates mapping the lateral extent of each zone (Basso and Hood, 2005).

Combined groundwater withdrawals from both aquifers were approximately 58 million gallons per day (mgd) in 2006 with roughly 3 mgd occurring within the CFWI region. In 2018, there were groundwater withdrawals of 48 mgd from both aquifers with only about 3.2 mgd occurring within the CFWI region. Due to the unknown extent, water bearing limitations, limited water use, and minimal projected future demands for the HAS within the CFWI region, the HAT determined not to simulate individual aquifers within the HAS as part of the ECFTX model.

Initial Kh values for the intermediate confining unit (layer 2) were derived from vertical hydraulic conductivity (Kv) values calculated from aquifer performance tests (Figure 1). An anisotropy ratio was applied from the Kv value to the Kh (Kh:Kv) of 10:1. After further review, the HAT decided to pursue a more conceptually accurate Kh value to allow horizontal flow through the HAS while

maintaining the vertical head differences and fluxes between the surficial aquifer and Upper Floridan aquifer. The revised values of the horizontal hydraulic conductivity were modified from the District Wide Regulatory Model (DWRM3) model developed by Southwest Florida Water Management District (Environmental Simulations, Inc., 2014). The DWRM3 model simulates both aquifers and the confining units above and below the aquifers. Figure 2 shows the modified layer 2 hydraulic conductivity values.

The ECFTX model was re-run with the same transient calibration configuration except for the changes to layer 2 Kh. This modification resulted in very minor changes to the simulated heads of the overlying surficial aquifer and underlying Upper Floridan aquifer. Figures 3 and 4 show simulated head change with the increased layer Kh values in the surficial aquifer and Upper Floridan aquifer, respectively. The change of Layer 2 Kh did not cause significant change to the model calibration statistics of the target monitoring wells as given in Tables 1 and 2. There was a very minor change of -0.1% to simulated spring flow of Lithia Spring and essentially no change on other target springs (Table 3). The summary water budgets in Table 4 and 5 demonstrate that the HAS Kh modification did not cause significant changes to mass balance. A detailed breakdown of general head boundary (GHB) fluxes in Table 6 show that the boundary fluxes were essentially the same except very minor change to the layer 2 (HAS) of southern boundary. Additionally, the modified Layer 2 Kh values improved numerical convergence encountered in layer 2 and shortened model run times.

Table 1. Transient model calibration statistics of the target monitoring wells in the ECCTX Model domain comparing February 2020 calibration versus that with HAS Kh change.

	February 2020 calibration			HAS Kh change calibration		
	SA	UFA	LFA	SA	UFA	LFA
Residual Mean	-0.46	0.46	0.46	-0.47	0.45	0.46
Error Standard Dev	4.24	4.7	3.33	4.26	4.69	3.33
5% of Observation Range	8.97	7.59	2.79	8.97	7.59	2.79
Absolute Residual Mean	2.83	3.78	2.65	2.84	3.78	2.65
Error Sum of Squares	18156	20666	329	18333	20620	329
RMS Error	4.27	4.72	3.31	4.29	4.71	3.31
Minimum Residual	-31.65	-22.1	-10.19	-31.69	-22.1	-10.19
Maximum Residual	21.15	19.14	5.73	21.15	19.14	5.73
Number of Observations	997	928	30	997	928	30
Percentage with MAE < 2.5 ft	68%	48%	60%	68%	48%	60%
Percentage with MAE < 5.0 ft	88%	76%	87%	88%	76%	87%
Percentage with R2 > 0.4	78%	93%	93%	78%	93%	93%

All values in feet except as noted. Calibration period is 2004-2012. Mean error expressed as simulated minus observed

Table 2. Transient model calibration statistics of the target monitoring wells in the CFWI Area comparing February 2020 calibration versus HAS Kh change calibration.

	February 2020 calibration			HAS Kh change calibration		
	SA	UFA	LFA	SA	UFA	LFA
Residual Mean	-0.64	0.34	1.23	-0.65	0.32	1.23
Error Standard Dev	3.47	3.75	2.68	3.48	3.74	2.68
5% of Observation Range	8.6	6.2	2.62	8.6	6.2	2.62
Absolute Residual Mean	2.61	3.24	2.48	2.62	3.24	2.49
Error Sum of Squares	3442	2729	202	3465	2714	202
RMS Error	3.53	3.75	2.9	3.54	3.74	2.9
Minimum Residual	-16.51	-11.93	-5.46	-16.51	-11.93	-5.49
Maximum Residual	13.29	10.11	5.73	13.28	10.08	5.73
Number of Observations	277	194	24	277	194	24
Percentage with MAE < 2.5 ft	71%	52%	58%	71%	52%	58%
Percentage with MAE < 5.0 ft	87%	85%	88%	87%	85%	88%
Percentage with R2 > 0.4	78%	96%	92%	78%	96%	92%

All values in feet except as noted. Calibration period is 2004-2012. Mean error expressed as simulated minus observed

Table 3. Transient model calibration statistics of the target springs simulated in the ECFTX model.

Spring Name	Observed Flux (cfs)	Simulated Flux (cfs)	
		Original	Modified
Lithia Spring Major	34.7	33.2	33.1
Buckhorn Main Spring	12.2	12.1	12.1
Sulphur Spring (Hillsborough)	34.7	35.4	35.4
Crystal Main Spring (Pasco)	45.5	46.4	46.4
Weeki Wachee Spring	160.4	167.3	167.3
Chassahowitzka Spring Main	59.6	59.3	59.3
Homosassa Spring #1	83.5	84.5	84.5
Gum Spring Main	63.8	64.8	64.8
Rainbow Spring #1	71.8	73.3	73.3
Apopka Spring	24.9	24.8	24.8
Sanlando Springs	18.8	19.9	19.9
Starbuck Spring	12.1	12.6	12.6
Wekiwa Spring (Orange)	61.0	64.6	64.6
Bugg Spring (Lake)	10.6	9.7	9.7
Rock Springs (Orange)	54.9	51.6	51.6
Volusia Blue Spring	143.6	132.4	132.4
Alexander Spring	100.1	98.9	98.9

Table 4. Annual average boundary condition influx (in/yr) in the ECCTX transient model (2003-2014)

Layer	Constant Head	GHB	Well	River	Rech	ET	Spring	Drain Return	Drain	Storage
In/yr										
February 2020 calibration (calib0)										
1	0.22	0.041	0.085	3.0	22.1	-	-	-	-	0.18
2	-	0.25	-	-	-	-	-	-	-	-
3	-	0.47	4E-4	-	-	-	-	0.059	-	-
4	-	0.23	2E-6	-	-	-	-	-	-	-
5	-	0.56	-	-	-	-	-	-	-	-
6	-	0.096	-	-	-	-	-	-	-	-
7	-	0.080	-	-	-	-	-	-	-	-
8	-	0.060	-	-	-	-	-	-	-	-
9	-	0.22	-	-	-	-	-	-	-	-
10	-	-	-	-	-	-	-	-	-	-
11	-	0.11	-	-	-	-	-	-	-	-
HAS Kh change calibration (calibHAS)										
1	0.22	0.041	0.085	3.0	22.1	-	-	-	-	0.18
2	-	0.25	-	-	-	-	-	-	-	-
3	-	0.47	4E-4	-	-	-	-	0.059	-	-
4	-	0.23	2E-6	-	-	-	-	-	-	-
5	-	0.56	-	-	-	-	-	-	-	-
6	-	0.096	-	-	-	-	-	-	-	-
7	-	0.080	-	-	-	-	-	-	-	-
8	-	0.060	-	-	-	-	-	-	-	-
9	-	0.22	-	-	-	-	-	-	-	-
10	-	-	-	-	-	-	-	-	-	-
11	-	0.11	-	-	-	-	-	-	-	-

Table 5. Annual average boundary condition outflux (in/yr) in ECFTX transient model (2003-2014)

Layer	Constant Head	GHB	Well	River	Rech	ET	Spring	Drain Return	Drain	Storage
In/yr										
February 2020 calibration (calib0)										
1	-2.6	-0.022	-0.17	-3.9	-	-13.1	-	-	-3.5	-
2	-	-0.028	-0.021	-	-	-	-	-	-	-
3	-	-0.46	-0.62	-	-	-	-1.3	-	-	-
4	-	-0.11	-0.30	-	-	-	-	-	-	-
5	-	-0.064	-0.58	-	-	-	-7E-3	-	-	-
6	-	-6E-5	-2E-4	-	-	-	-	-	-	-
7	-	-8E-4	-1E-4	-	-	-	-	-	-	-
8	-	-5E-5	-9E-4	-	-	-	-	-	-	-
9	-	-0.18	-0.30	-	-	-	-	-	-	-
10	-	-	-	-	-	-	-	-	-	-
11	-	-0.57	-	-	-	-	-	-	-	-
HAS Kh change calibration (calibHAS)										
1	-2.6	-0.022	-0.17	-3.9	-	-13.1	-	-	-3.5	-
2	-	-0.028	-0.021	-	-	-	-	-	-	-
3	-	-0.46	-0.62	-	-	-	-1.3	-	-	-
4	-	-0.11	-0.30	-	-	-	-	-	-	-
5	-	-0.064	-0.58	-	-	-	-7E-3	-	-	-
6	-	-6E-5	-2E-4	-	-	-	-	-	-	-
7	-	-8E-4	-1E-4	-	-	-	-	-	-	-
8	-	-5E-5	-9E-4	-	-	-	-	-	-	-
9	-	-0.18	-0.30	-	-	-	-	-	-	-
10	-	-	-	-	-	-	-	-	-	-
11	-	-0.57	-	-	-	-	-	-	-	-

Table 6. Annual average general head boundary (GHB) condition flux (mgd) in the ECCTX transient model (2003-2014).

Layer	GHB North		GHB South		GHB East		GHB West		Total	
	influx	outflux	influx	outflux	influx	outflux	influx	outflux	influx	outflux
mgd										
February 2020 calibration (calib0)										
1	14	-3.0	21	-16	-	-	-	-	35	-19
2	195	-29	0.12	-0.37	0.06	-1E-4	74	-0.19	268	-30
3	439	-221	28	-54	16	-83	3.6	-125	486	-482
4	126	-54	9.8	-21	9.8	-0.63	90	-37	236	-112
5	98	-8.1	292	-1.4	84	-30	87	-24	561	-64
6	1.3	-0.04	70	-7E-3	-	-	14	-	85	-0.05
7	1.4	-0.63	11	-9E-5	37	-0.03	20	-	68	-0.67
8	5E-3	-0.03	2.5	-	34	-6E-4	-	-	36	-0.03
9	50	-5.5	-	-	41	-69	6.2	-6.7	97	-82
10	-	-	-	-	-	-	-	-	-	-
11	2.4	-1.1	-	-	25	-155	5.2	-17	33	-172
HAS Kh change calibration (calibHAS)										
1	14	-3.0	21	-16	-	-	-	-	35	-19
2	195	-29	0.08	-0.43	0.06	-1E-4	74	-0.19	268	-30
3	439	-221	28	-54	16	-83	3.6	-125	486	-482
4	126	-54	9.7	-21	10	-0.63	90	-37	236	-112
5	98	-8.1	292	-1.5	84	-30	87	-24	561	-64
6	1.3	-0.04	70	-7E-3	-	-	14	-	85	-0.05
7	1.4	-0.63	11	-9E-5	37	-0.03	20	-	68	-0.67
8	5E-3	-0.03	2.5	-	34	-6E-4	-	-	36	-0.03
9	50	-5.5	-	-	41	-69	6.2	-6.7	97	-82
10	-	-	-	-	-	-	-	-	-	-
11	2.4	-1.1	-	-	25	-155	5.2	-17	33	-172

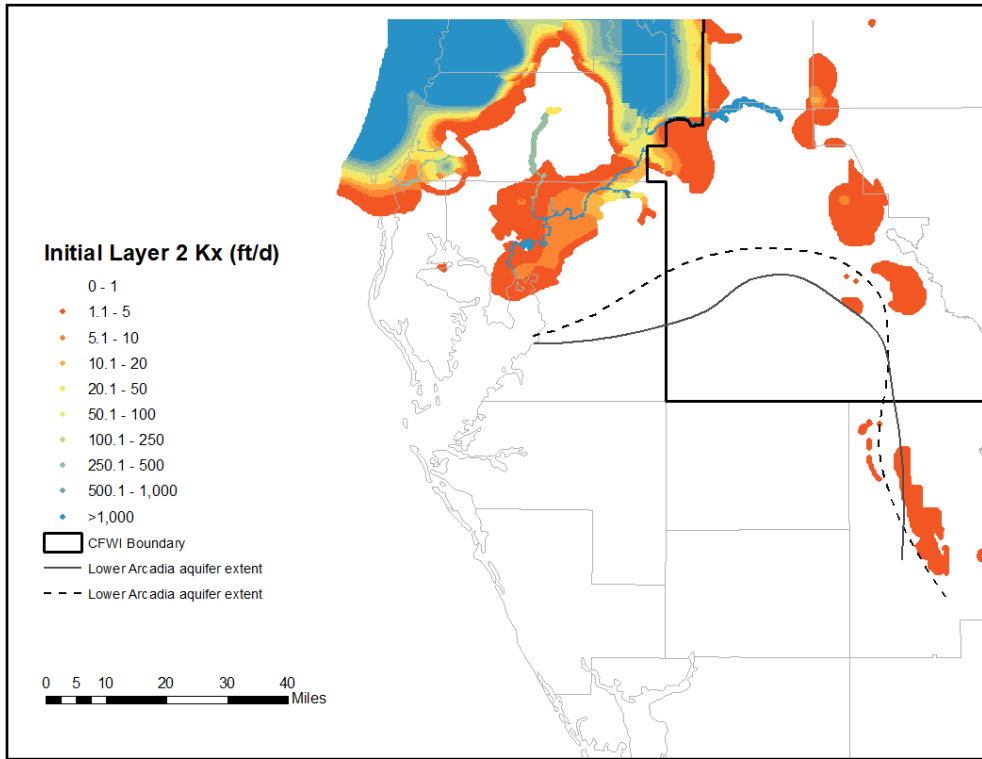


Figure 1. ECFTX Initial Layer 2 Kx Array

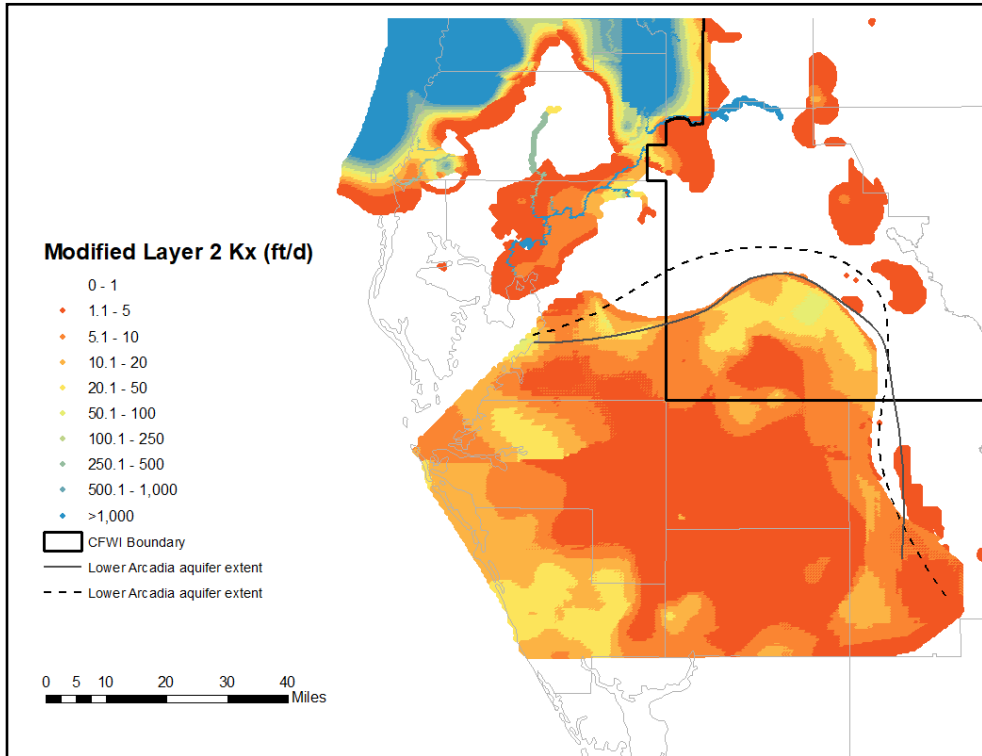


Figure 2. ECFTX Modified Layer 2 Kx Array

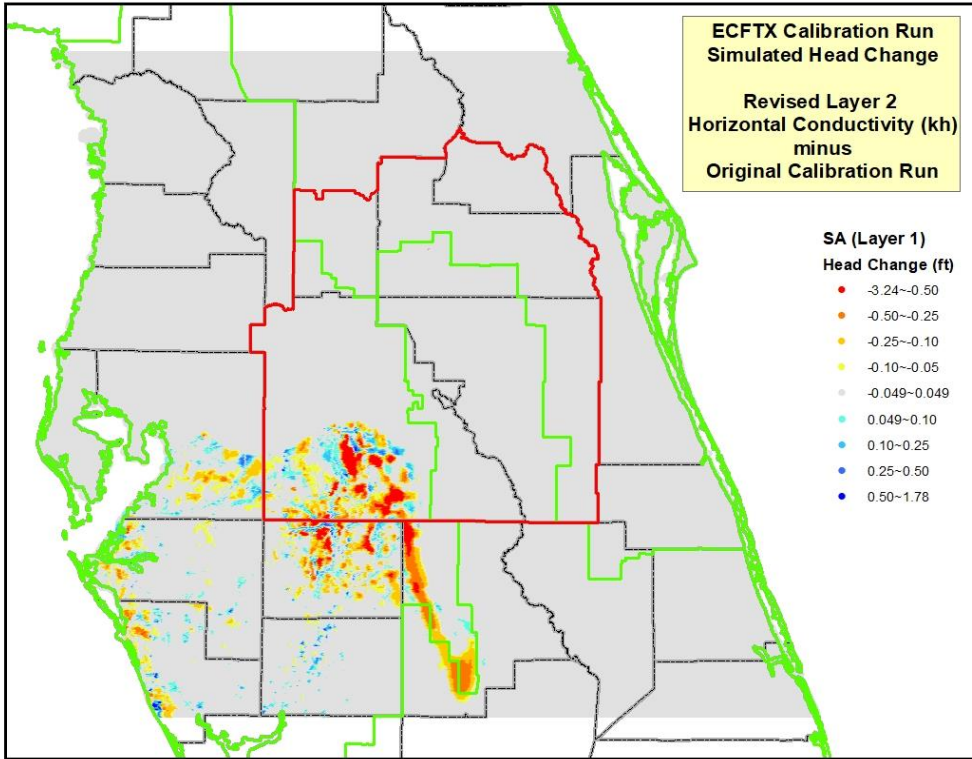


Figure 3. ECFTX Modified Layer 2 Kx Array

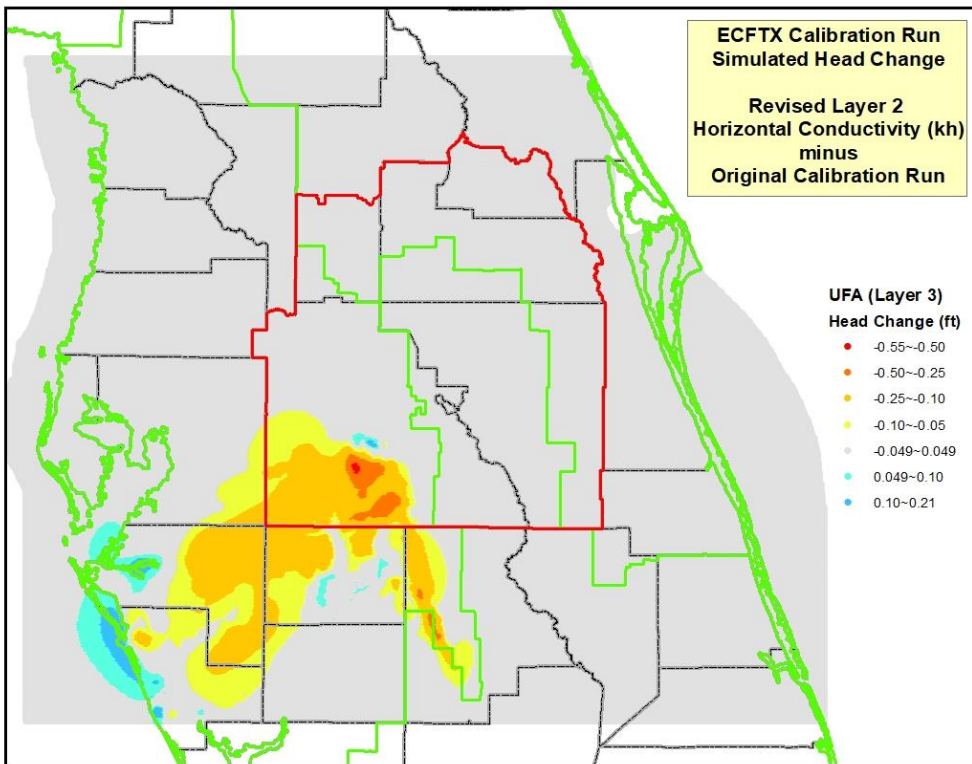


Figure 4. ECFTX Modified Layer 2 K

References

Basso, R. and J. Hood, 2005, Assessment of Minimum Aquifer Levels for the Intermediate Aquifer System in the Southwest Florida Water Management District, Southwest Florida Water Management District Technical Report, 217 p.

Central Florida Water Initiative (CFWI) Hydrologic Analysis Team (HAT), 2020. Model Documentation Report: East-Central Florida Transient Expanded (ECFTX) Model.

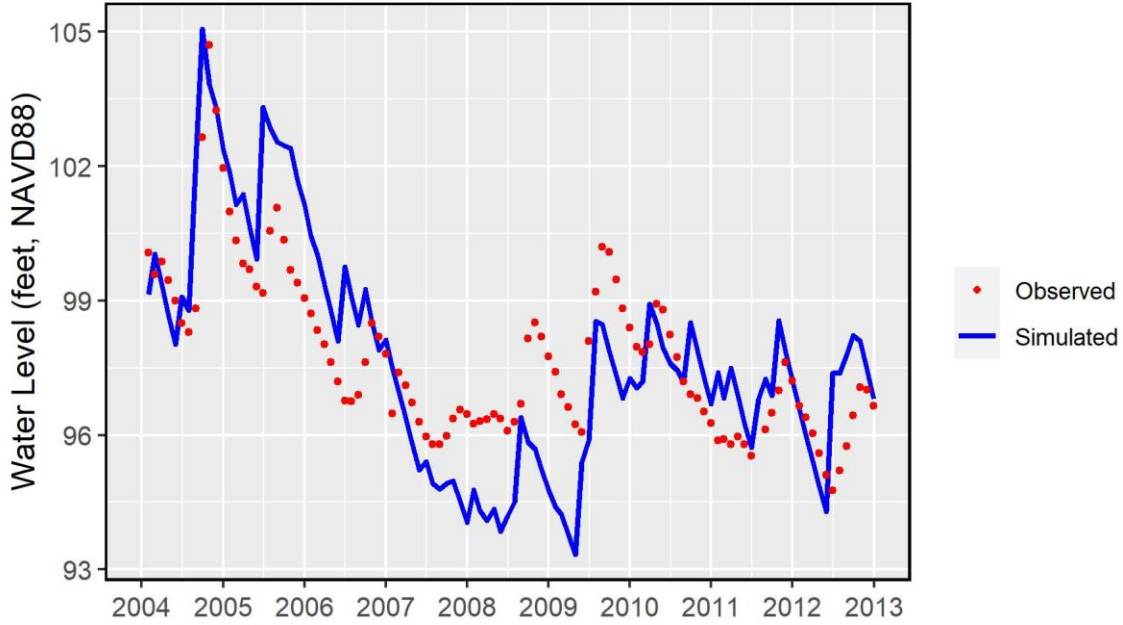
Environmental Simulations Inc. (ESI), 2014. Development and Calibration of the District Wide Regulation Model Version 3 for Southwest Florida Water Management District.

**Appendix B – Simulated Versus Observed Water Level Hydrographs
at Target Wells in Focus Area**

Surficial Aquifer (SA) Target Wells

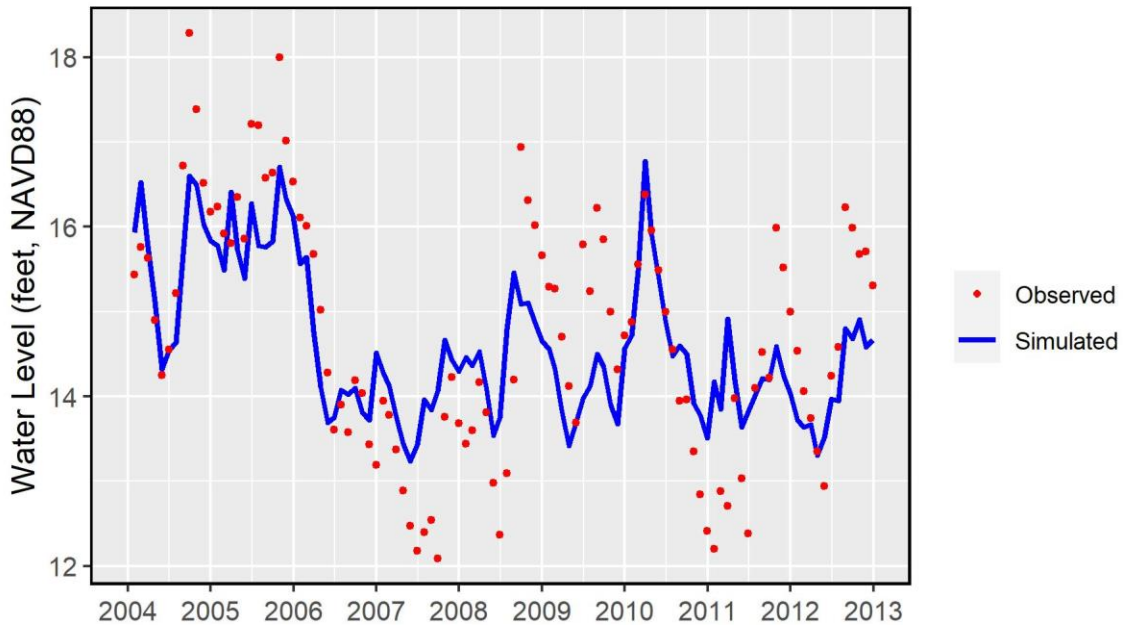
SA: TURLAK_GW1

ME=-0.025 MAE=1.339 nMAE=1.339 R2=0.614 NS=0.212



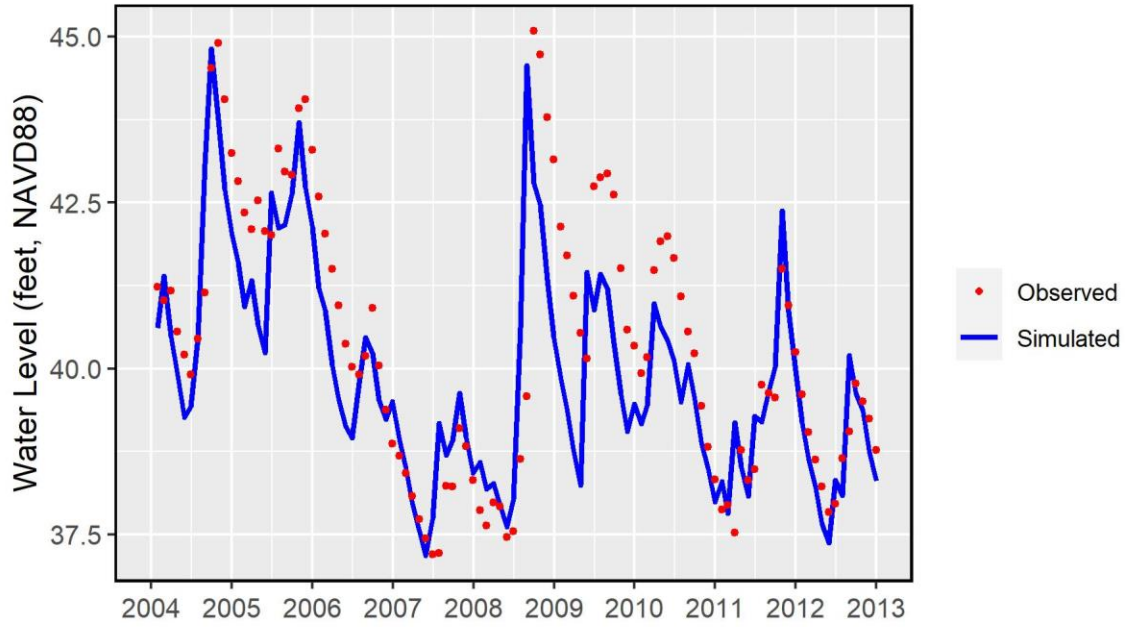
SA: SJRWMD01140472

ME=-0.105 MAE=0.742 nMAE=0.732 R2=0.629 NS=0.595



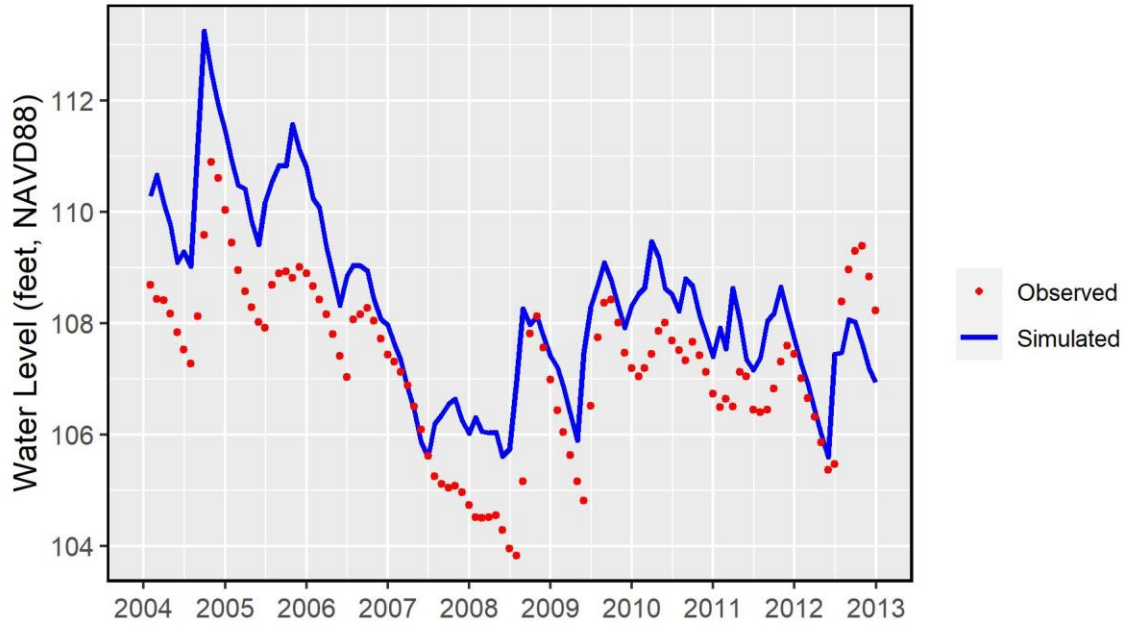
SA: SJRWMD01270536

ME=-0.464 MAE=0.93 nMAE=0.831 R2=0.687 NS=0.633



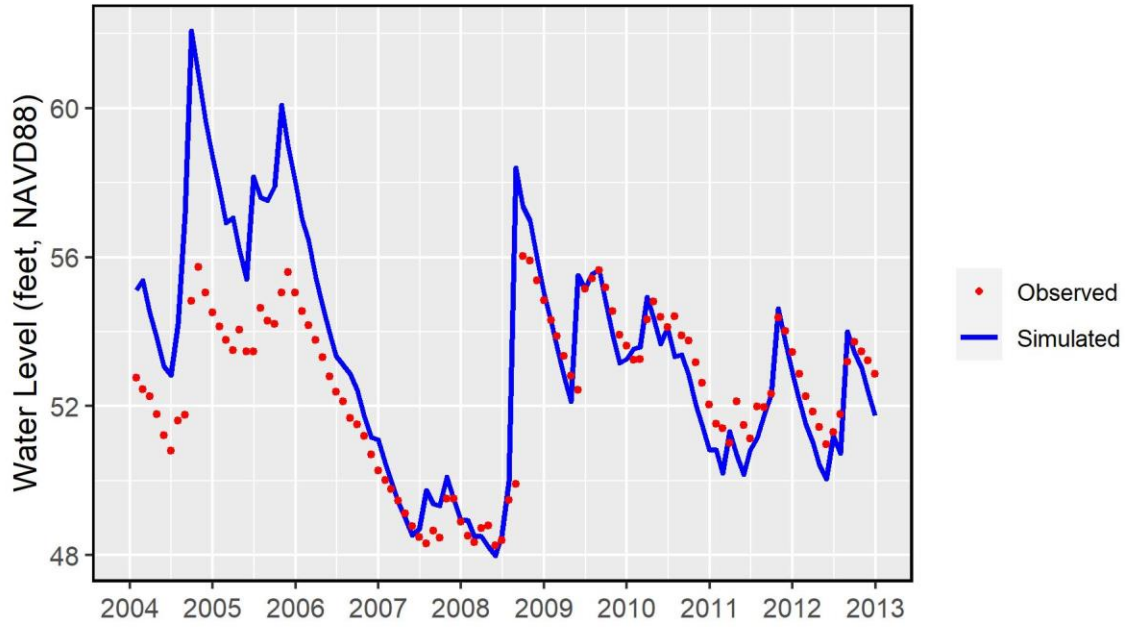
SA: SJRWMD01840092

ME=1.056 MAE=1.207 nMAE=0.73 R2=0.675 NS=0.065



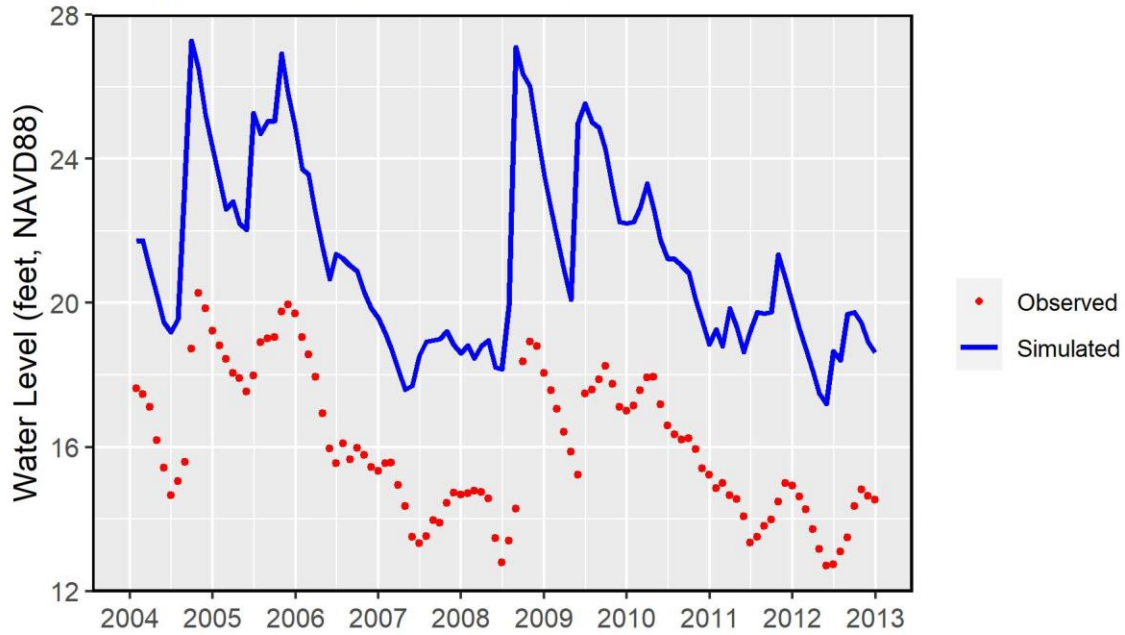
SA: SJRWMD01850095

ME=0.871 MAE=1.373 nMAE=1.454 R2=0.652 NS=0.035

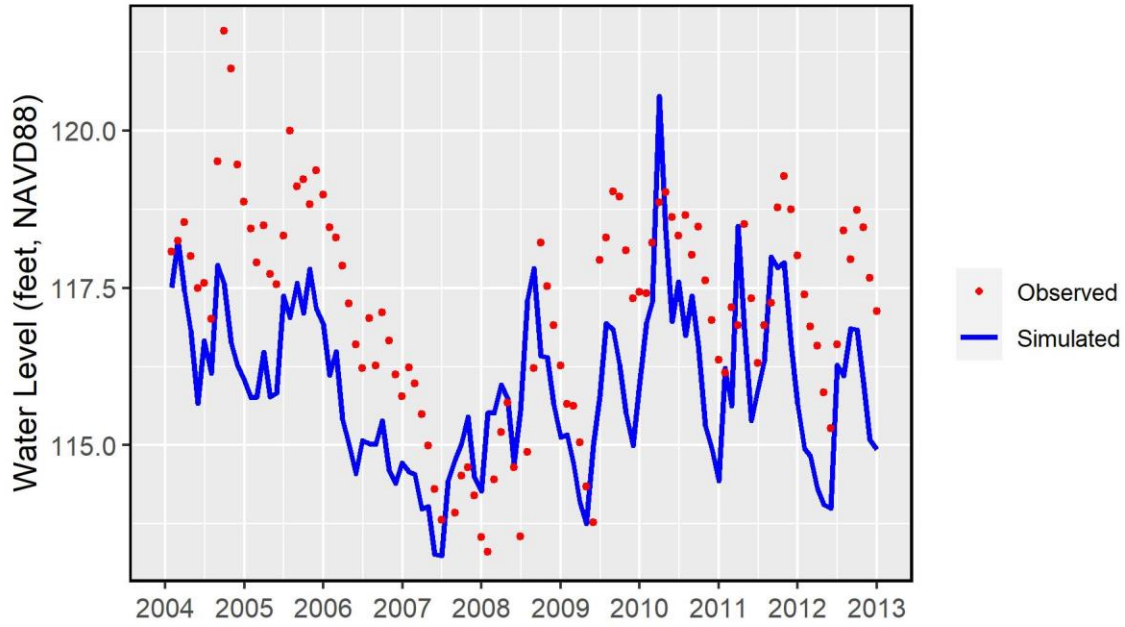


SA: SJRWMD02751562

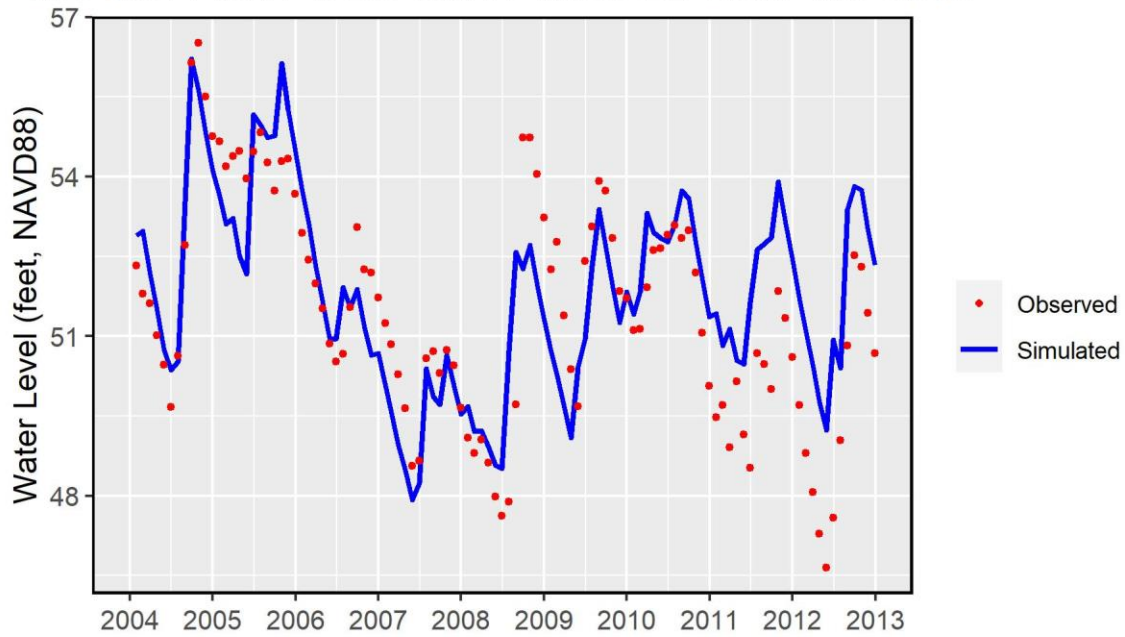
ME=5.183 MAE=5.183 nMAE=0.937 R2=0.724 NS=-6.504



SA: SJRWMD05170970
ME=-1.217 MAE=1.565 nMAE=0.98 R2=0.443 NS=-0.083

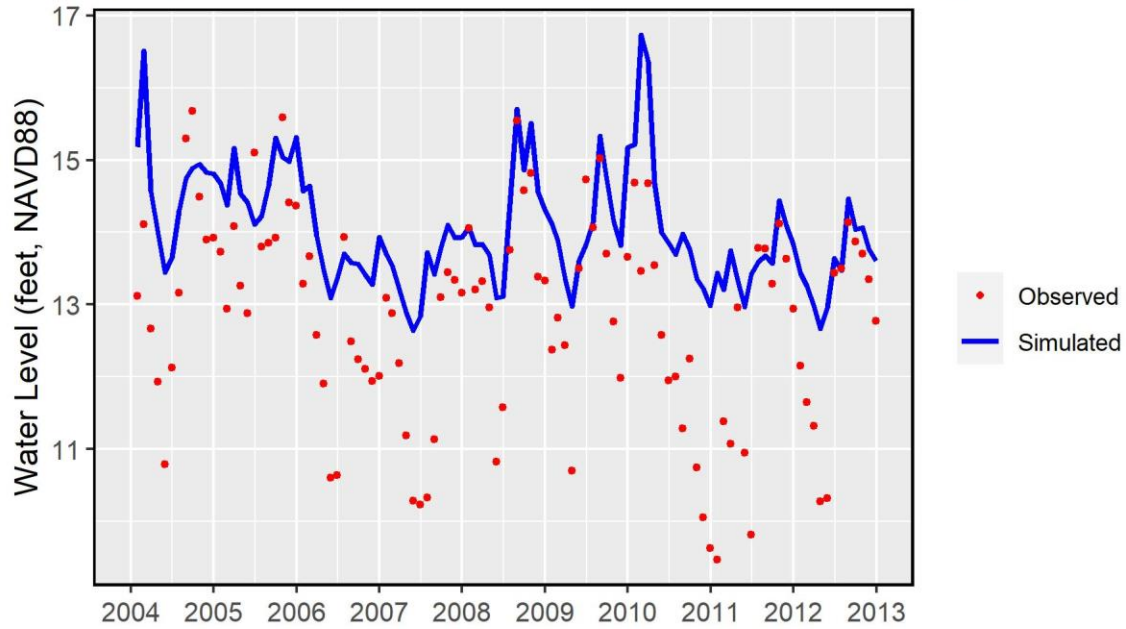


SA: SJRWMD05322143
ME=0.332 MAE=1.133 nMAE=1.088 R2=0.597 NS=0.563



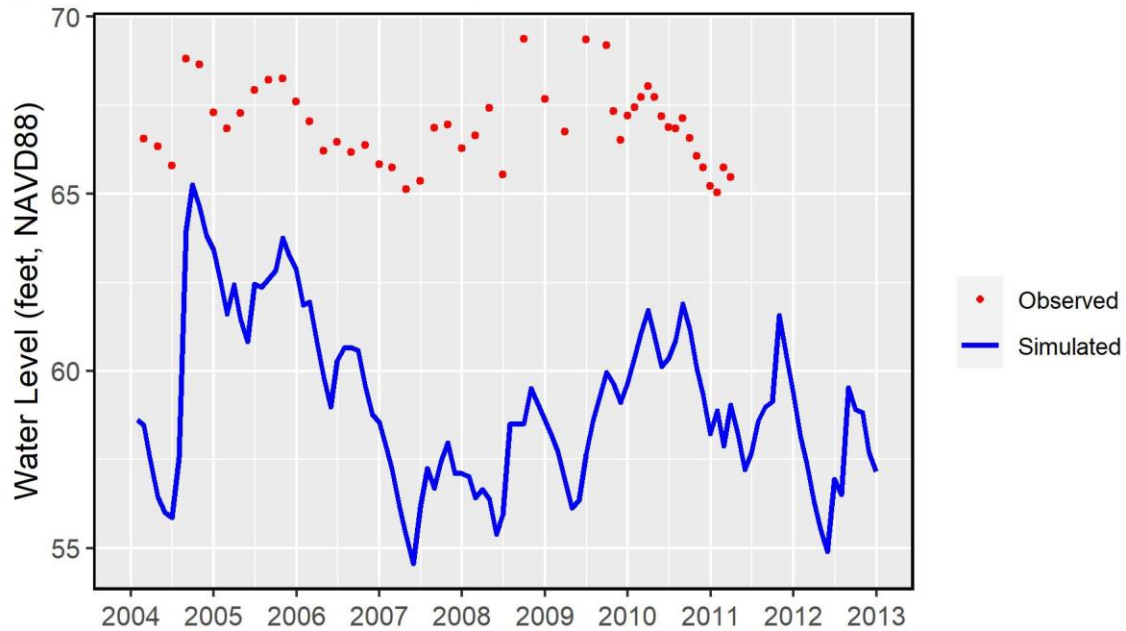
SA: SJRWMD05521047

ME=1.223 MAE=1.302 nMAE=0.799 R2=0.542 NS=-0.216



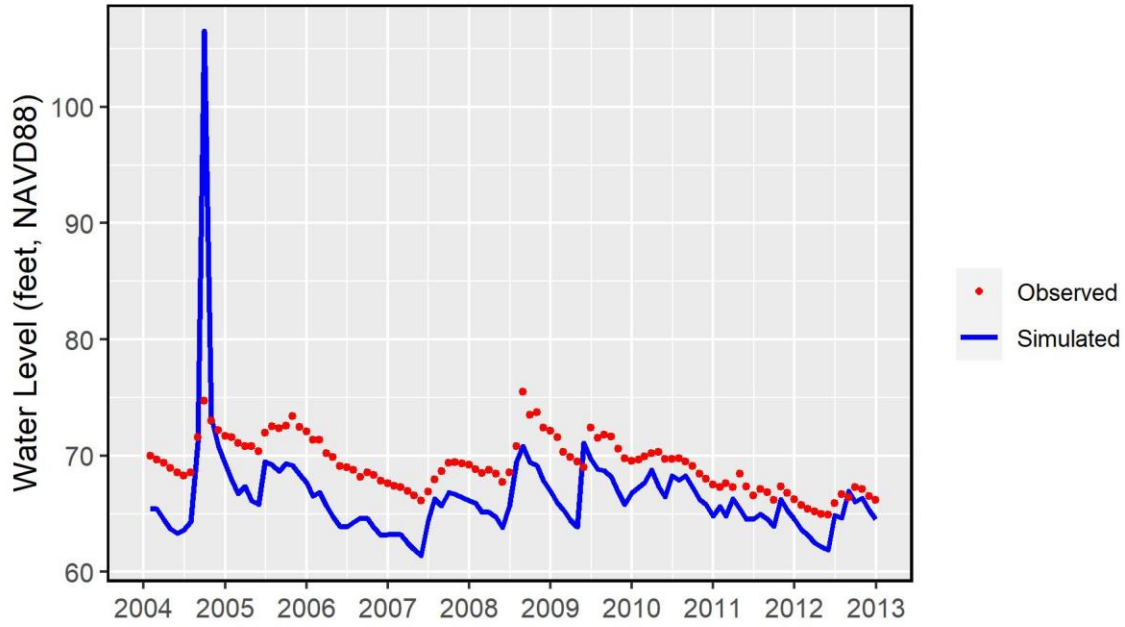
SA: SJRWMD05581055

ME=-7.322 MAE=7.322 nMAE=1.683 R2=0.278 NS=-47.658



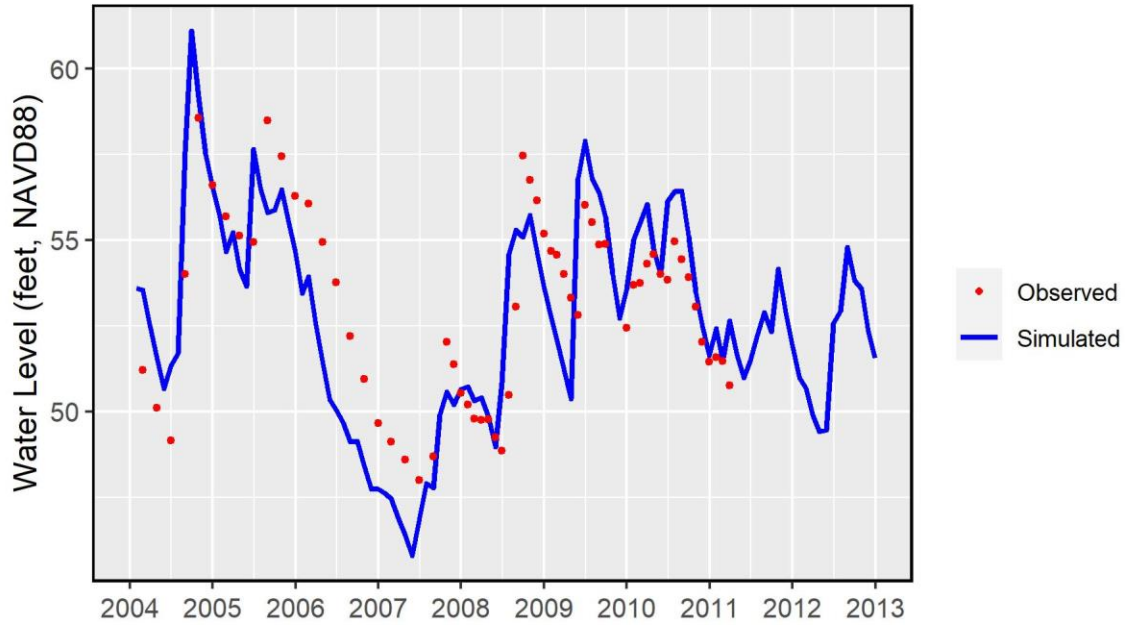
SA: SJRWMD05601059

ME=-2.814 MAE=3.459 nMAE=1.531 R2=0.347 NS=-3.178



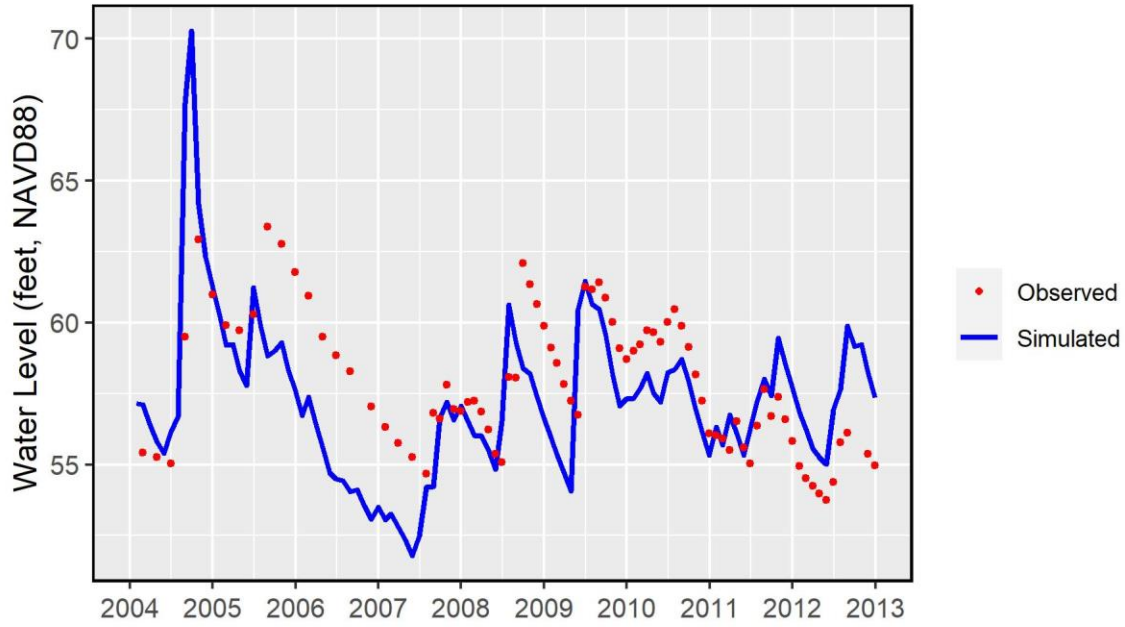
SA: SJRWMD05691074

ME=-0.004 MAE=1.597 nMAE=1.597 R2=0.622 NS=0.497



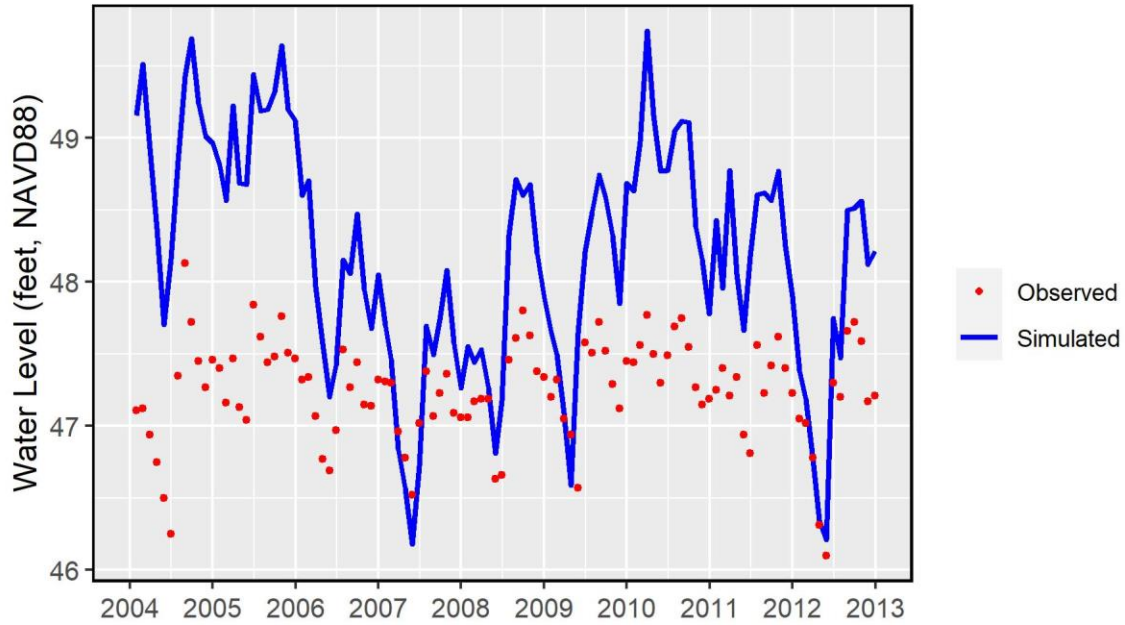
SA: SJRWMD05781085

ME=-0.602 MAE=1.91 nMAE=1.815 R2=0.294 NS=0.001



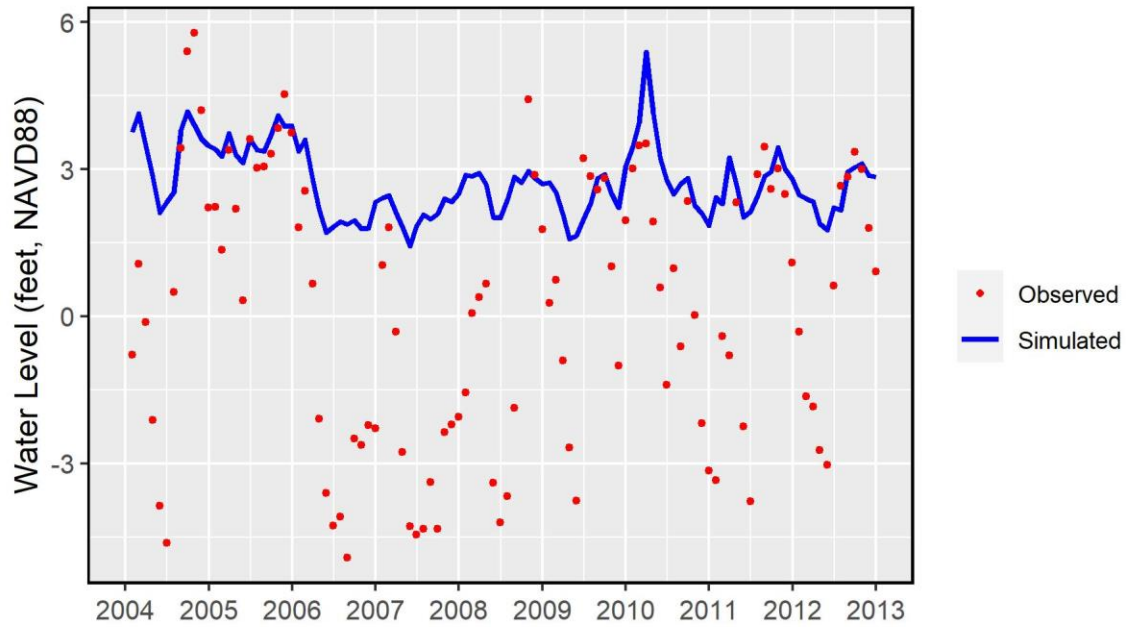
SA: SJRWMD09992686

ME=0.946 MAE=0.971 nMAE=0.503 R2=0.525 NS=-9.294



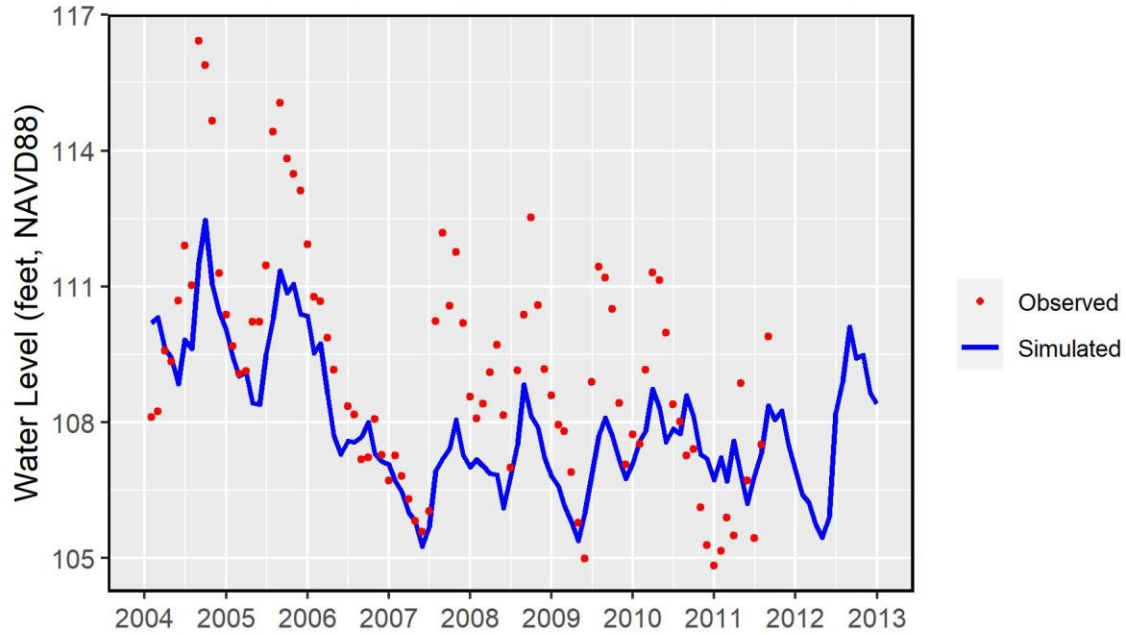
SA: SJRWMD10291655

ME=2.561 MAE=2.738 nMAE=1.993 R2=0.564 NS=-0.53

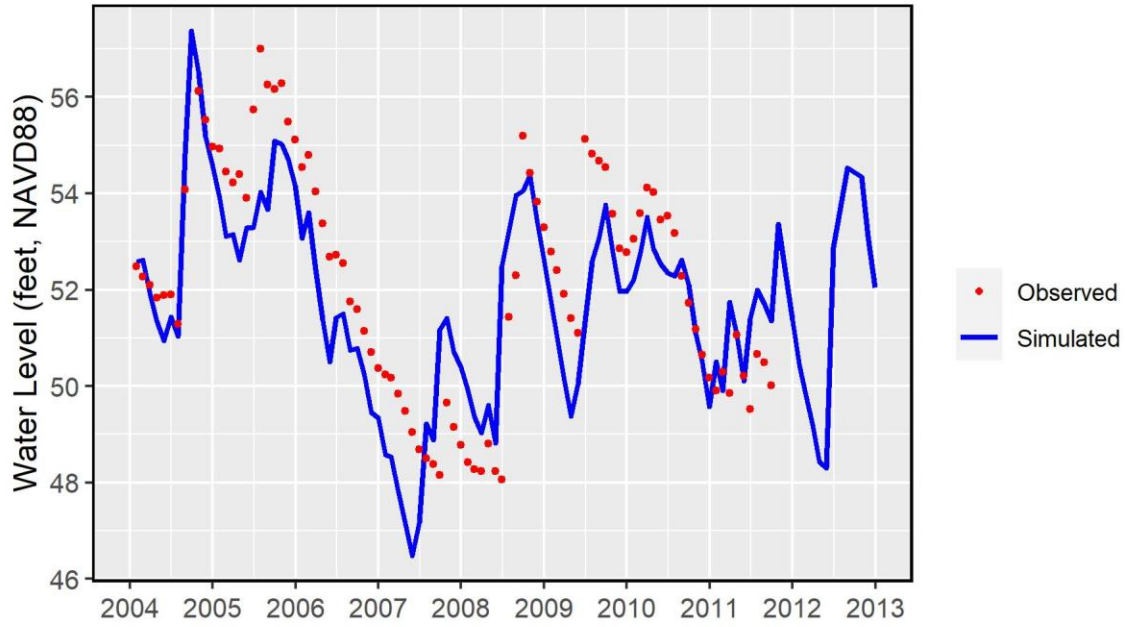


SA: SJRWMD11083691

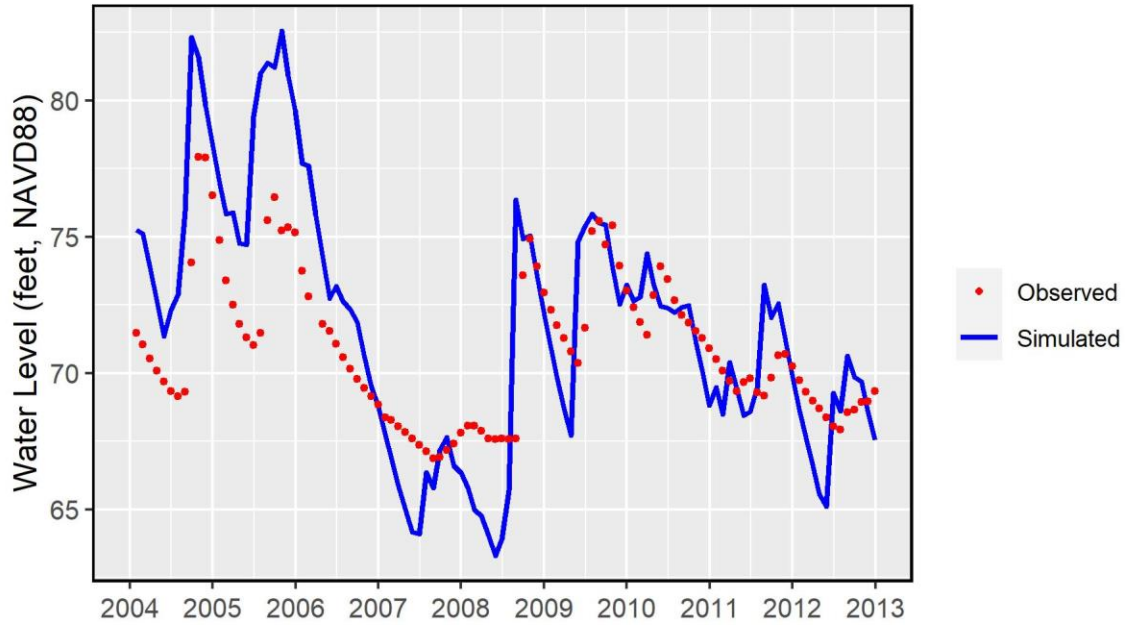
ME=-1.169 MAE=1.615 nMAE=1.321 R2=0.616 NS=0.371



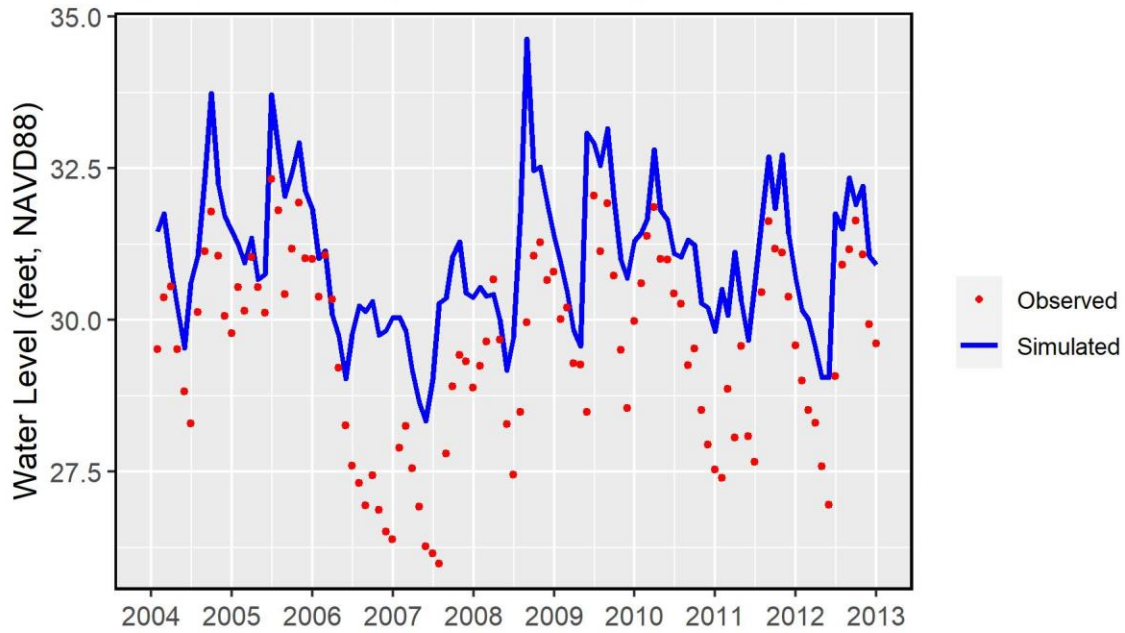
SA: SJRWMD11123689
ME=-0.483 MAE=1.186 nMAE=1.04 R2=0.67 NS=0.628



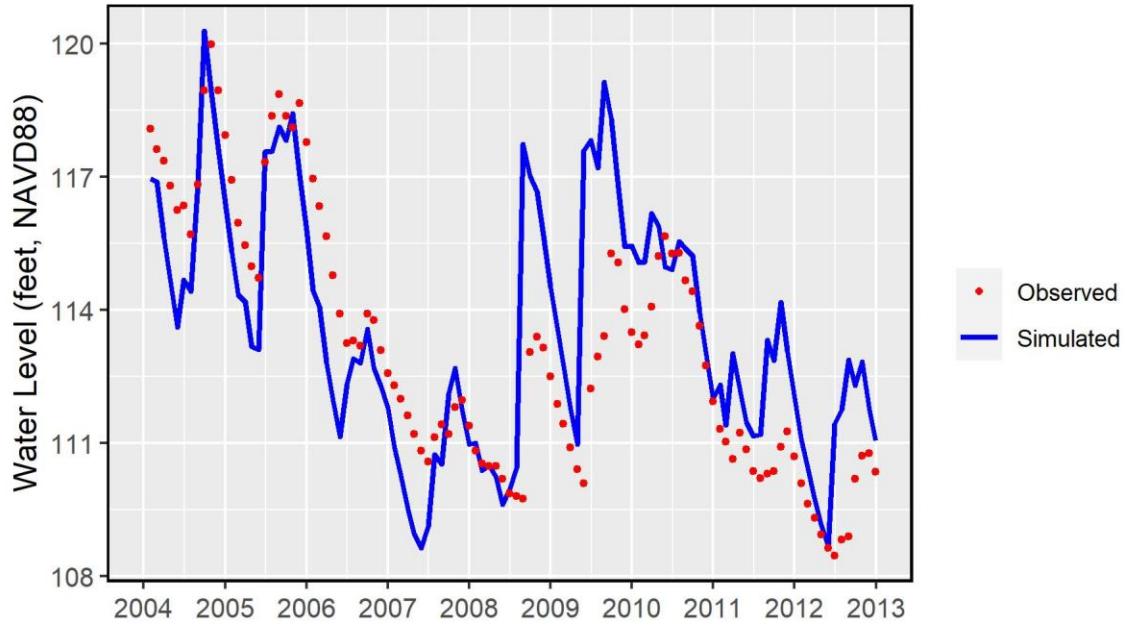
SA: SJRWMD11163694
ME=0.84 MAE=2.31 nMAE=2.309 R2=0.663 NS=-0.303



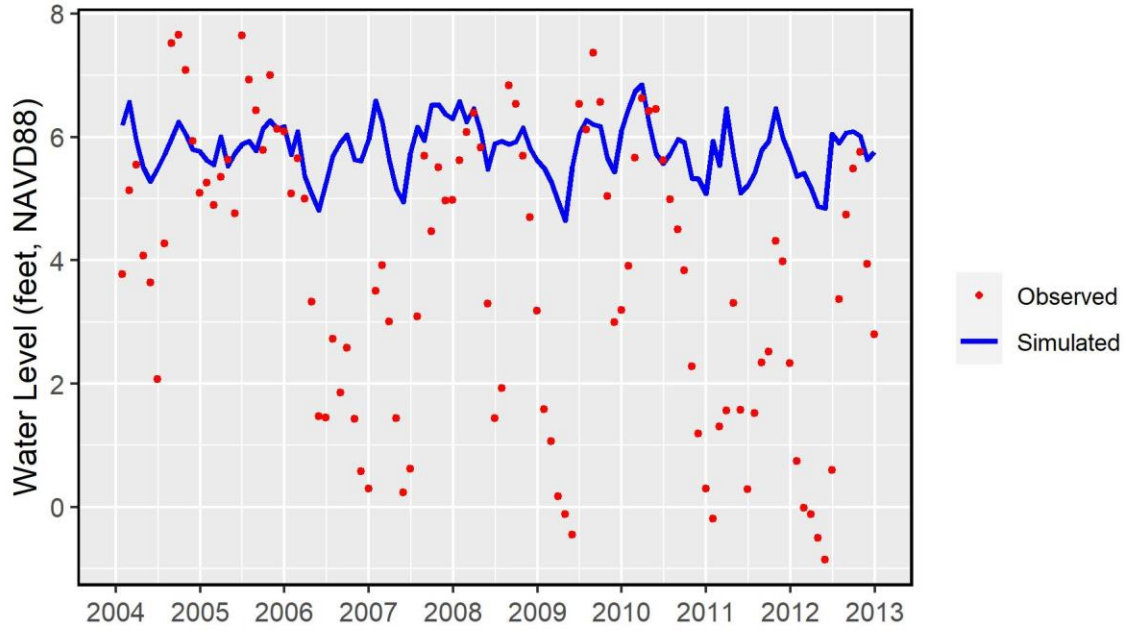
SA: SJRWMD11301672
ME=1.473 MAE=1.482 nMAE=0.743 R2=0.606 NS=-0.3



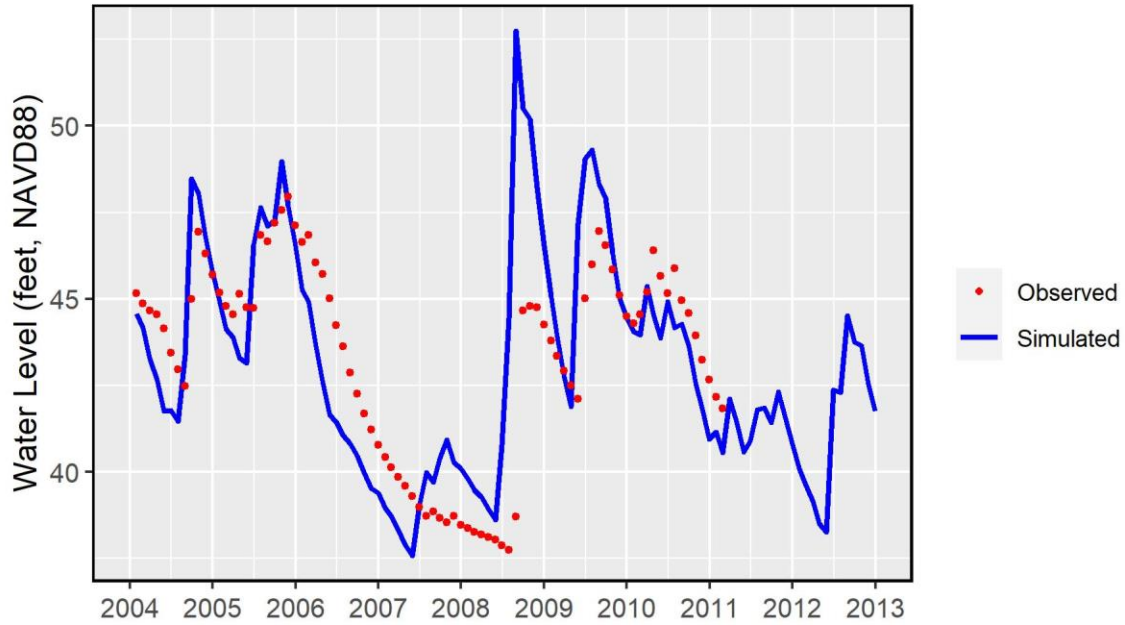
SA: SJRWMD11351677
ME=0.41 MAE=1.555 nMAE=1.556 R2=0.539 NS=0.475



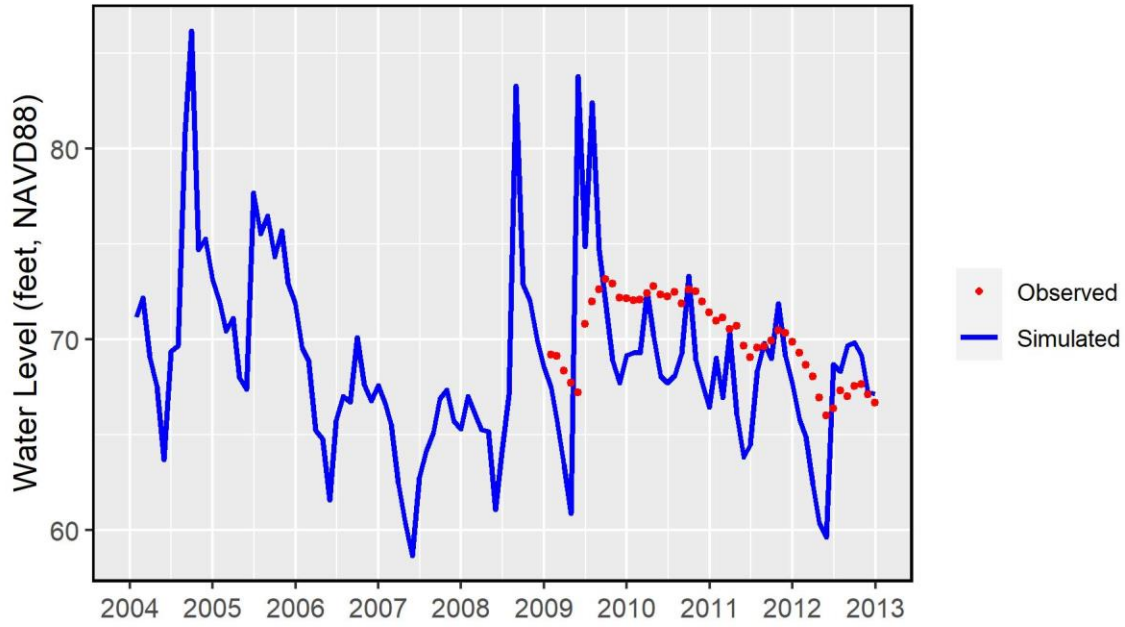
SA: SJRWMD11502179
ME=2.045 MAE=2.285 nMAE=1.76 R2=0.382 NS=-0.598



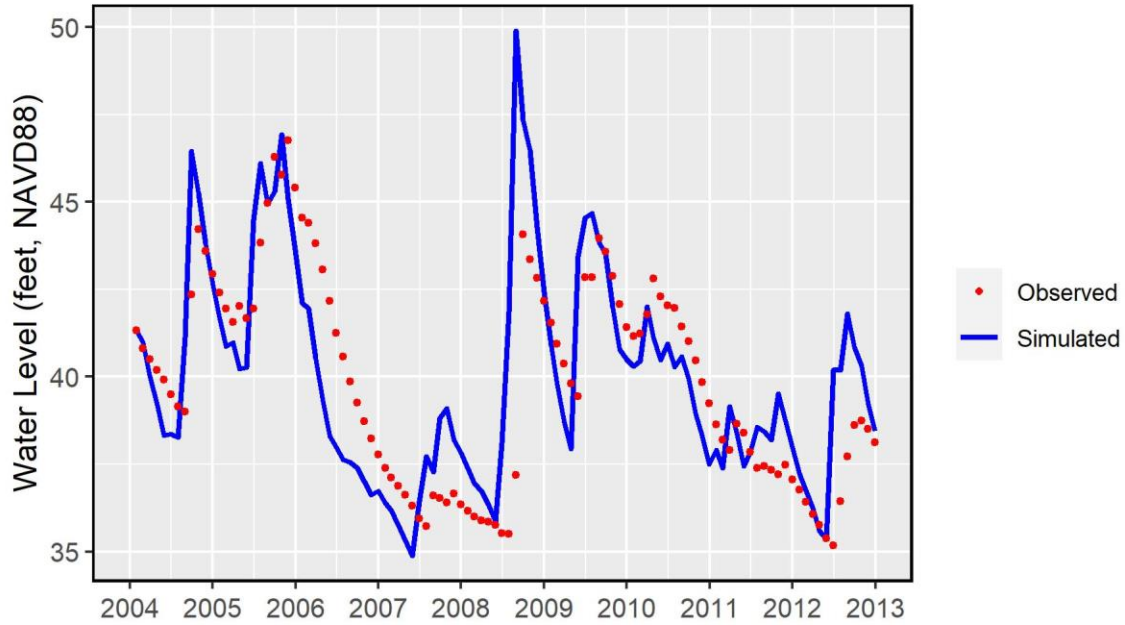
SA: SJRWMD11512185
ME=0.206 MAE=1.736 nMAE=1.77 R2=0.467 NS=0.233



SA: SJRWMD15282840
ME=-1.527 MAE=3.439 nMAE=2.987 R2=0.12 NS=-3.45

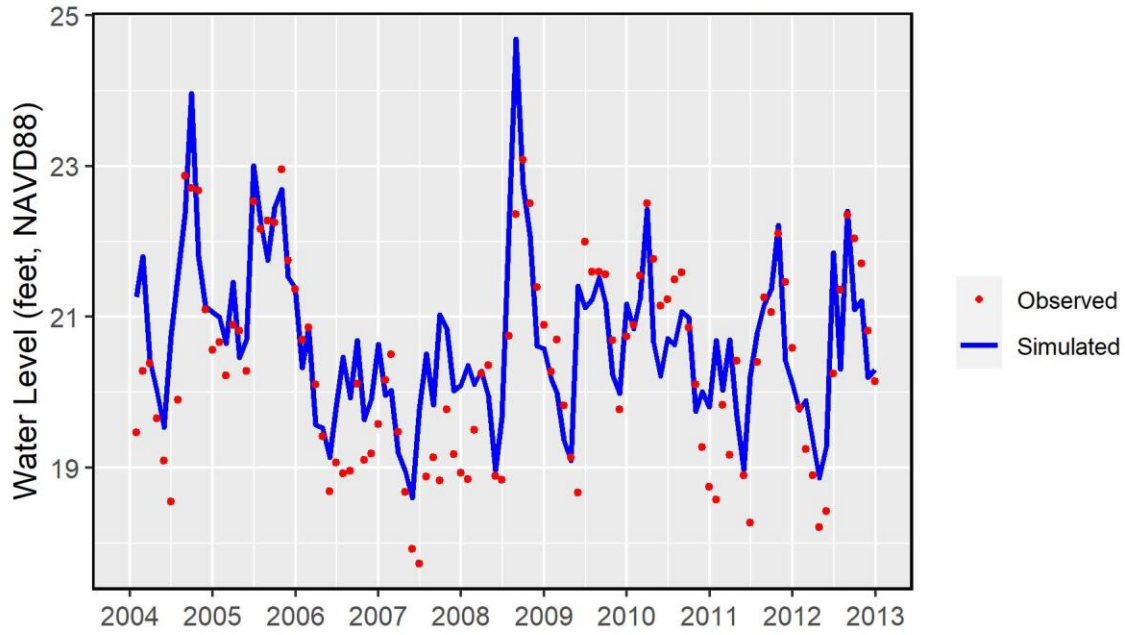


SA: SJRWMD17043379
ME=0.143 MAE=1.584 nMAE=1.601 R2=0.527 NS=0.428



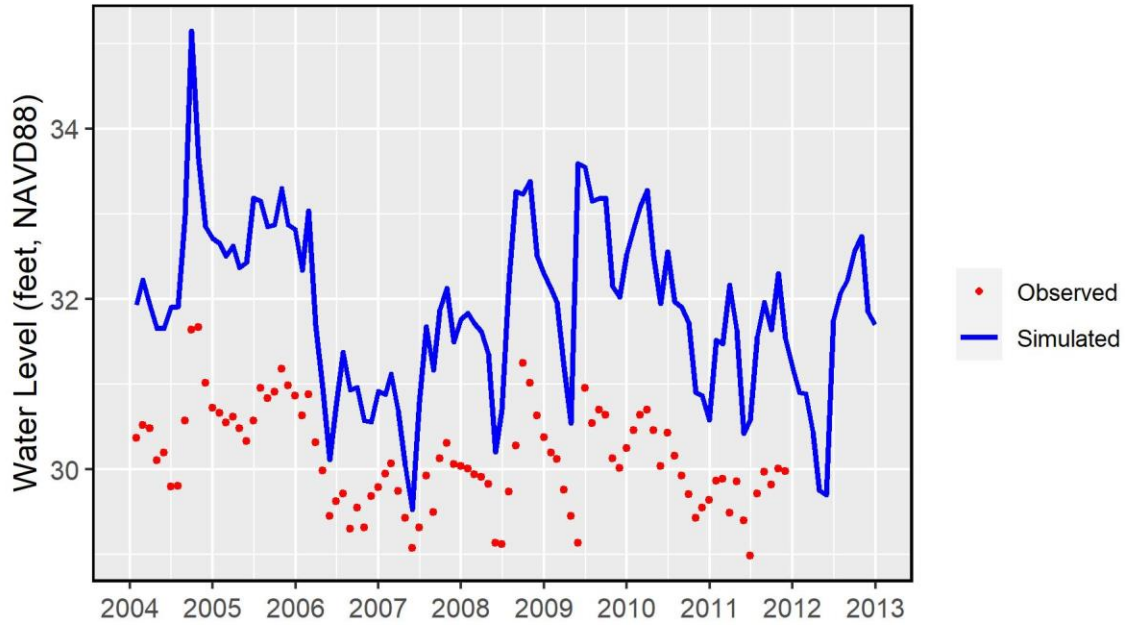
SA: SJRWMD17053380

ME=0.299 MAE=0.68 nMAE=0.679 R2=0.577 NS=0.52

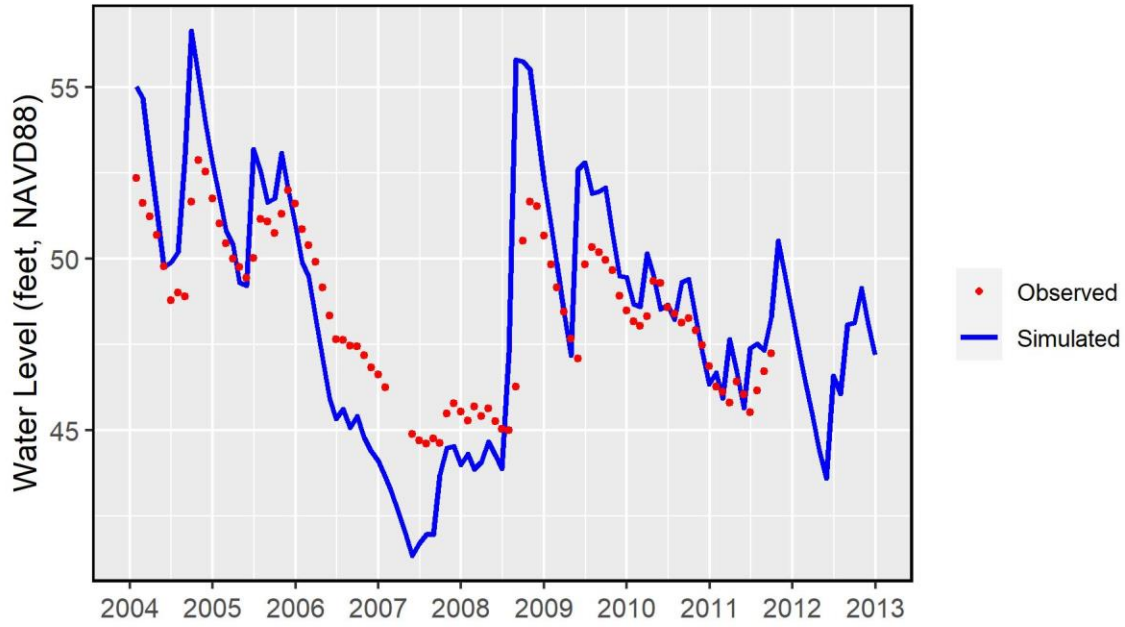


SA: SJRWMD17673592

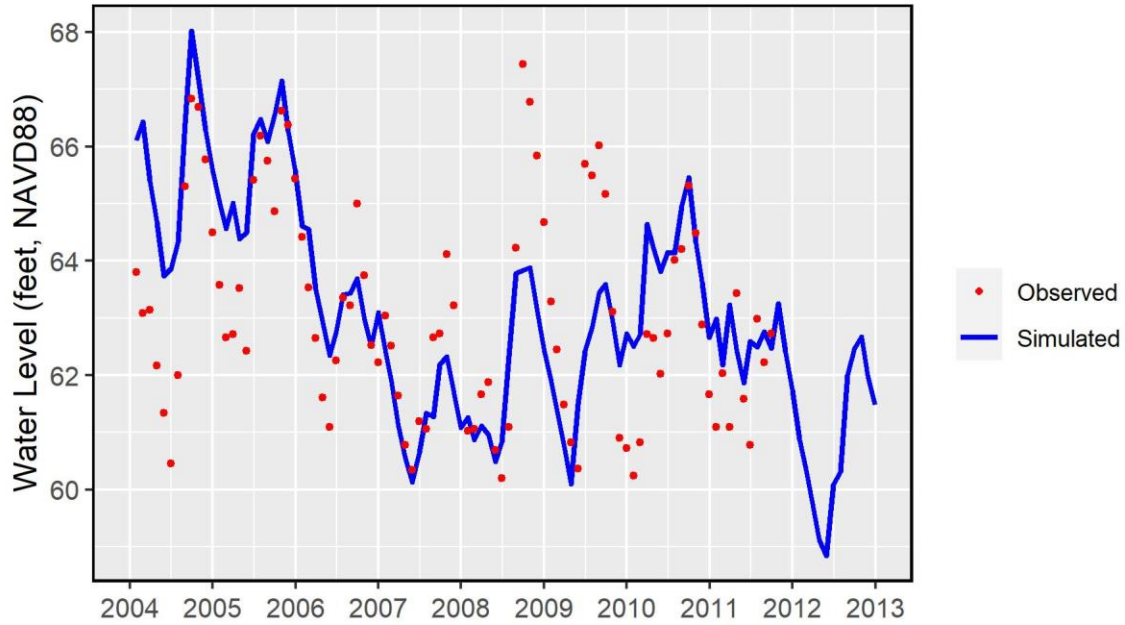
ME=1.821 MAE=1.821 nMAE=0.42 R2=0.718 NS=-10.174



SA: SJRWMD17853616
ME=0.396 MAE=1.613 nMAE=1.584 R2=0.728 NS=0.085

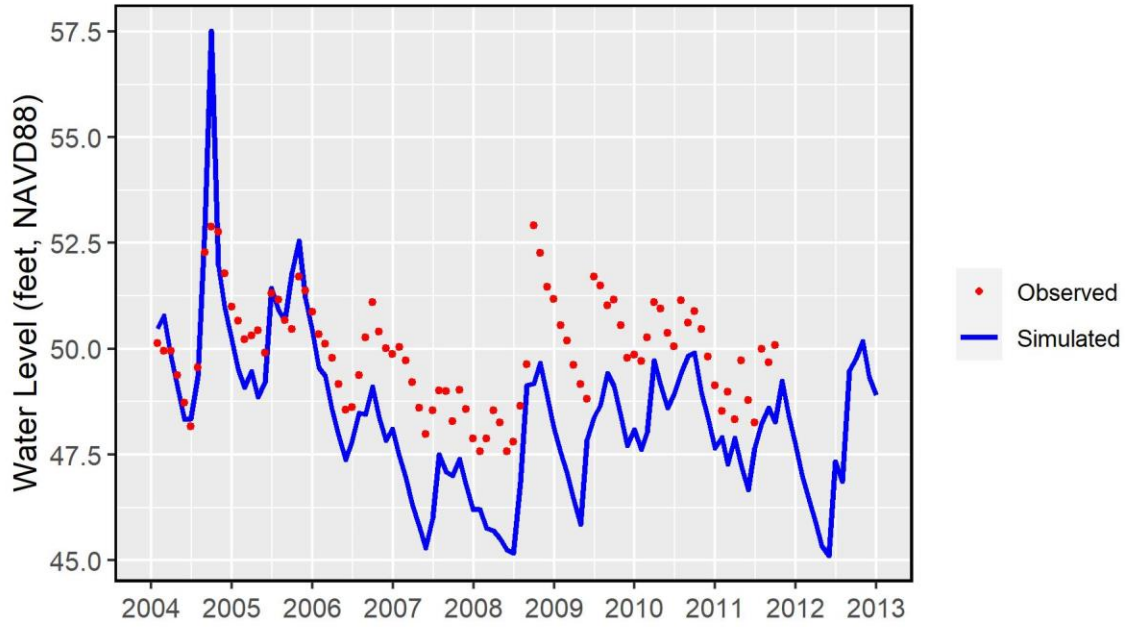


SA: SJRWMD18133697
ME=0.313 MAE=1.181 nMAE=1.142 R2=0.468 NS=0.358



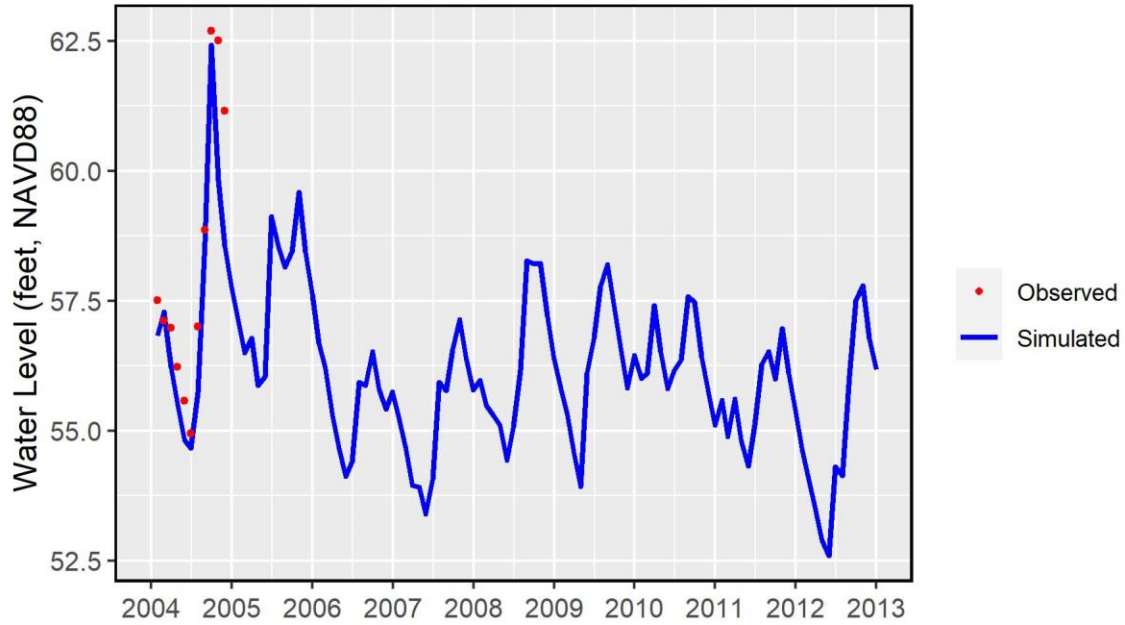
SA: SJRWMD18143698

ME=-1.421 MAE=1.613 nMAE=0.914 R2=0.604 NS=-1.312



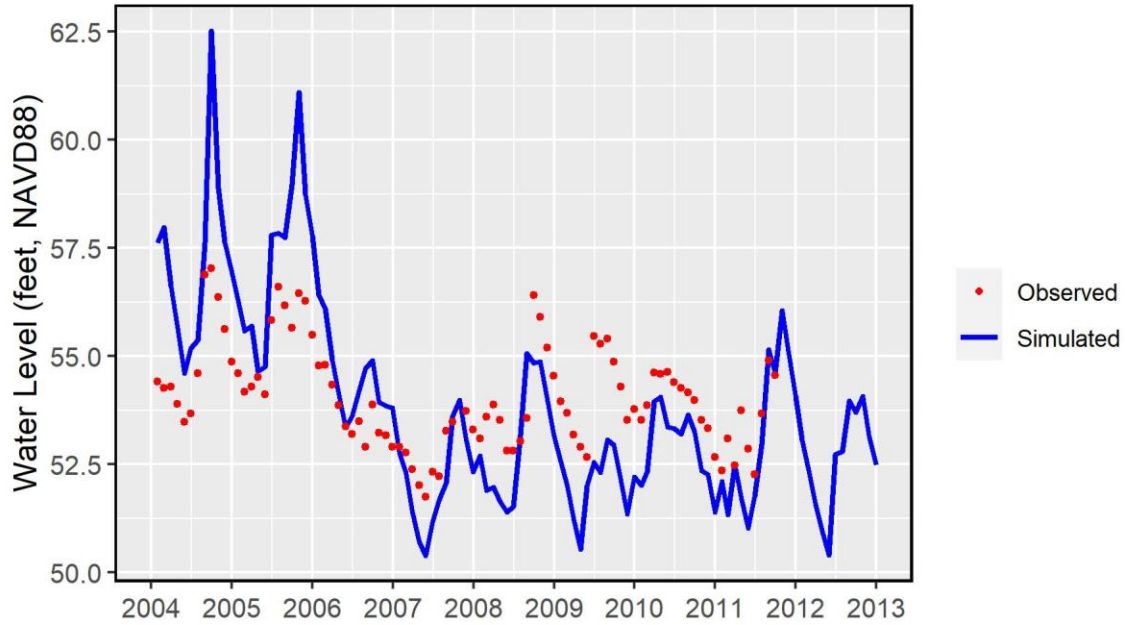
SA: SJRWMD18153699

ME=-0.913 MAE=0.945 nMAE=0.694 R2=0.887 NS=0.758



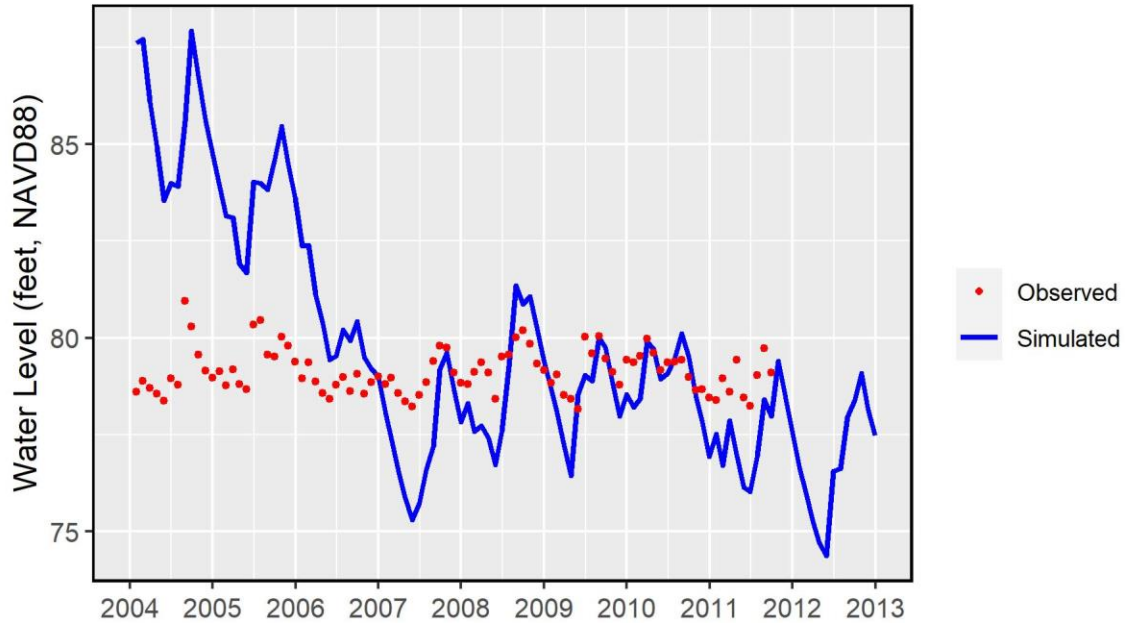
SA: SJRWMD18163700

ME=-0.043 MAE=1.394 nMAE=1.389 R2=0.605 NS=-1.03

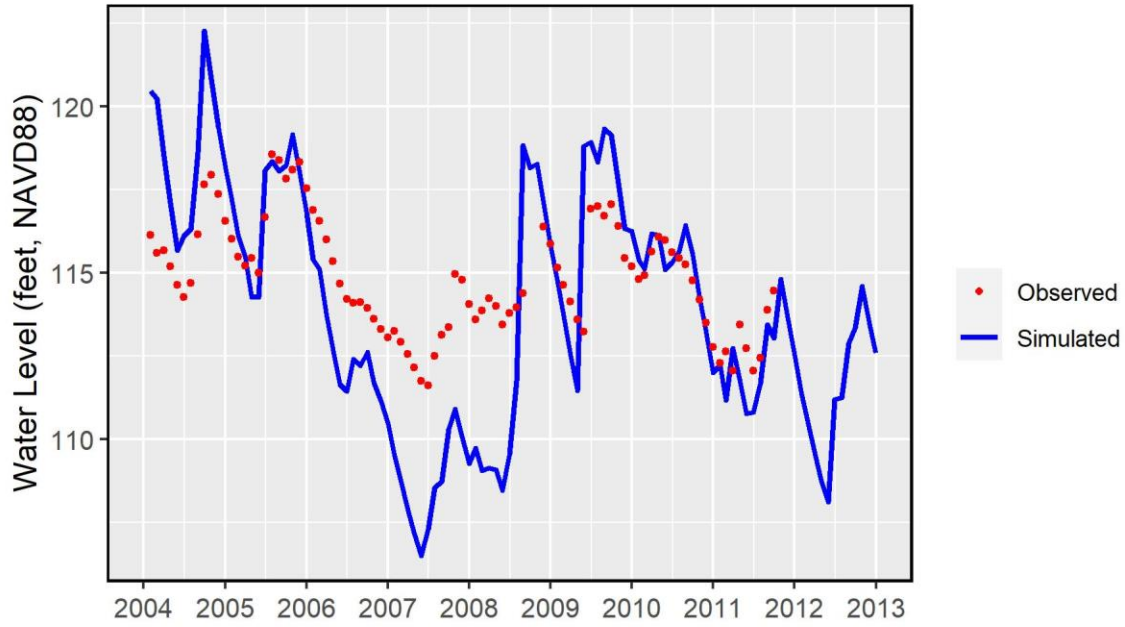


SA: SJRWMD18173701

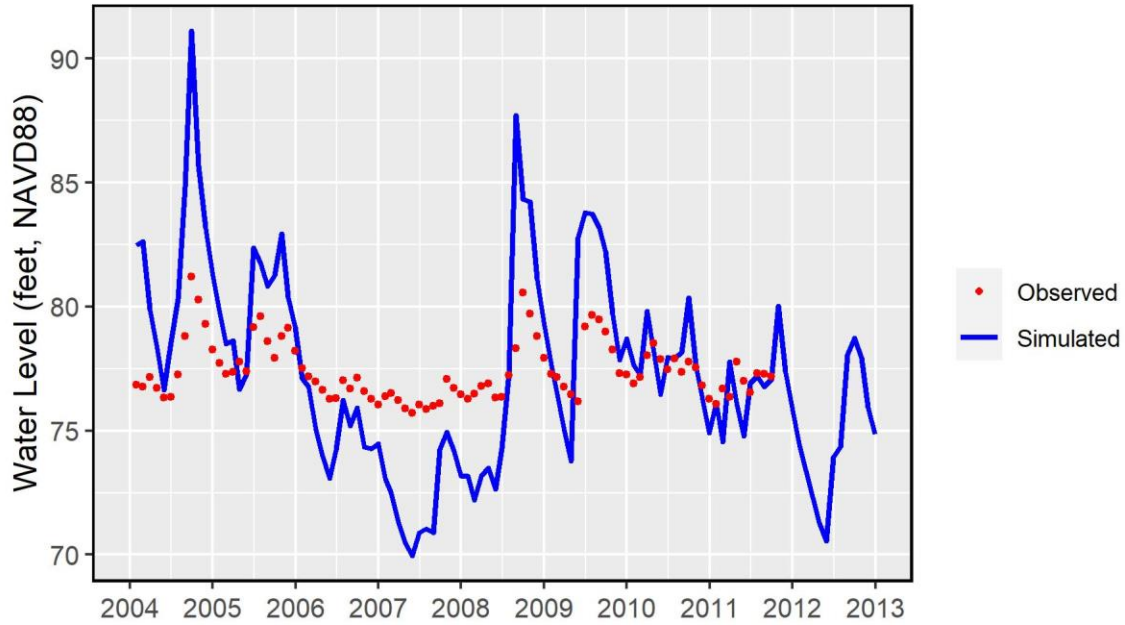
ME=1.025 MAE=2.258 nMAE=2.408 R2=0.133 NS=-29.863



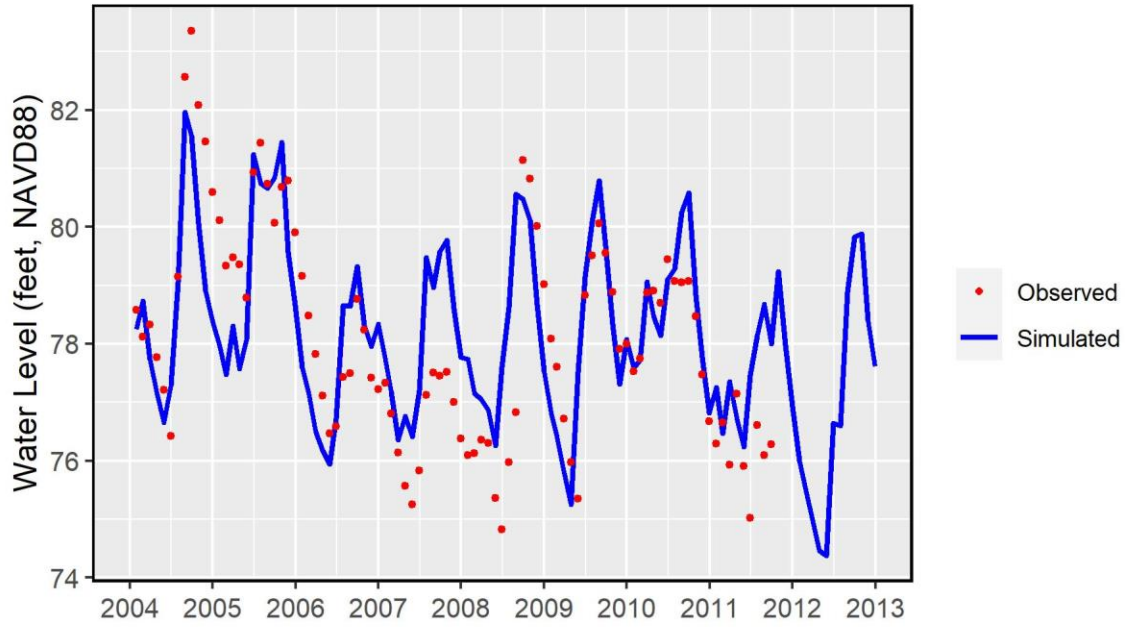
SA: SJRWMD18183702
ME=-0.705 MAE=2.089 nMAE=2.045 R2=0.667 NS=-1.436



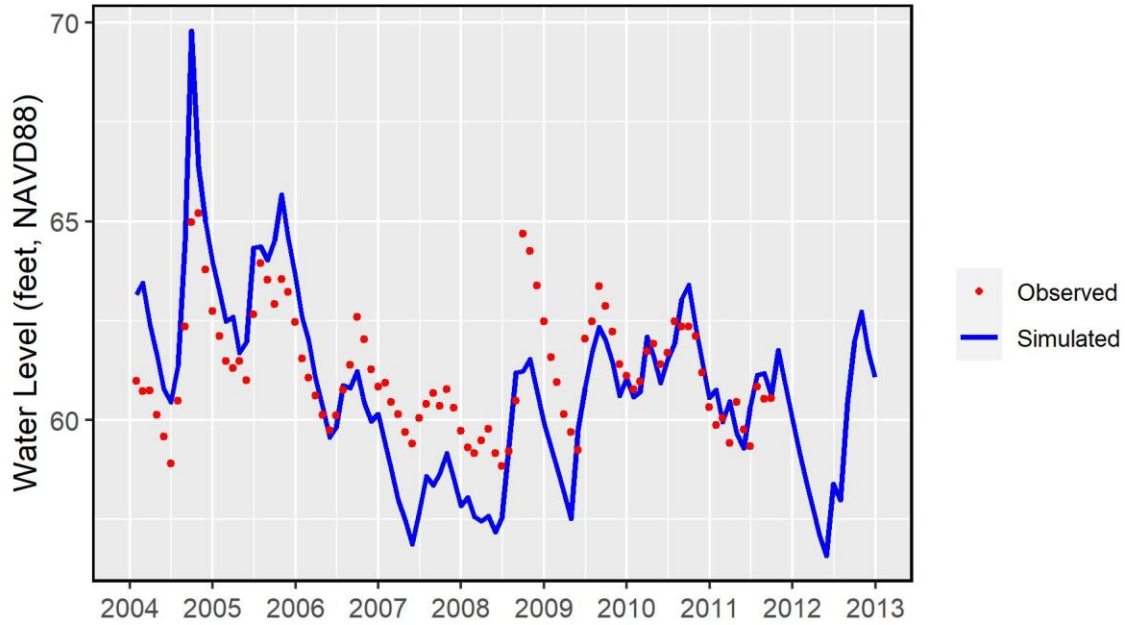
SA: SJRWMD18193703
ME=0.238 MAE=2.502 nMAE=2.513 R2=0.697 NS=-6.594



SA: SJRWMD18213705
ME=0.189 MAE=1.036 nMAE=1.022 R2=0.523 NS=0.507

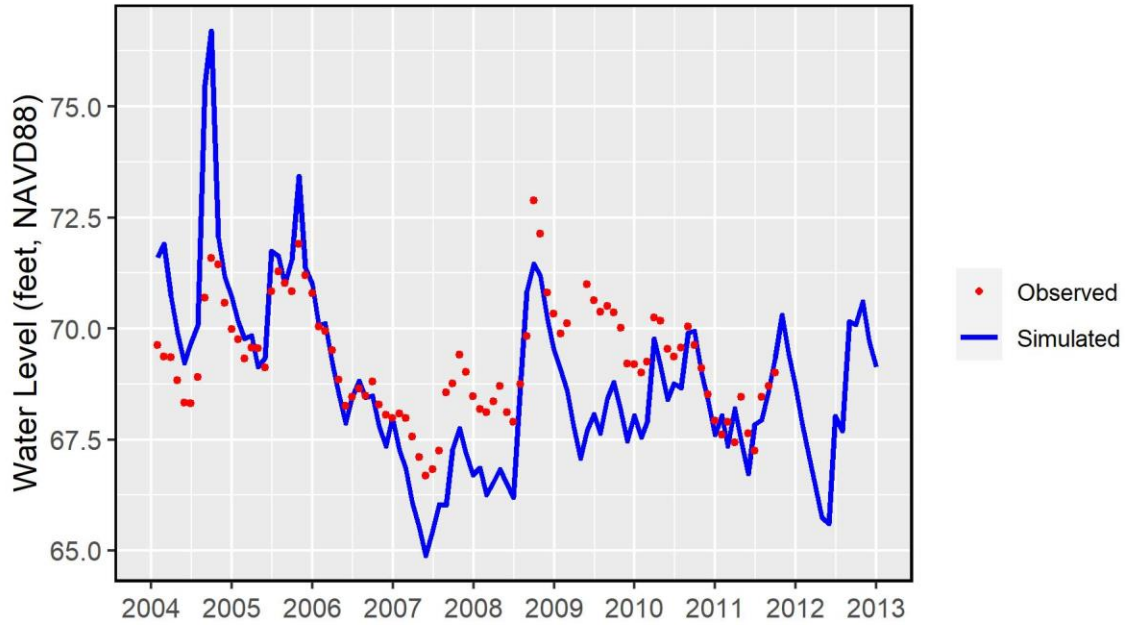


SA: SJRWMD18223706
ME=-0.238 MAE=1.241 nMAE=1.238 R2=0.596 NS=-0.085



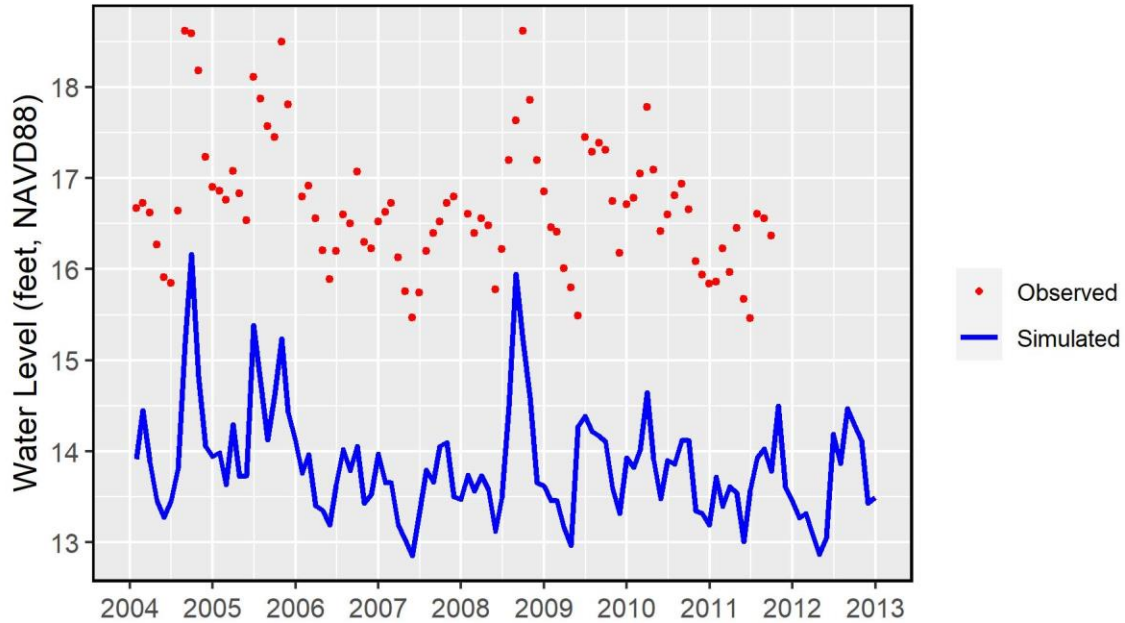
SA: SJRWMD18233707

ME=-0.379 MAE=1.065 nMAE=1.013 R2=0.577 NS=-0.263

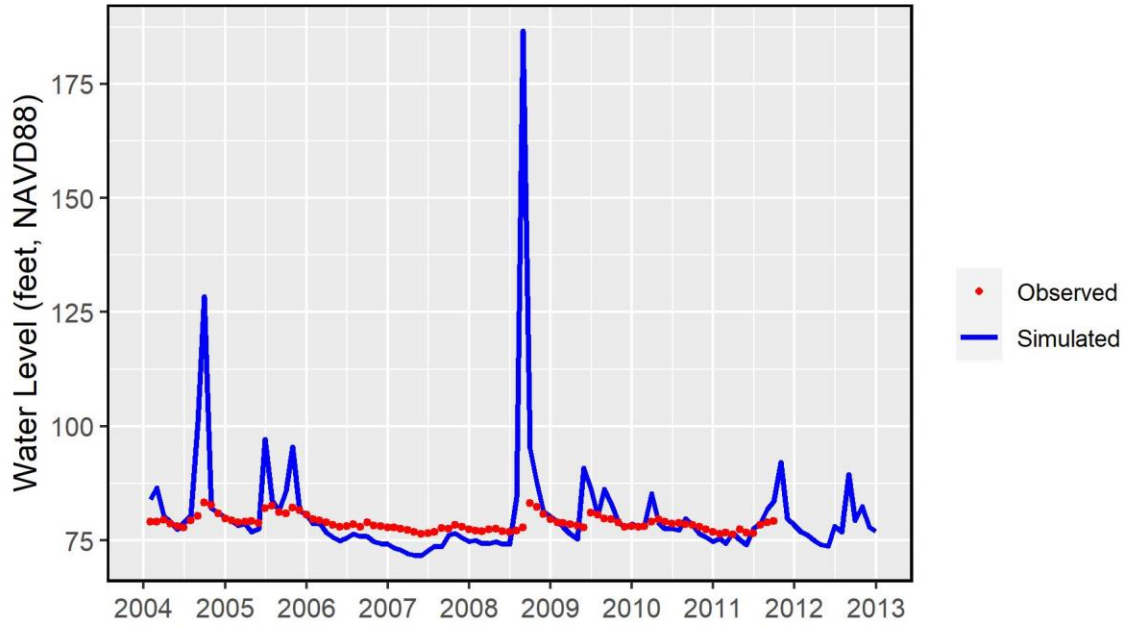


SA: SJRWMD18253709

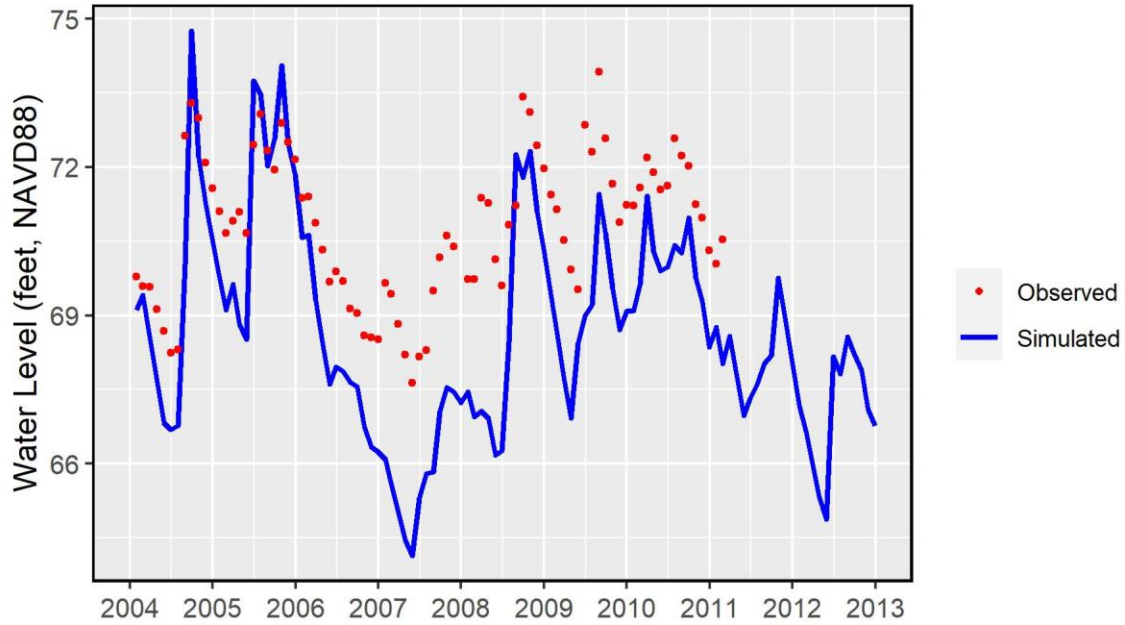
ME=-2.823 MAE=2.823 nMAE=0.257 R2=0.744 NS=-14.791



SA: SJRWMD18263710
ME=1.883 MAE=4.496 nMAE=5.434 R2=0.118 NS=-63.507

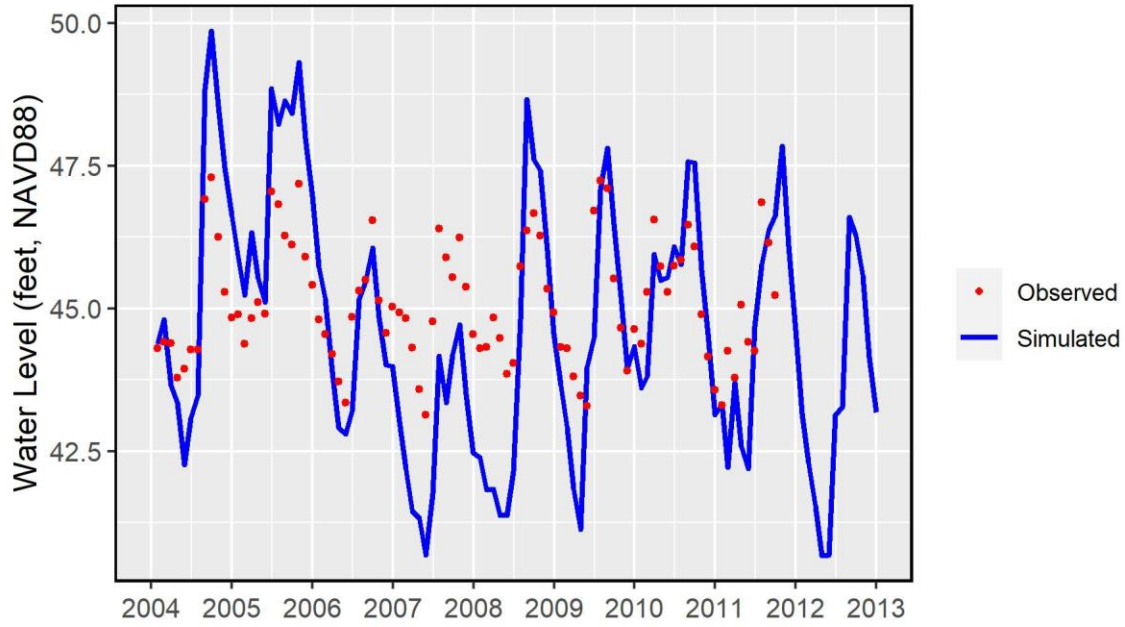


SA: SJRWMD18293713
ME=-1.828 MAE=1.97 nMAE=0.932 R2=0.749 NS=-1.259



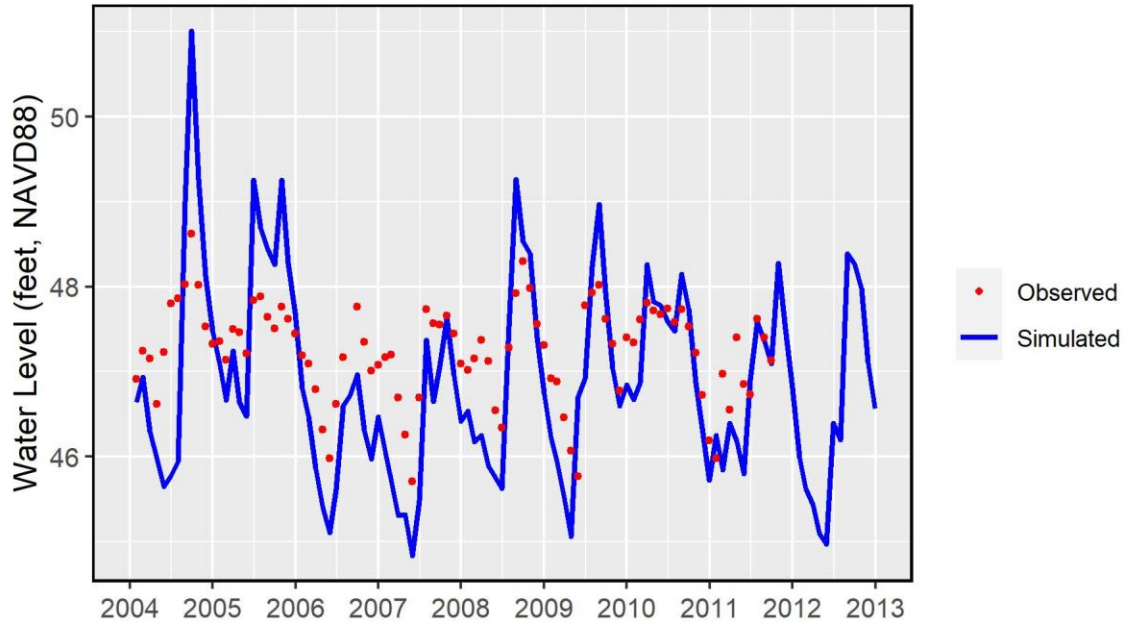
SA: SJRWMD18303714

ME=-0.318 MAE=1.28 nMAE=1.261 R2=0.615 NS=-1.138

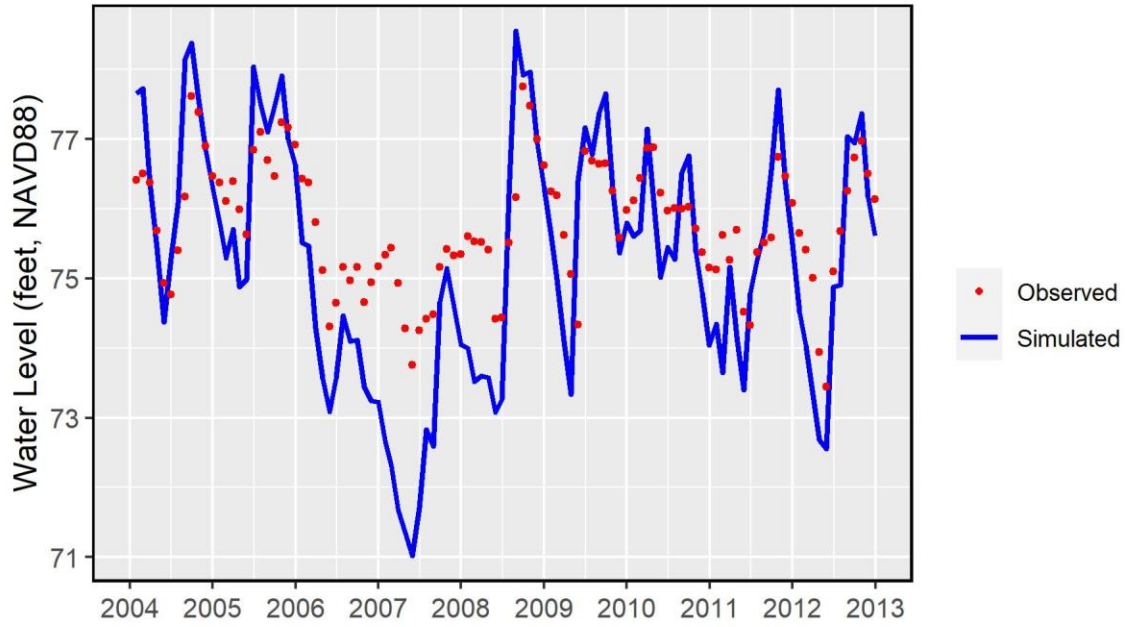


SA: SJRWMD18313715

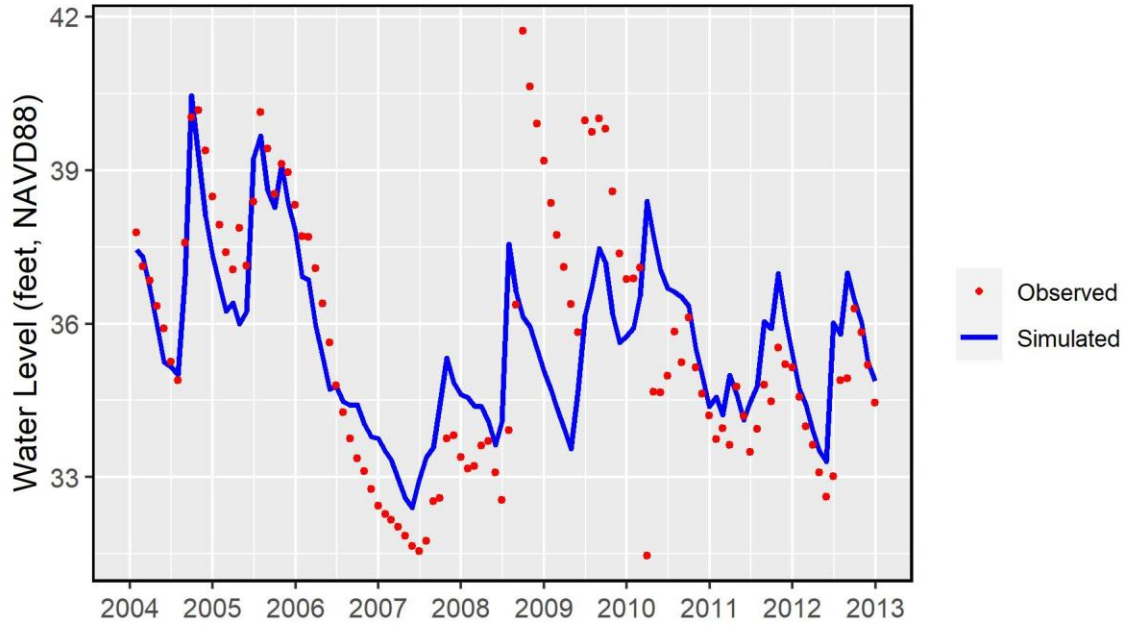
ME=-0.315 MAE=0.706 nMAE=0.622 R2=0.623 NS=-1.24



SA: SJRWMD22752272
ME=-0.524 MAE=0.964 nMAE=0.858 R2=0.71 NS=-0.803

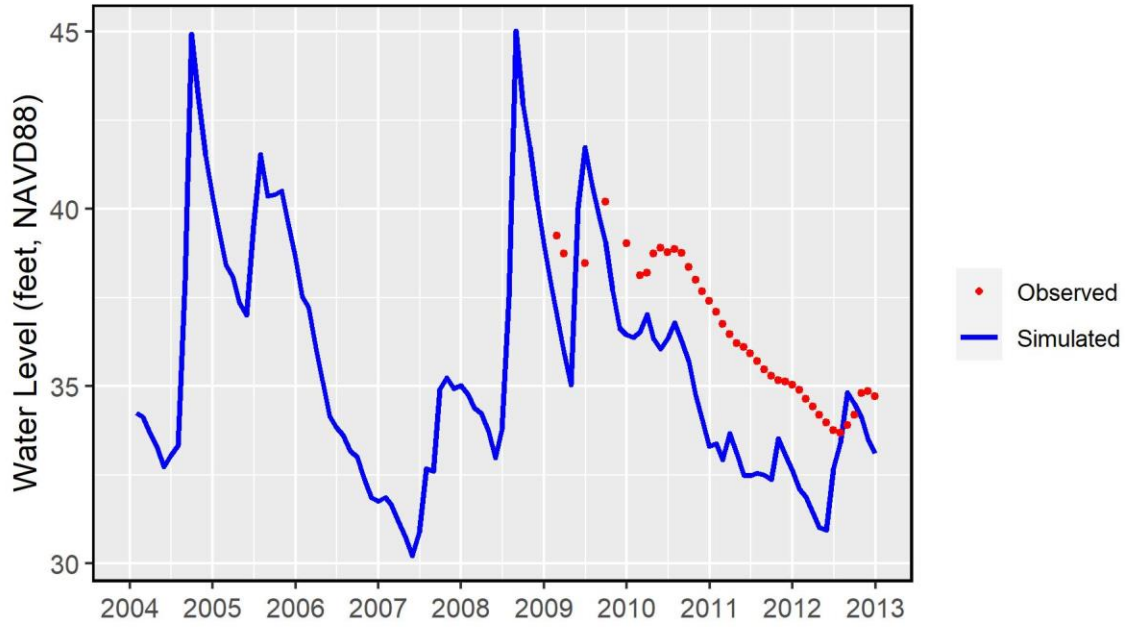


SA: SJRWMD30072618
ME=-0.037 MAE=1.27 nMAE=1.276 R2=0.492 NS=0.49



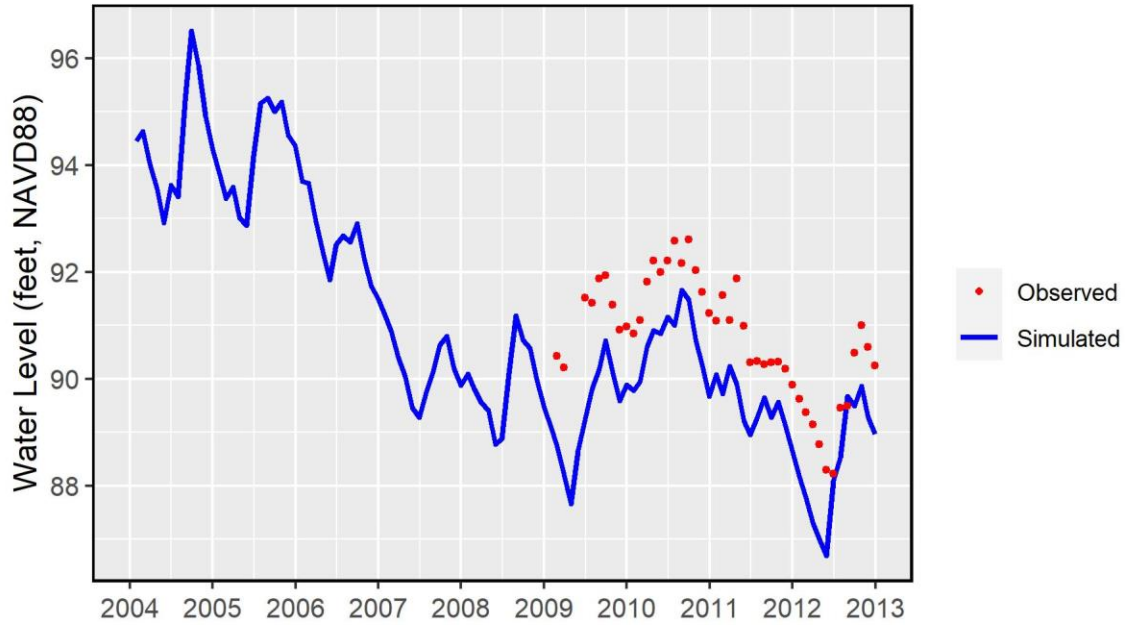
SA: SJRWMD30432904

ME=-2.208 MAE=2.432 nMAE=1.035 R2=0.61 NS=-0.899

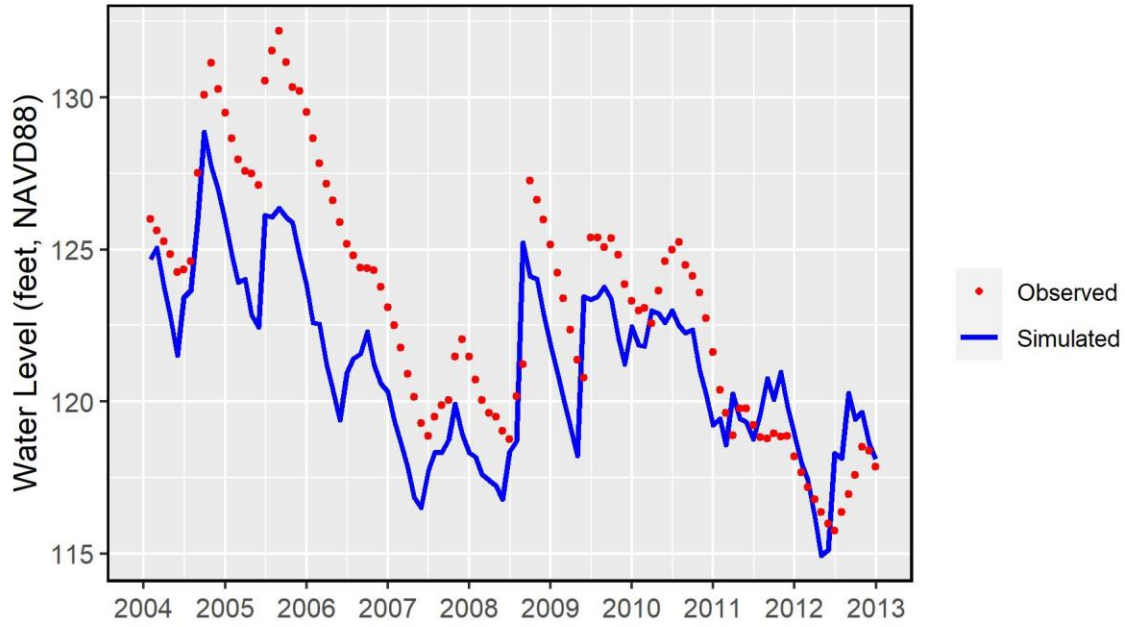


SA: SJRWMD30442915

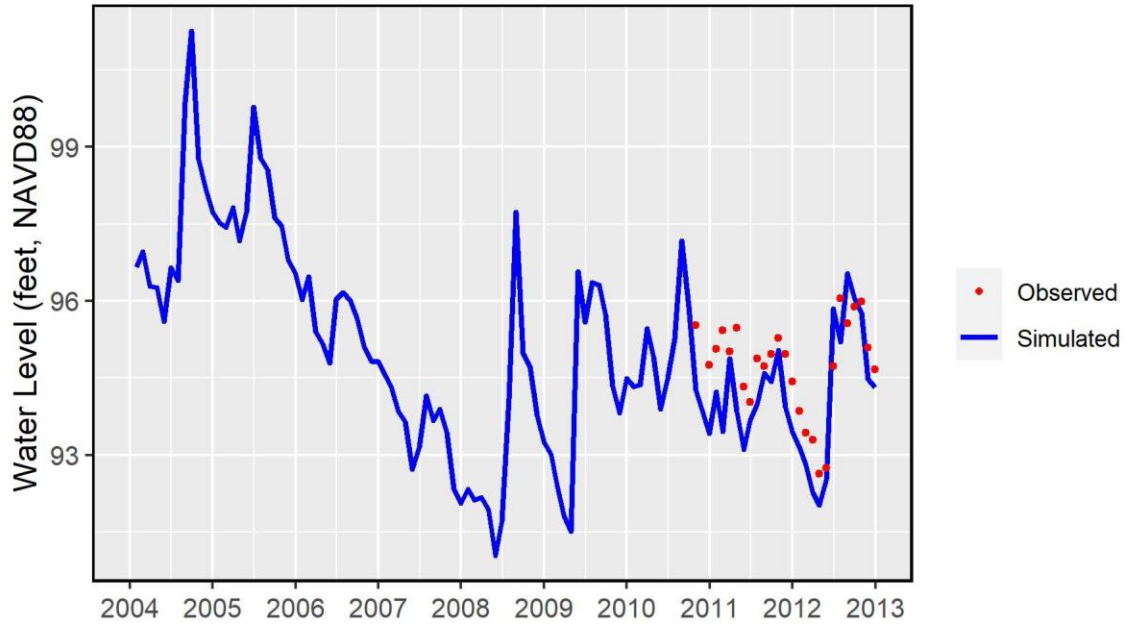
ME=-1.267 MAE=1.275 nMAE=0.34 R2=0.831 NS=-0.546



SA: SJRWMD51005097
ME=-1.961 MAE=2.477 nMAE=1.672 R2=0.742 NS=0.487

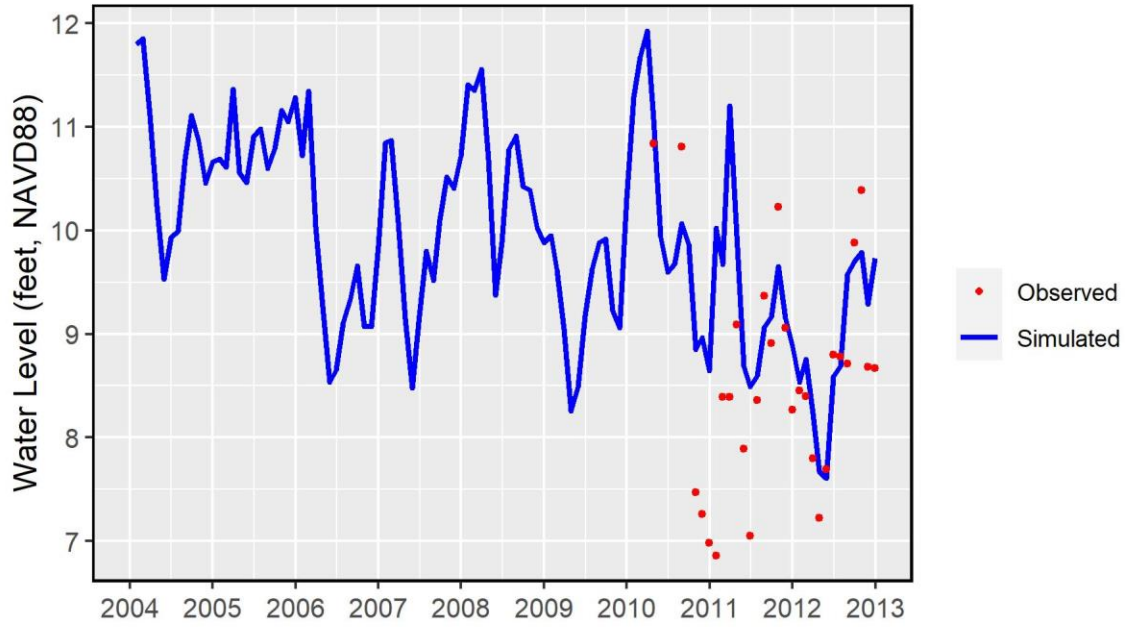


SA: SJRWMD31792875
ME=-0.596 MAE=0.769 nMAE=0.508 R2=0.65 NS=0.009



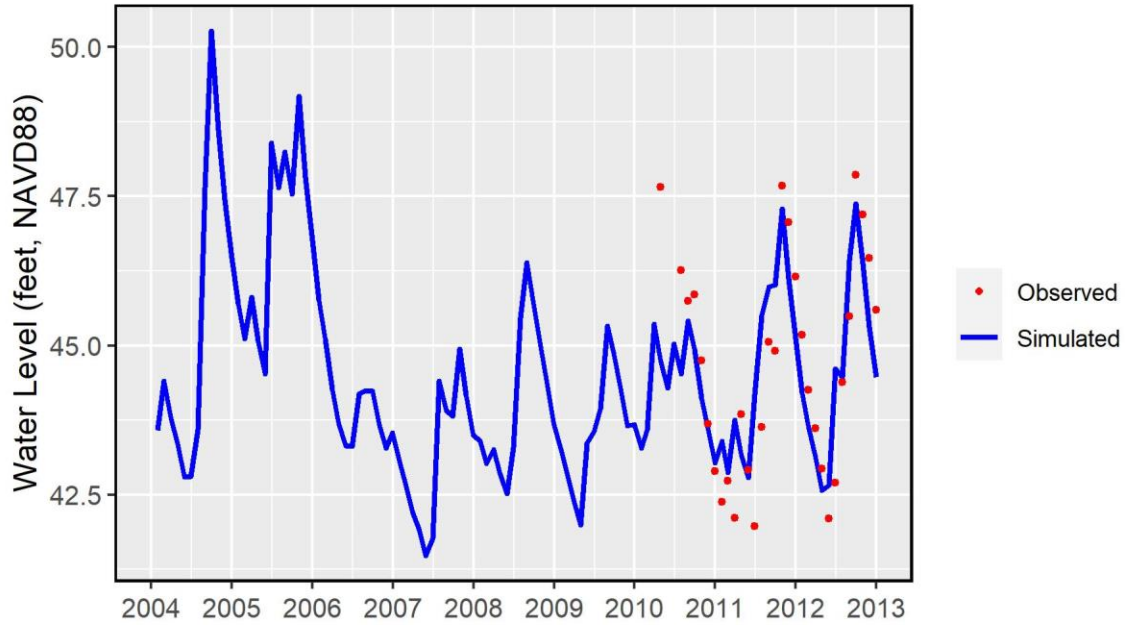
SA: SJRWMD31283464

ME=0.606 MAE=0.799 nMAE=0.715 R2=0.314 NS=-0.041

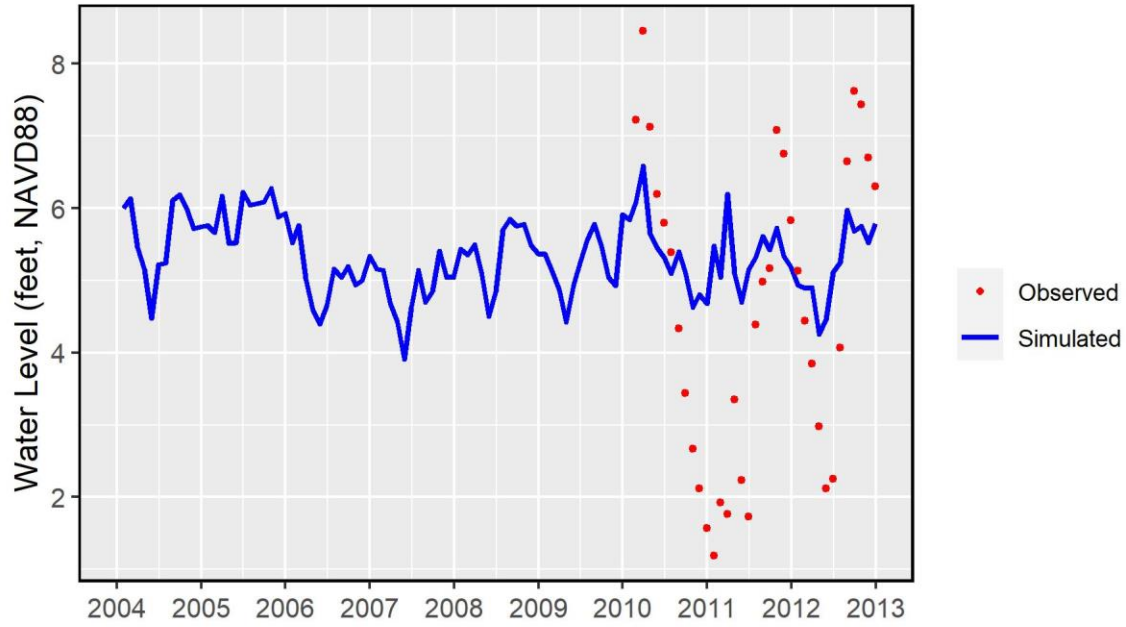


SA: SJRWMD31243454

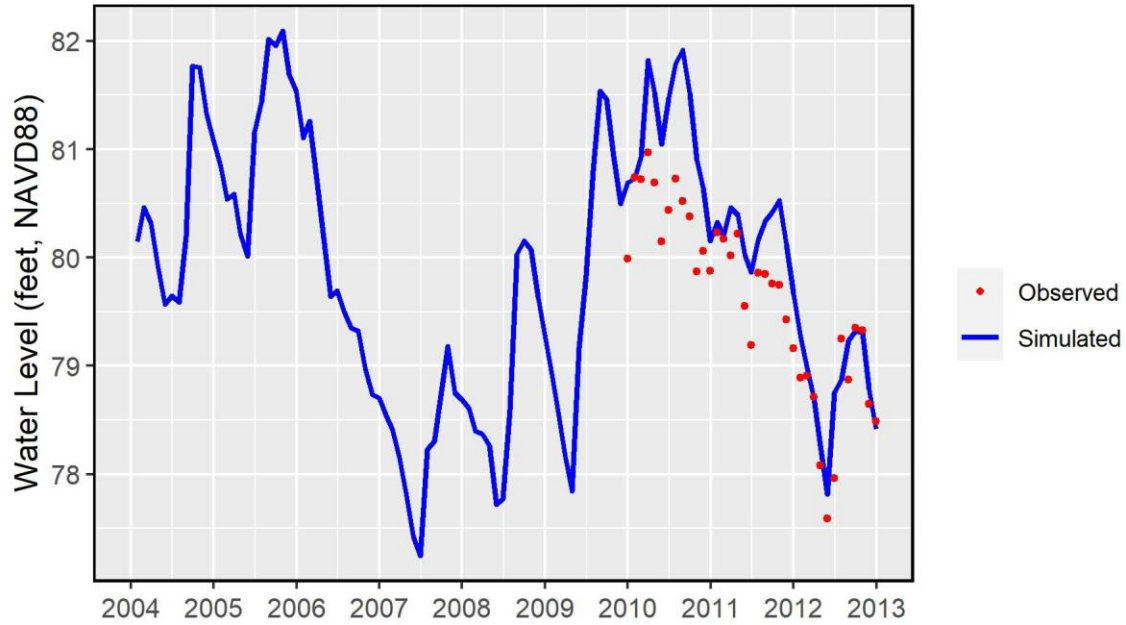
ME=-0.106 MAE=0.916 nMAE=0.893 R2=0.604 NS=0.6



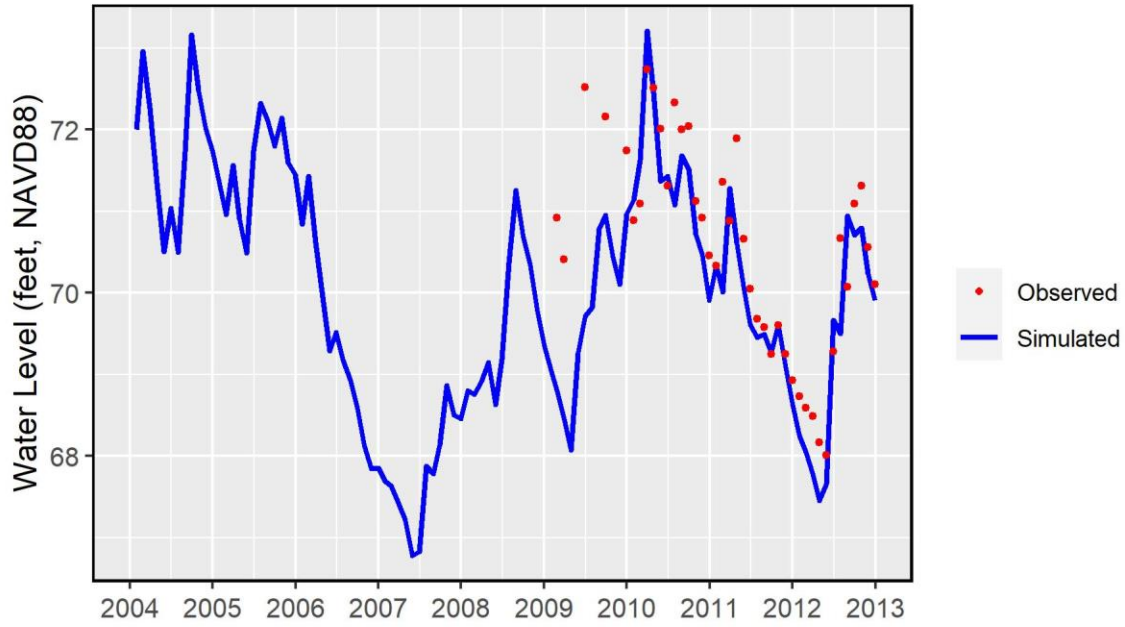
SA: SJRWMD31213440
ME=0.725 MAE=1.616 nMAE=1.56 R2=0.4 NS=0.124



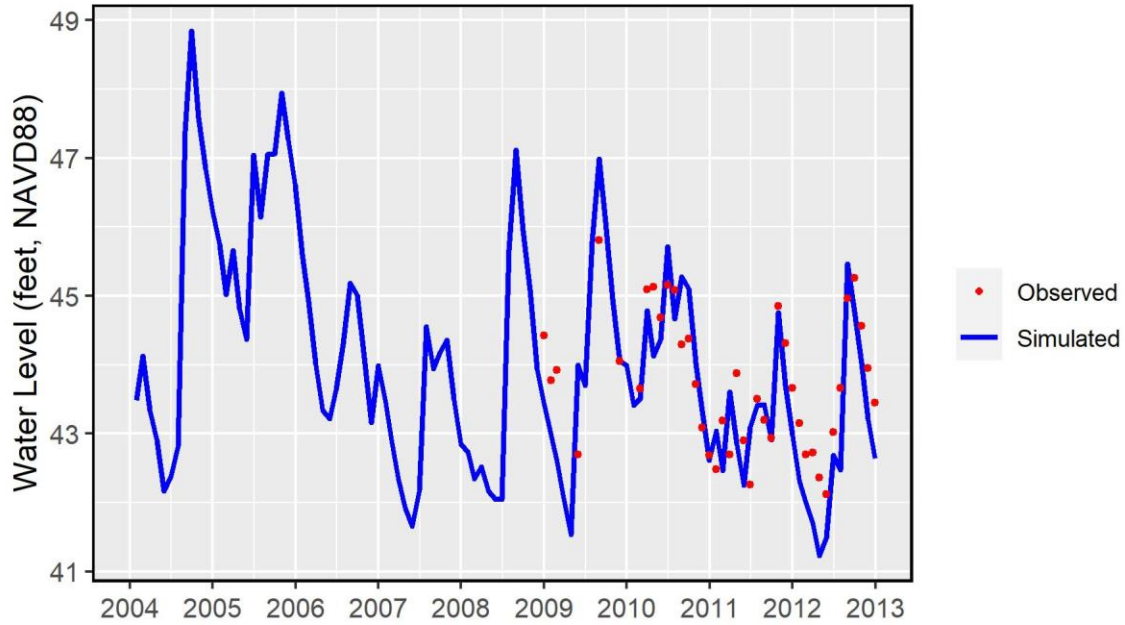
SA: SJRWMD31043396
ME=0.459 MAE=0.488 nMAE=0.342 R2=0.878 NS=0.454



SA: SJRWMD30452909
ME=-0.483 MAE=0.633 nMAE=0.512 R2=0.712 NS=0.533

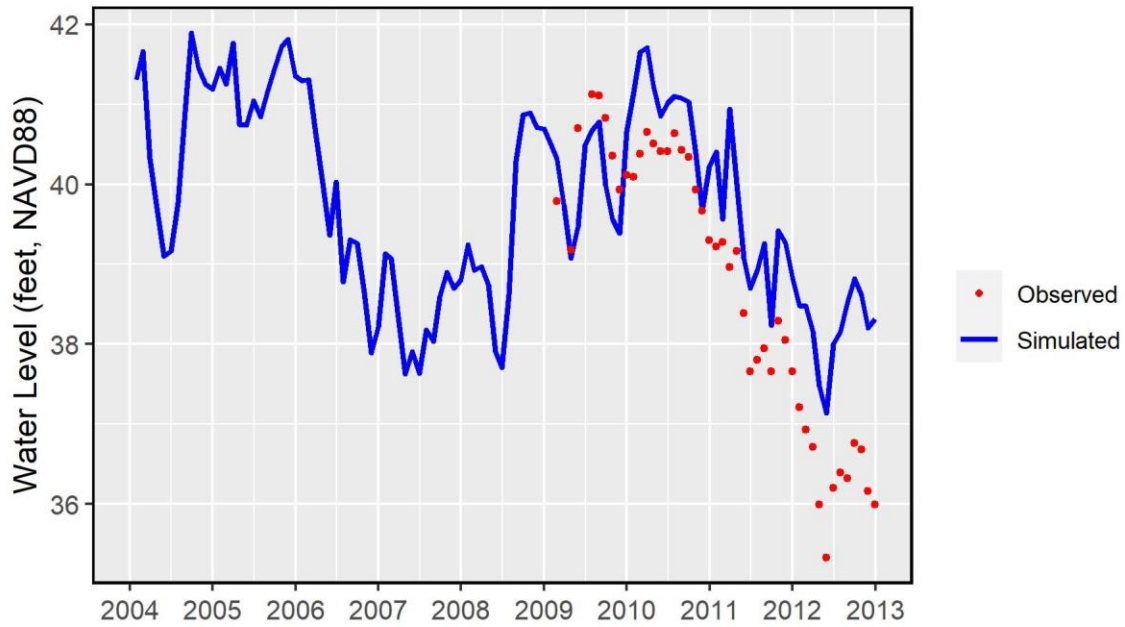


SA: SJRWMD30362860
ME=-0.228 MAE=0.634 nMAE=0.585 R2=0.67 NS=0.412



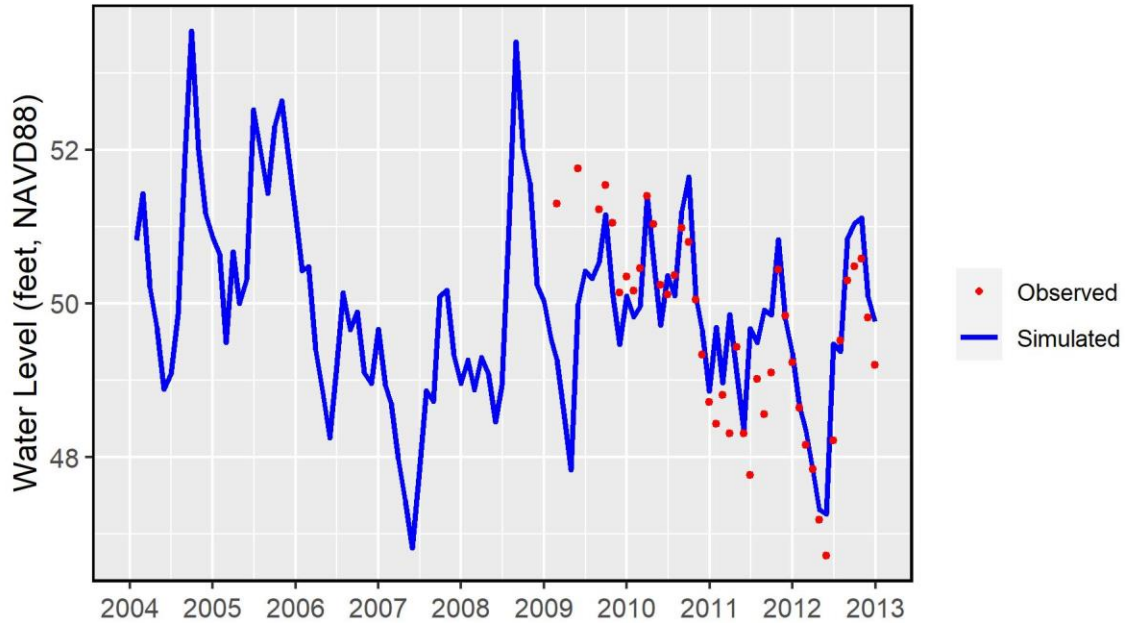
SA: SJRWMD30342852

ME=0.874 MAE=1.065 nMAE=0.675 R2=0.794 NS=0.491

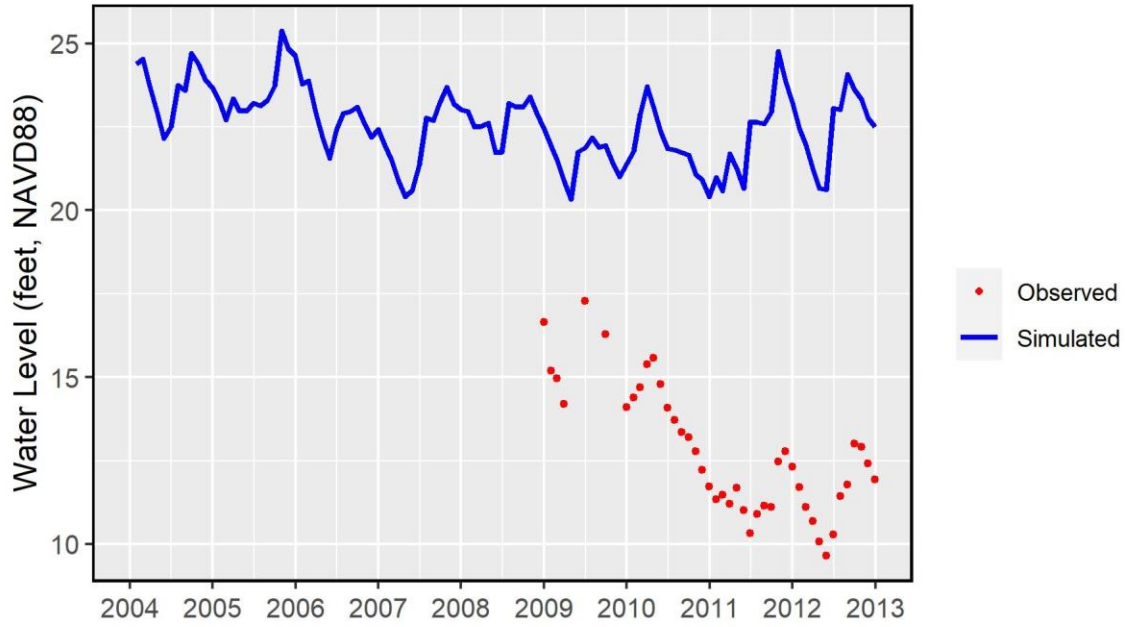


SA: SJRWMD30332846

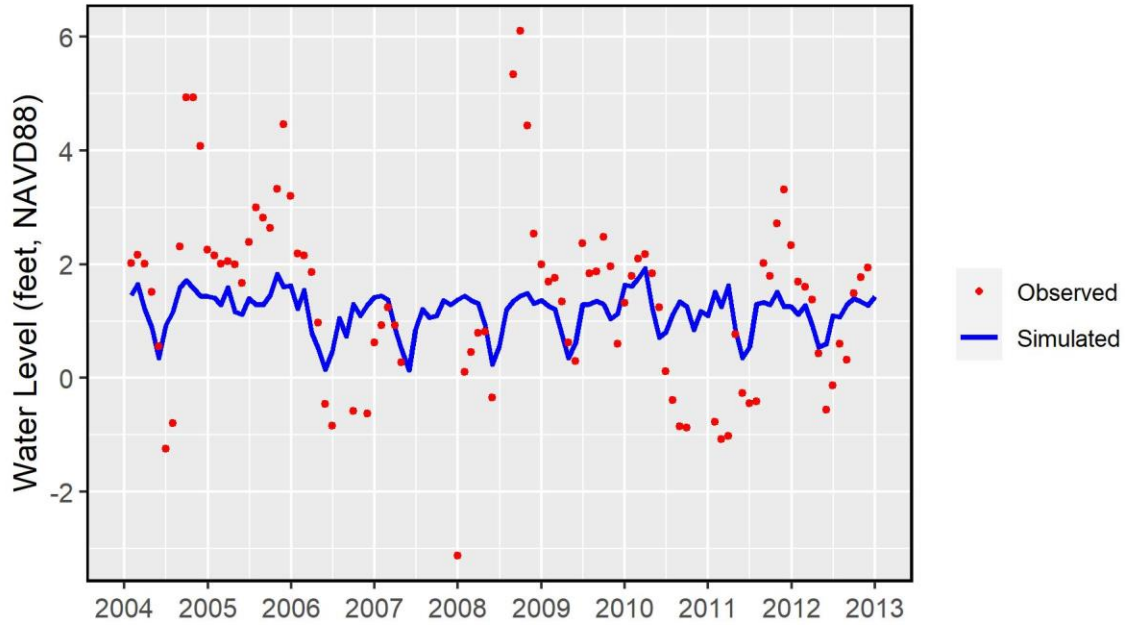
ME=0.119 MAE=0.554 nMAE=0.54 R2=0.625 NS=0.615



SA: SJRWMD30322836
ME=9.37 MAE=9.37 nMAE=1.753 R2=0.019 NS=-24.927

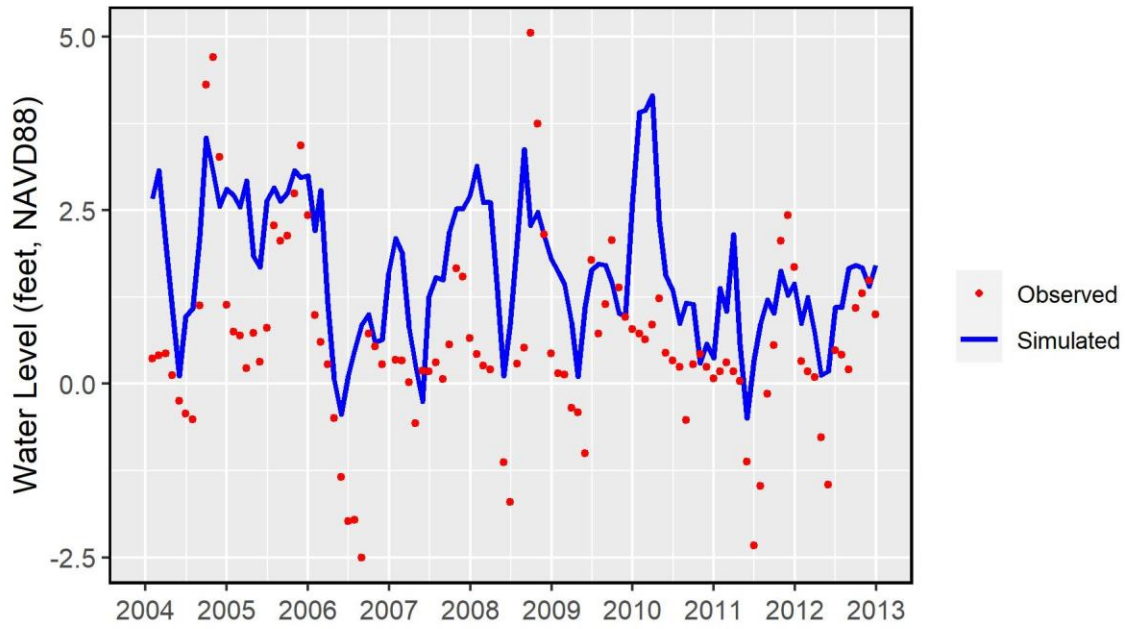


SA: SJRWMD18563778
ME=-0.206 MAE=1.115 nMAE=1.071 R2=0.206 NS=0.141



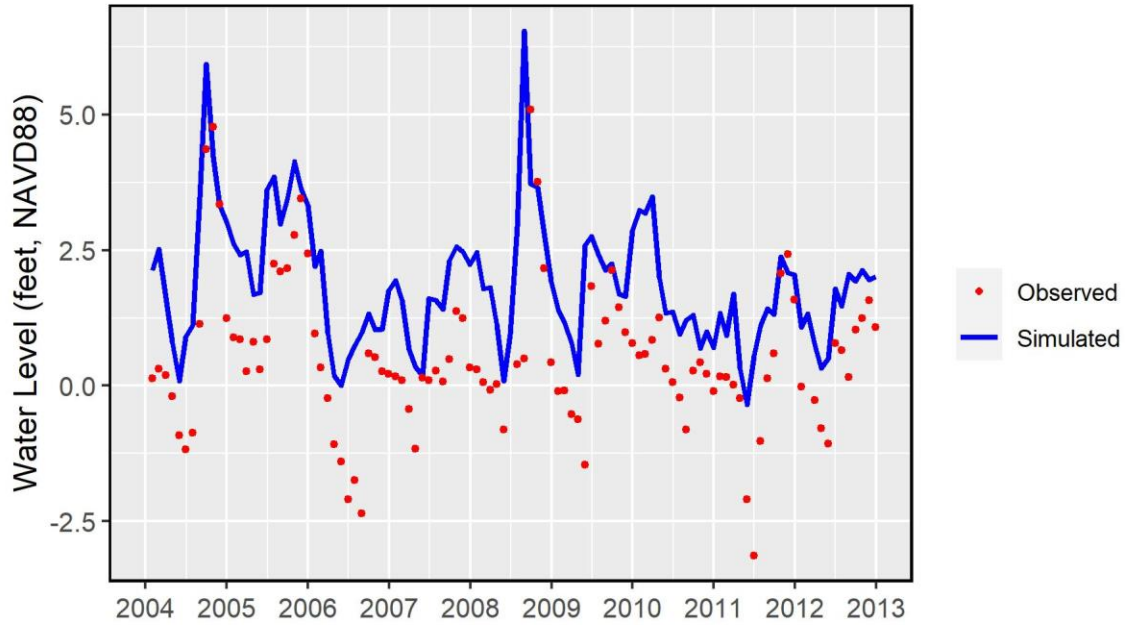
SA: SJRWMD18433760

ME=1.033 MAE=1.245 nMAE=0.829 R2=0.354 NS=-0.295

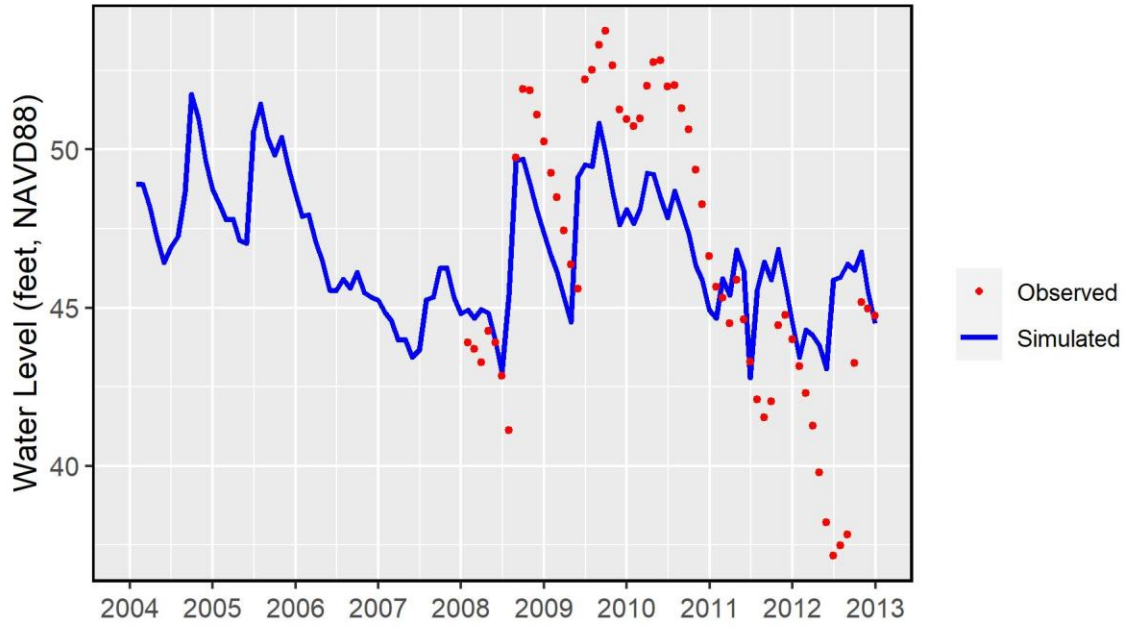


SA: SJRWMD18433759

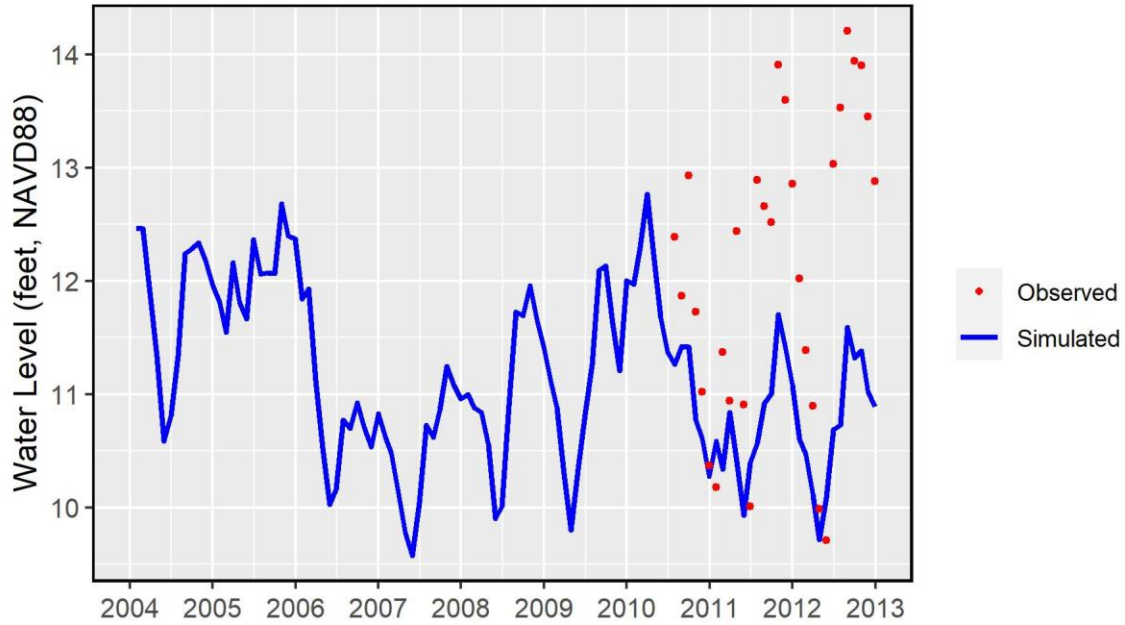
ME=1.361 MAE=1.406 nMAE=0.665 R2=0.544 NS=-0.45



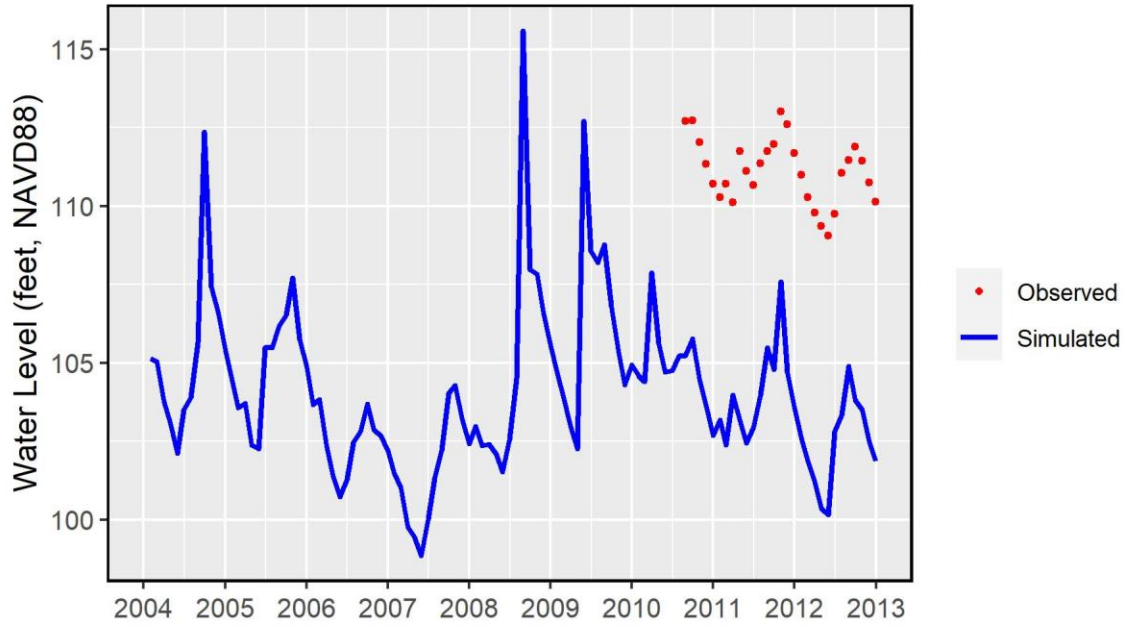
SA: SJRWMD15474993
ME=-0.082 MAE=2.654 nMAE=2.652 R2=0.6 NS=0.485



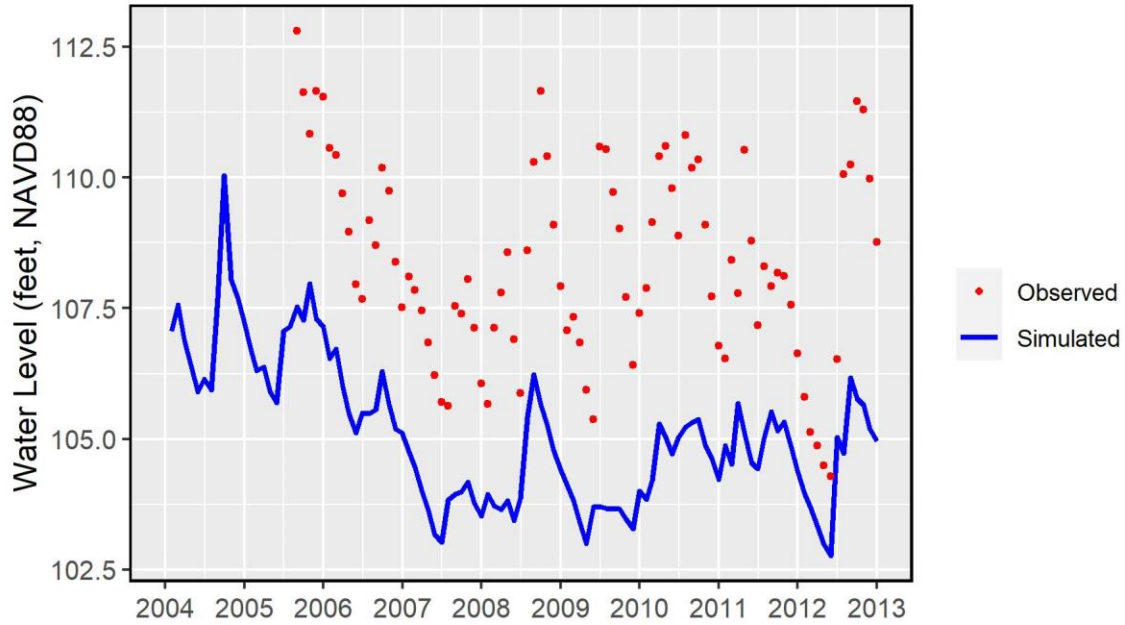
SA: SJRWMD31733211
ME=-1.333 MAE=1.411 nMAE=0.847 R2=0.619 NS=-0.564



SA: SJRWMD31713206
ME=-7.711 MAE=7.711 nMAE=0.676 R2=0.767 NS=-57.82

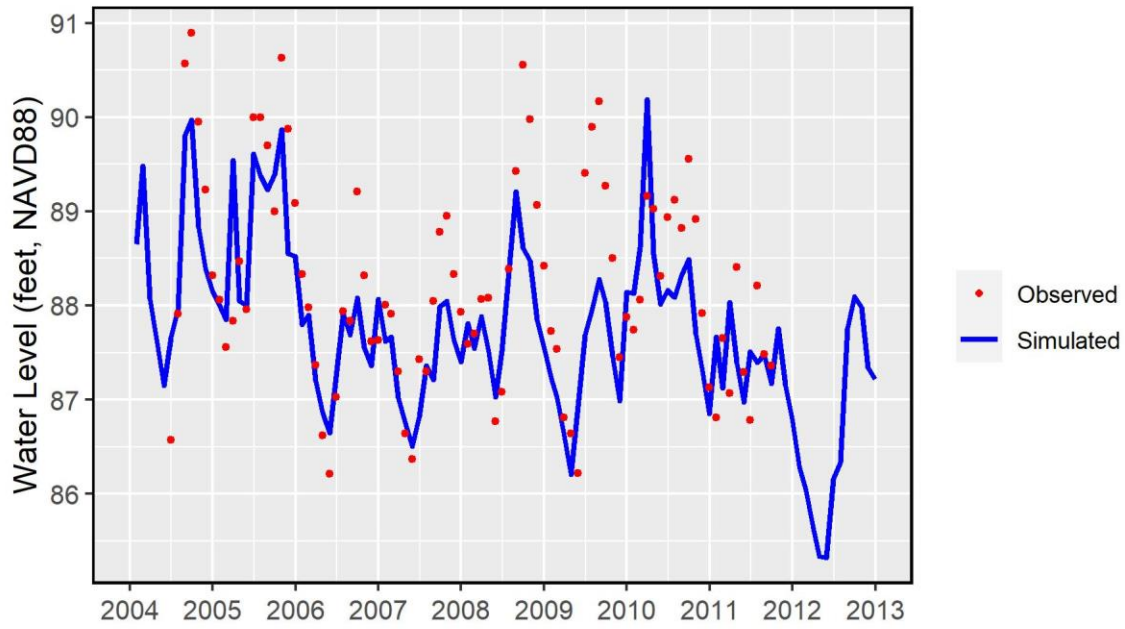


SA: SJRWMD19534471
ME=-3.646 MAE=3.646 nMAE=1.006 R2=0.579 NS=-3.199



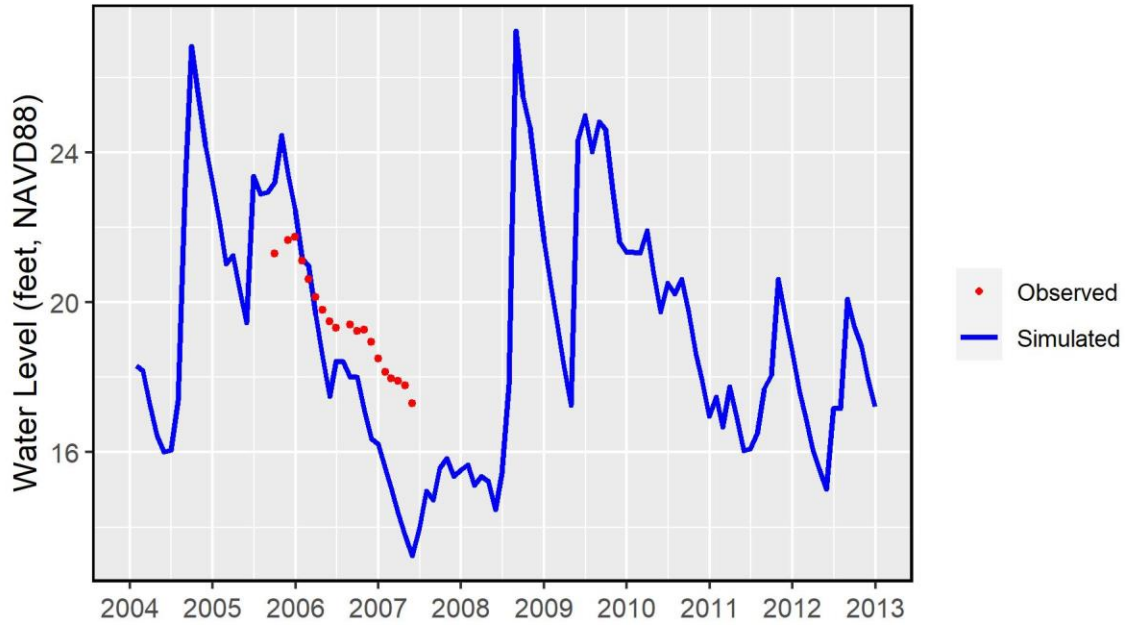
SA: SJRWMD18203704

ME=-0.351 MAE=0.616 nMAE=0.533 R2=0.617 NS=0.515

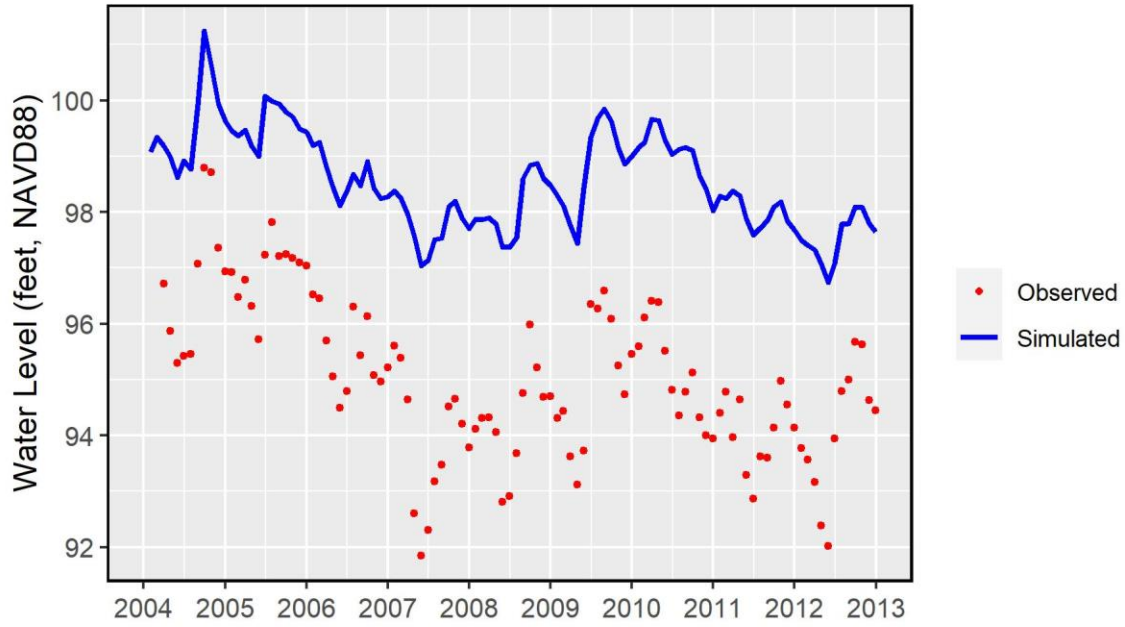


SA: USGS285543081133811

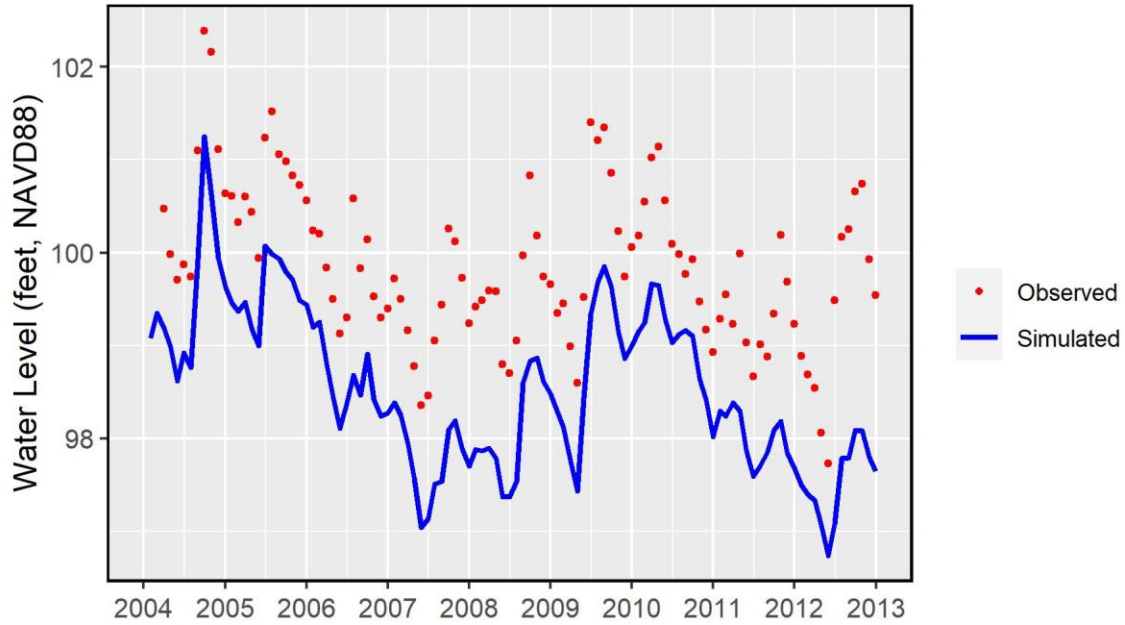
ME=-1.398 MAE=1.9 nMAE=1.423 R2=0.973 NS=-1.832



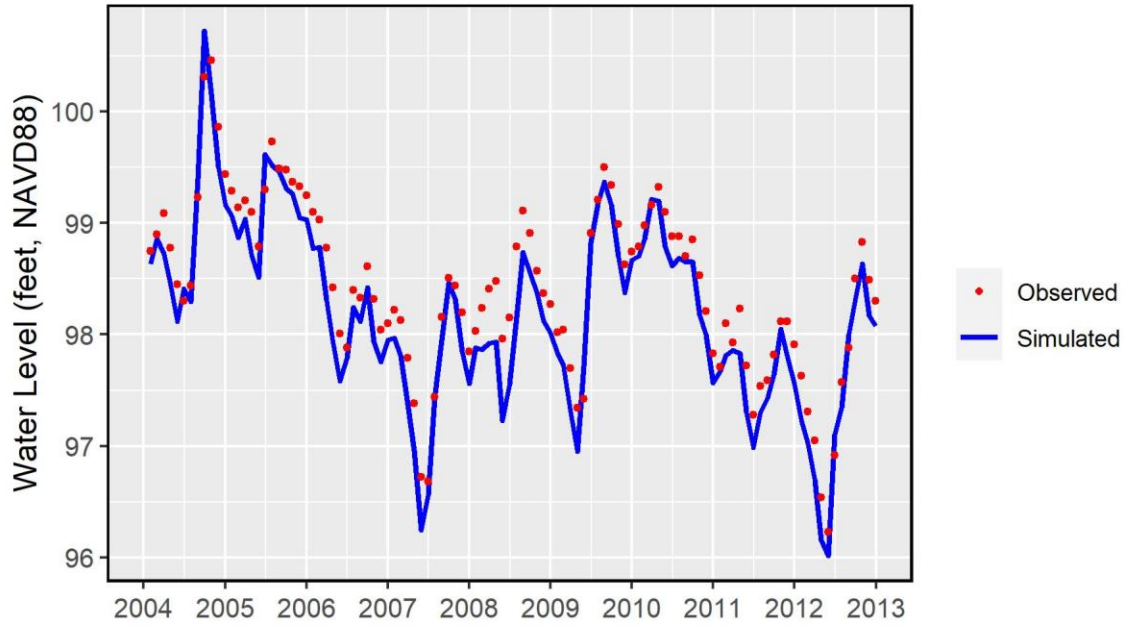
SA: ORH-1
ME=3.498 MAE=3.498 nMAE=0.581 R2=0.844 NS=-5.362



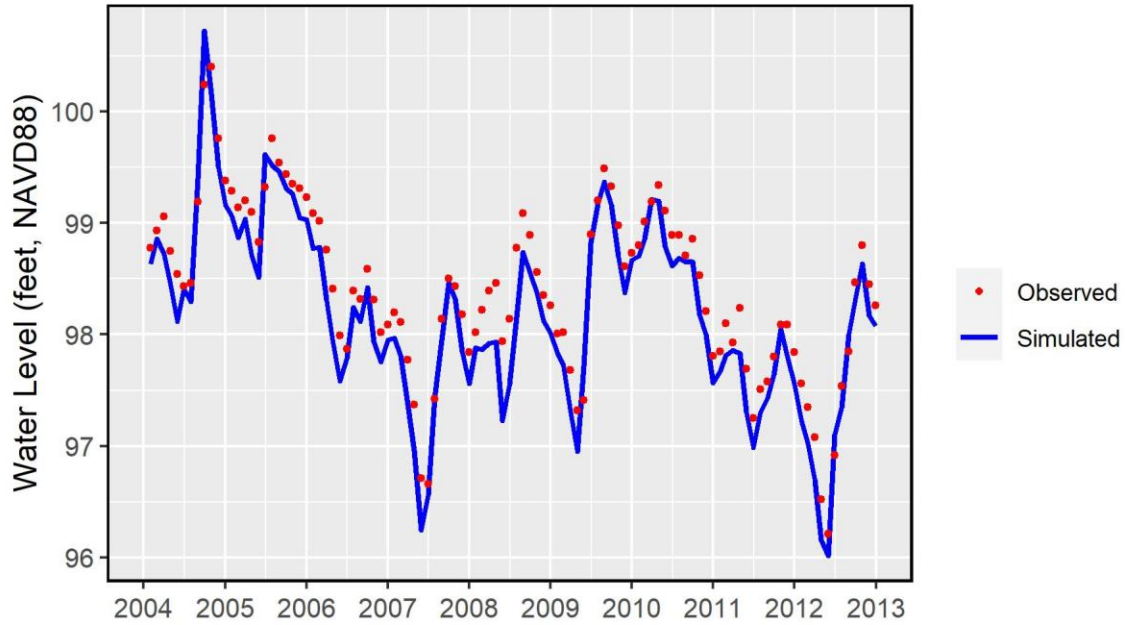
SA: ORS-3
ME=-1.334 MAE=1.334 nMAE=0.31 R2=0.786 NS=-1.682



SA: TB3_GW1
ME=-0.227 MAE=0.257 nMAE=0.136 R2=0.951 NS=0.857

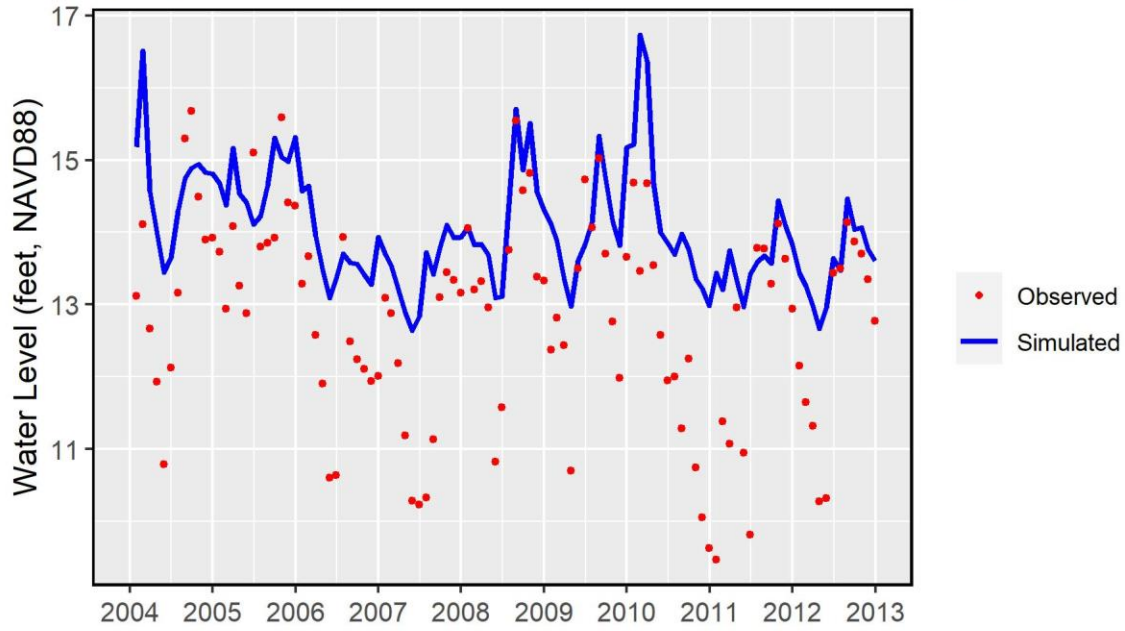


SA: TB3_GW2
ME=-0.218 MAE=0.248 nMAE=0.129 R2=0.954 NS=0.866



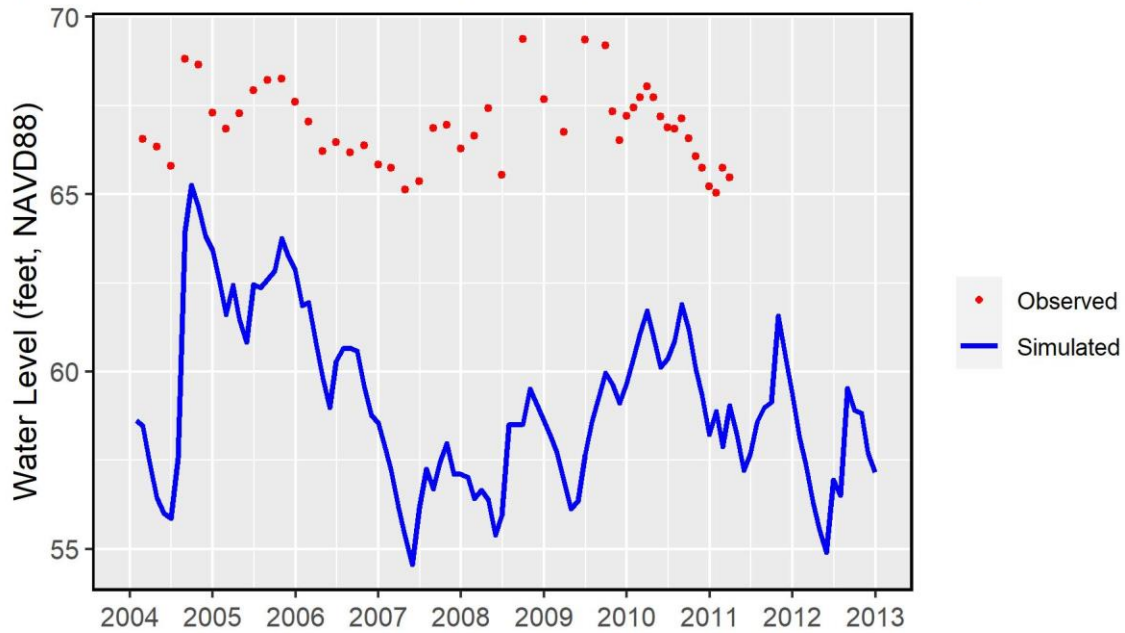
SA: SJRWMD05521047

ME=1.223 MAE=1.302 nMAE=0.799 R2=0.542 NS=-0.216



SA: SJRWMD05581055

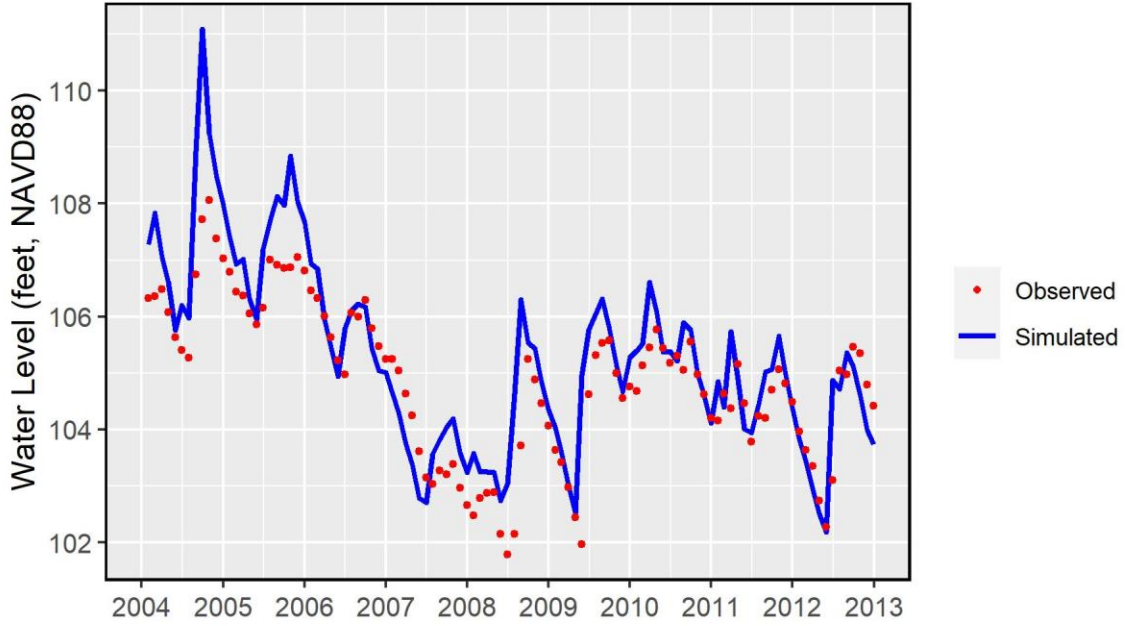
ME=-7.322 MAE=7.322 nMAE=1.683 R2=0.278 NS=-47.658



Upper Floridan Aquifer (UFA) Target Wells

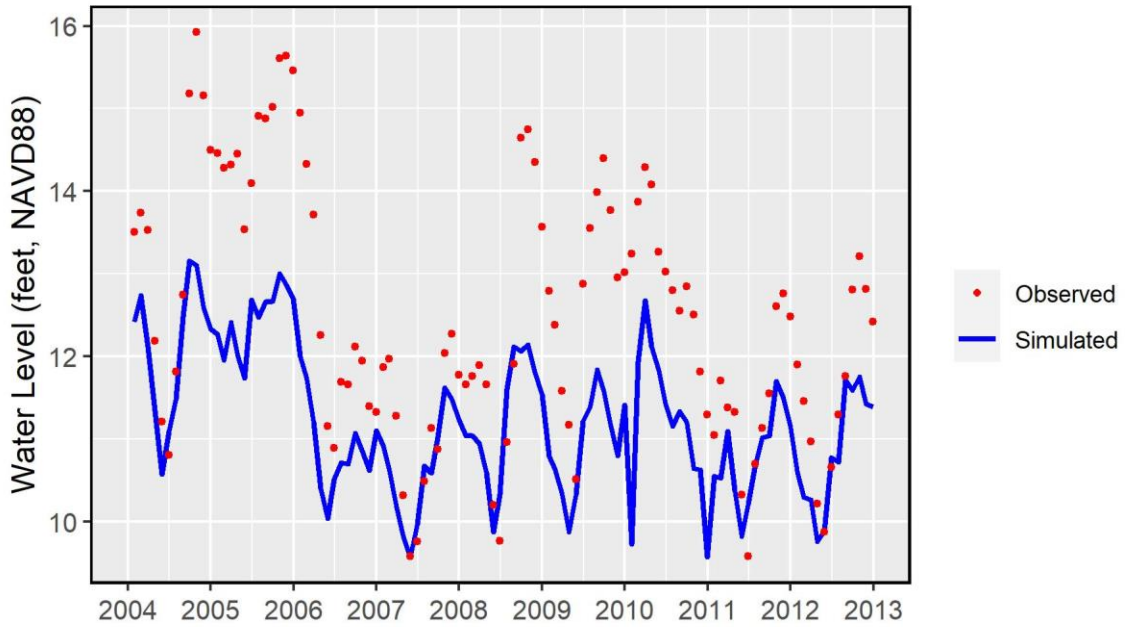
UFA: SJRWMD00660060

ME=0.435 MAE=0.645 nMAE=0.567 R2=0.787 NS=0.598

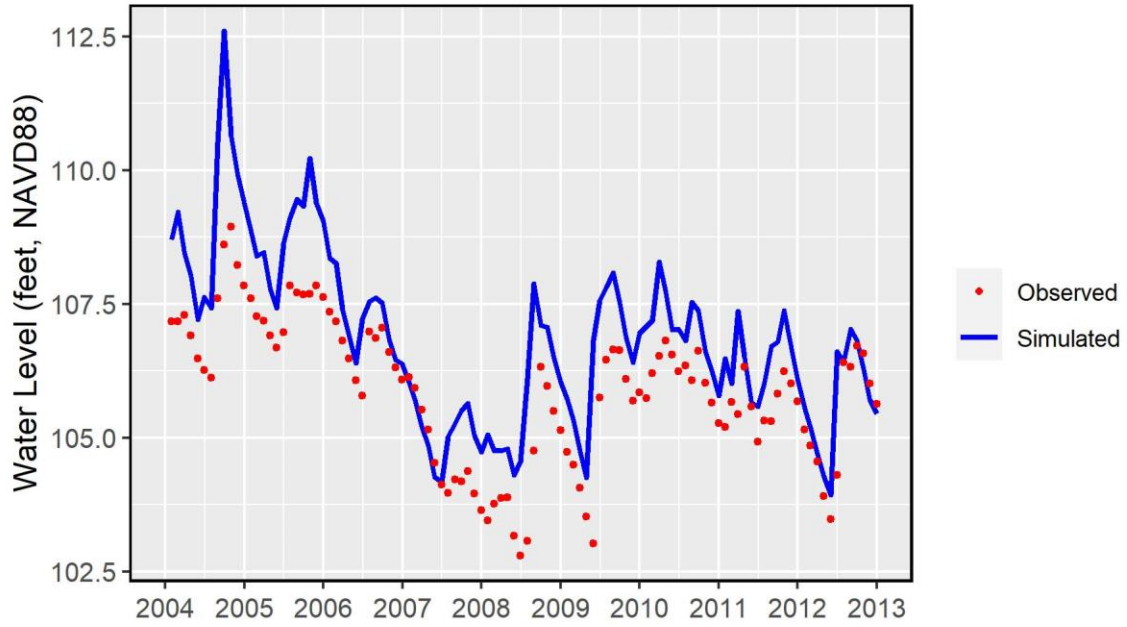


UFA: SJRWMD01140470

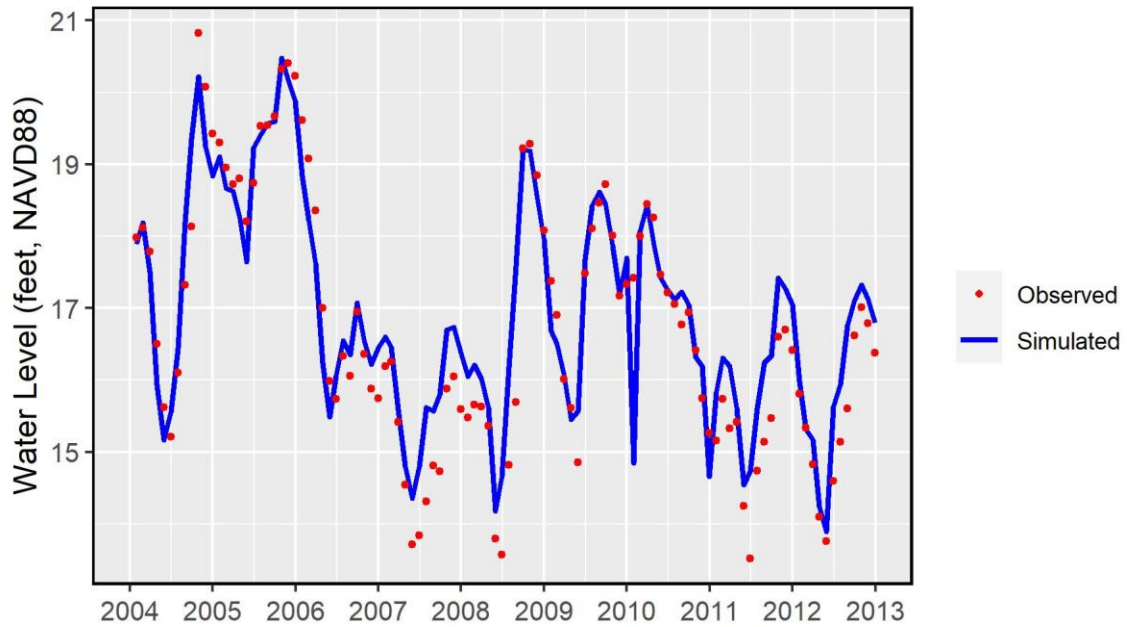
ME=-1.253 MAE=1.309 nMAE=0.763 R2=0.729 NS=-0.006



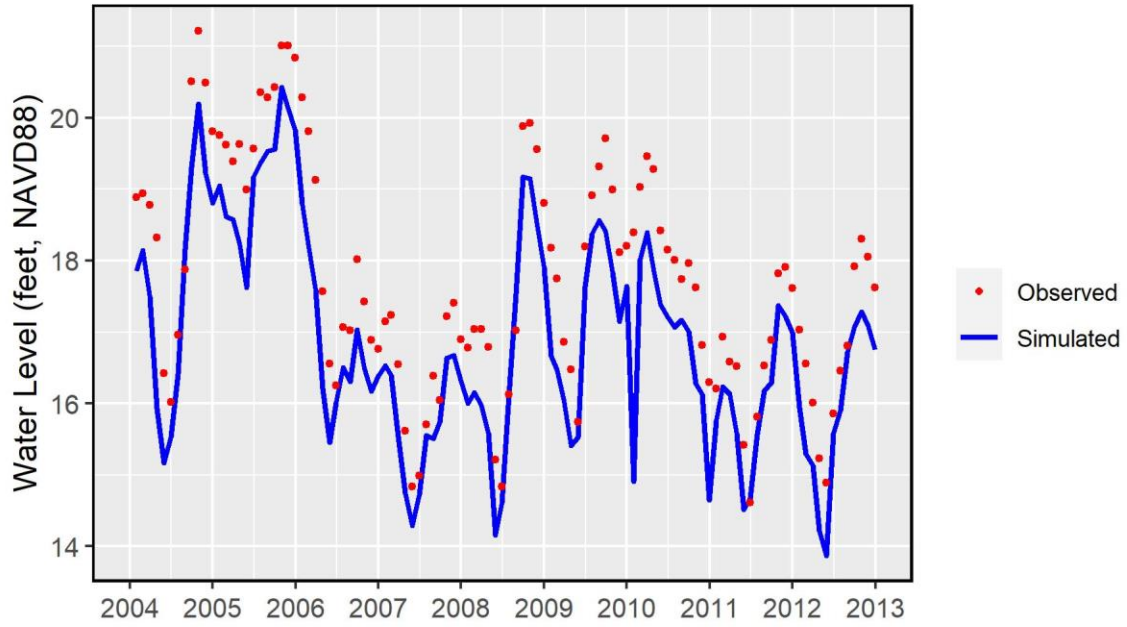
UFA: SJRWMD01840090
ME=1.001 MAE=1.038 nMAE=0.577 R2=0.758 NS=0.082



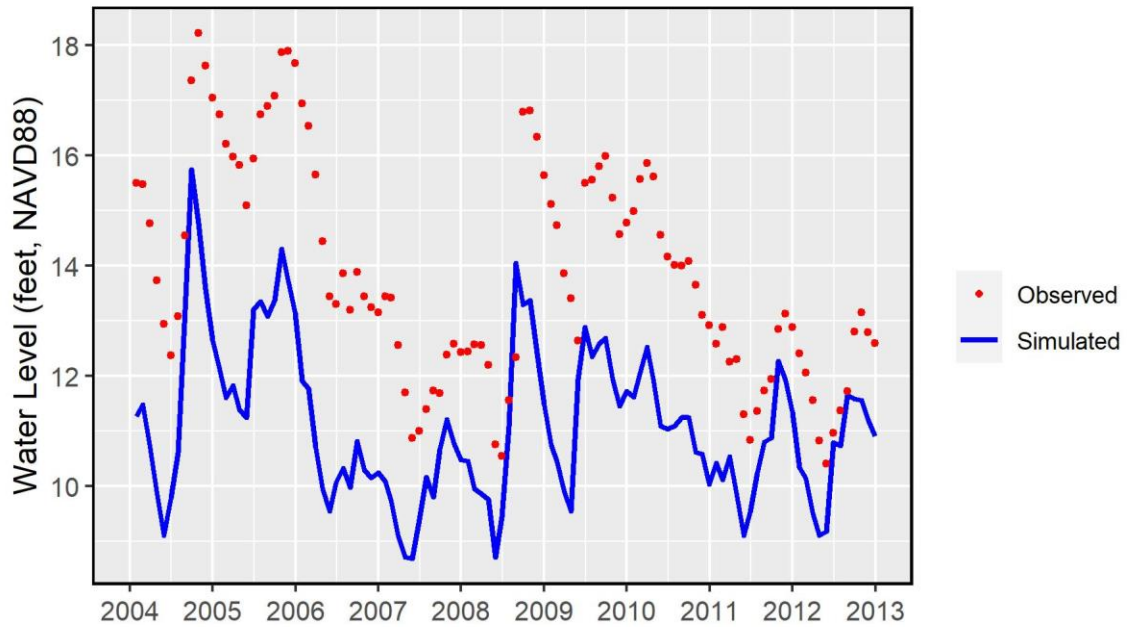
UFA: SJRWMD01850094
ME=0.202 MAE=0.488 nMAE=0.454 R2=0.89 NS=0.869



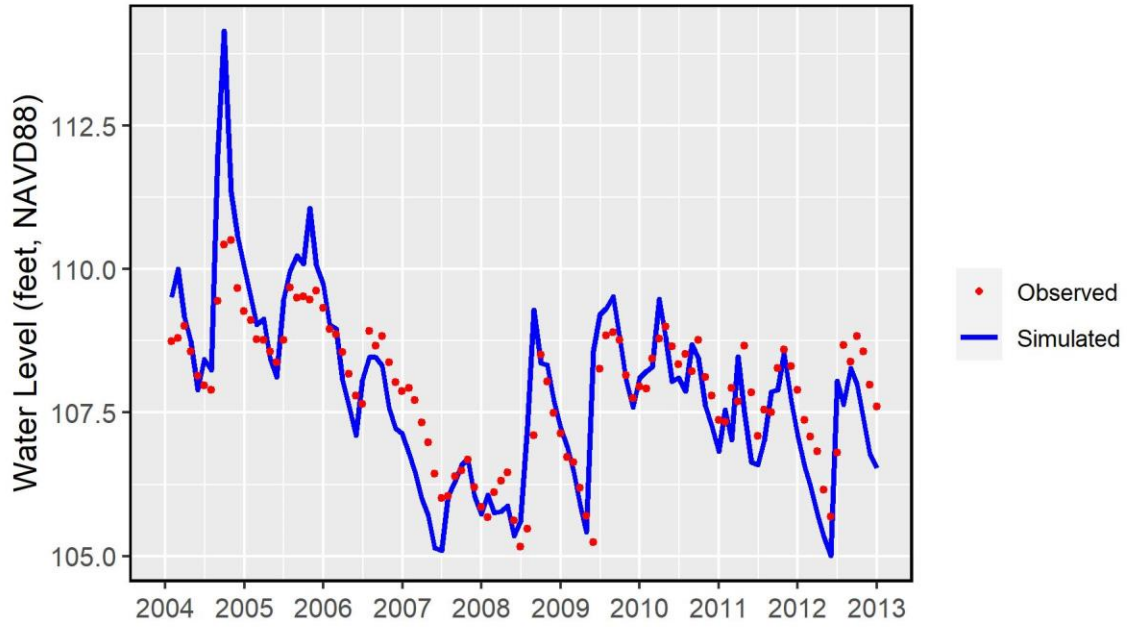
UFA: SJRWMD01850096
ME=-0.852 MAE=0.869 nMAE=0.342 R2=0.906 NS=0.623



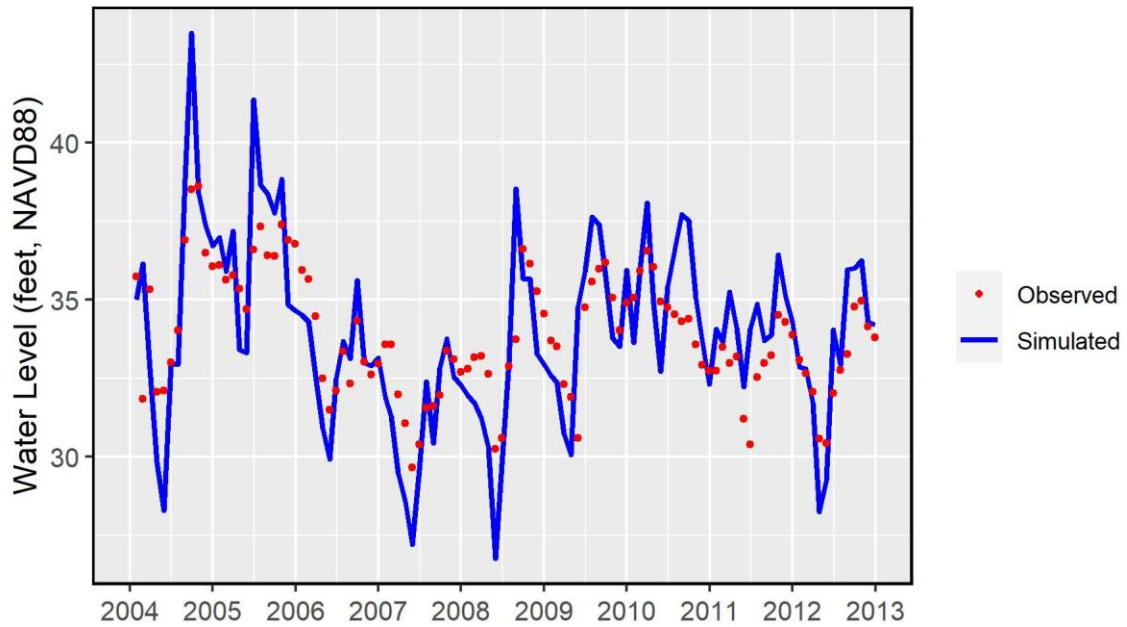
UFA: SJRWMD02751559
ME=-2.754 MAE=2.786 nMAE=1.037 R2=0.607 NS=-1.306



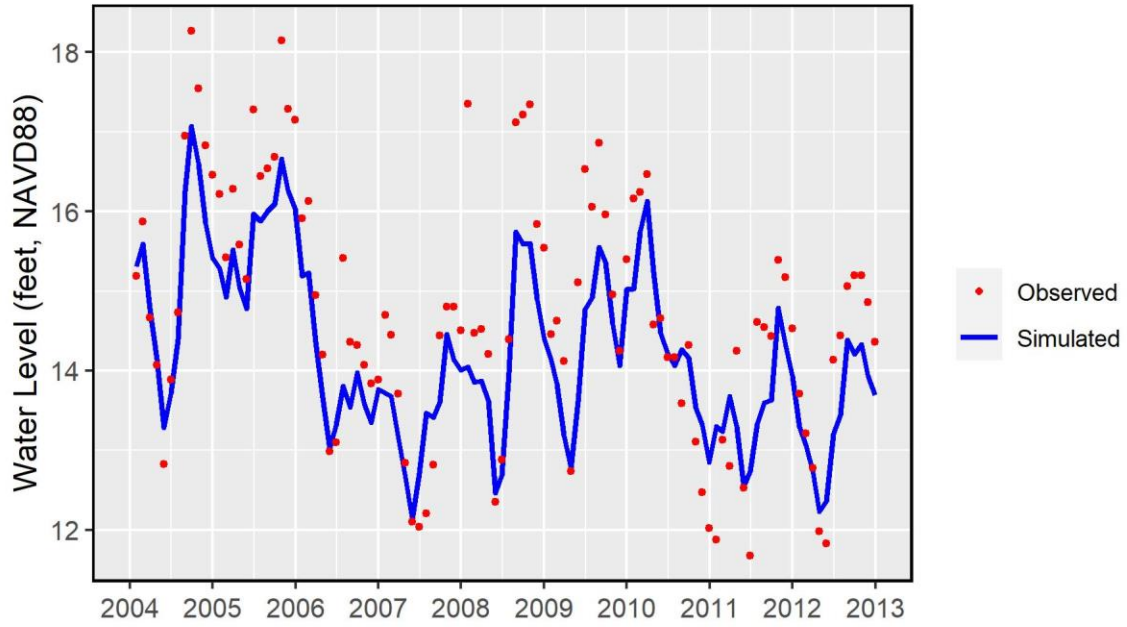
UFA: SJRWMD03242755
ME=-0.018 MAE=0.642 nMAE=0.64 R2=0.702 NS=0.428



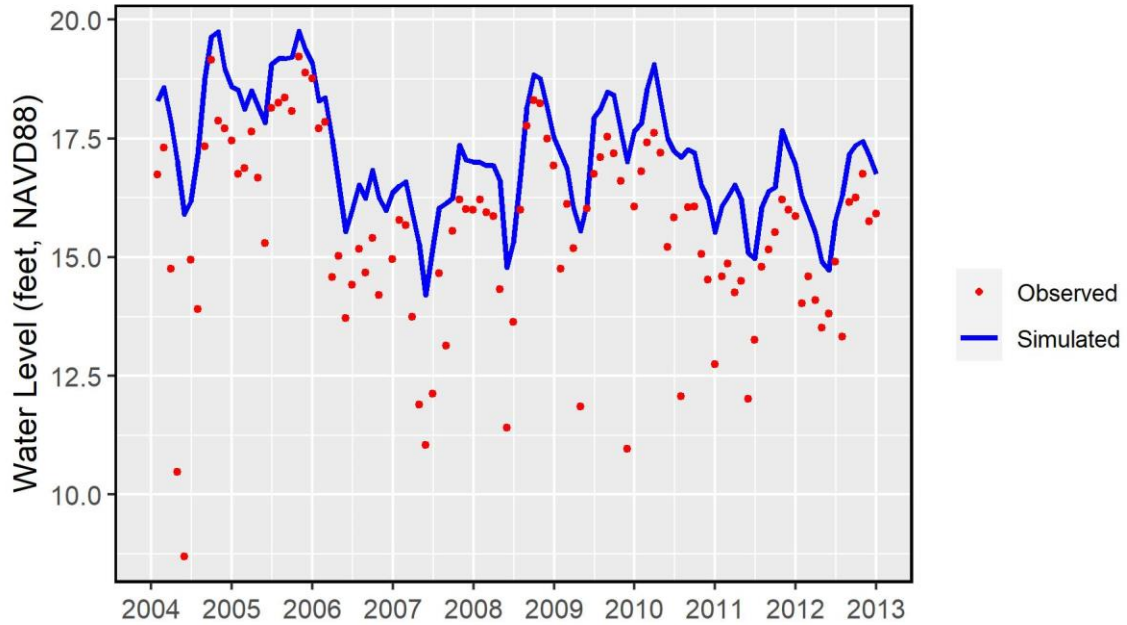
UFA: SJRWMD05321203
ME=0.092 MAE=1.456 nMAE=1.451 R2=0.619 NS=0.107



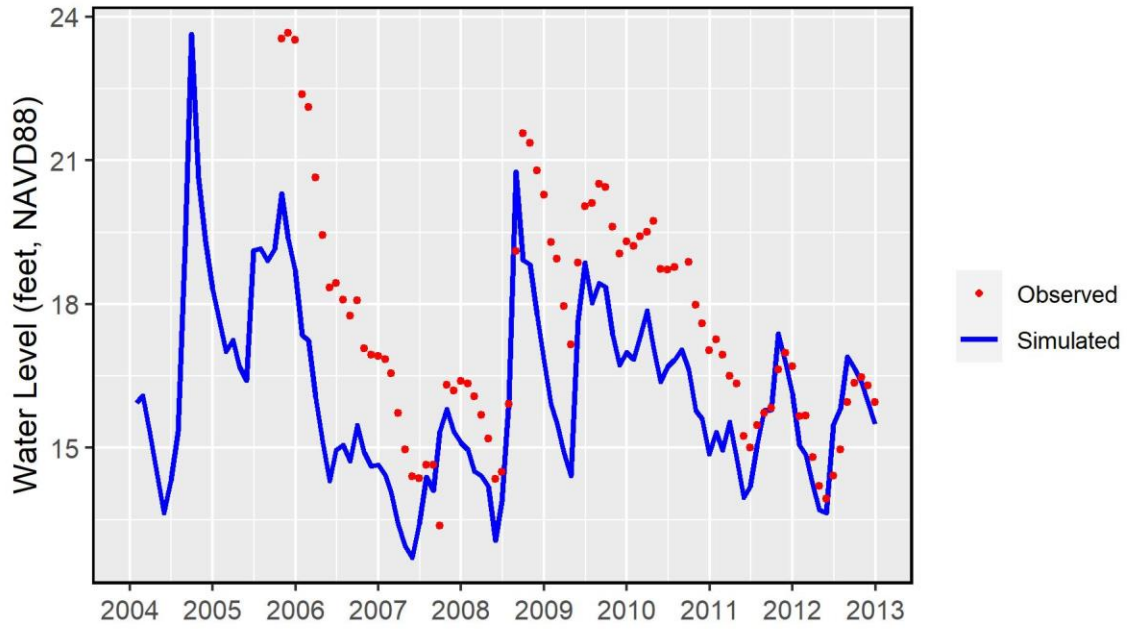
UFA: SJRWMD05521045
ME=-0.485 MAE=0.697 nMAE=0.525 R2=0.835 NS=0.696



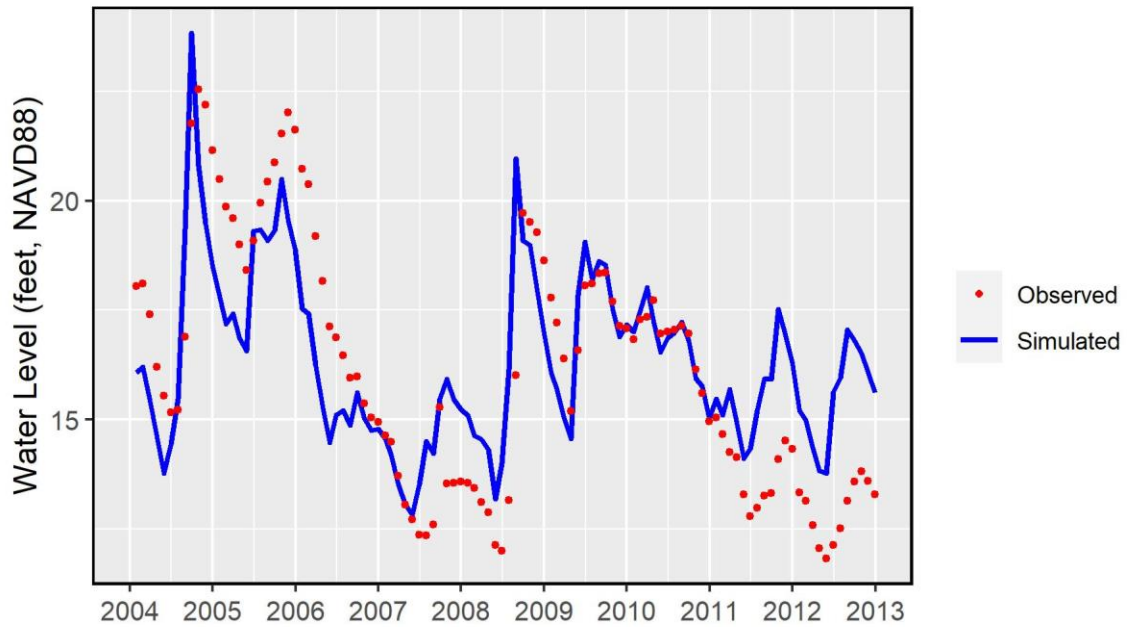
UFA: SJRWMD05592098
ME=1.619 MAE=1.619 nMAE=0.805 R2=0.688 NS=-0.002



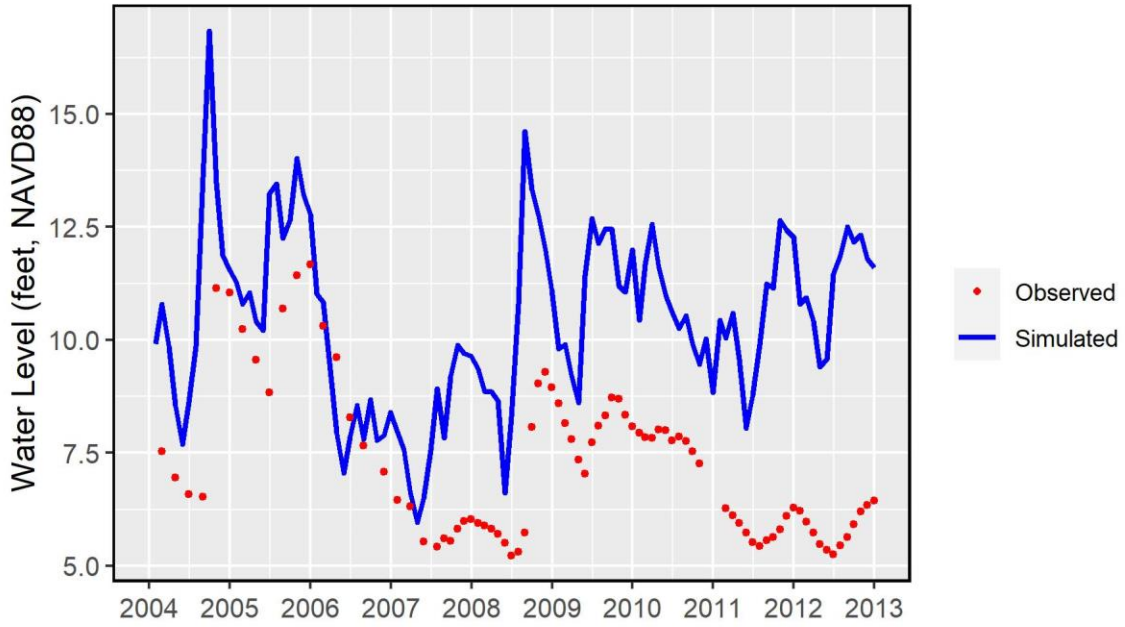
UFA: SJRWMD05701075
ME=-1.694 MAE=1.874 nMAE=1.173 R2=0.63 NS=0.113



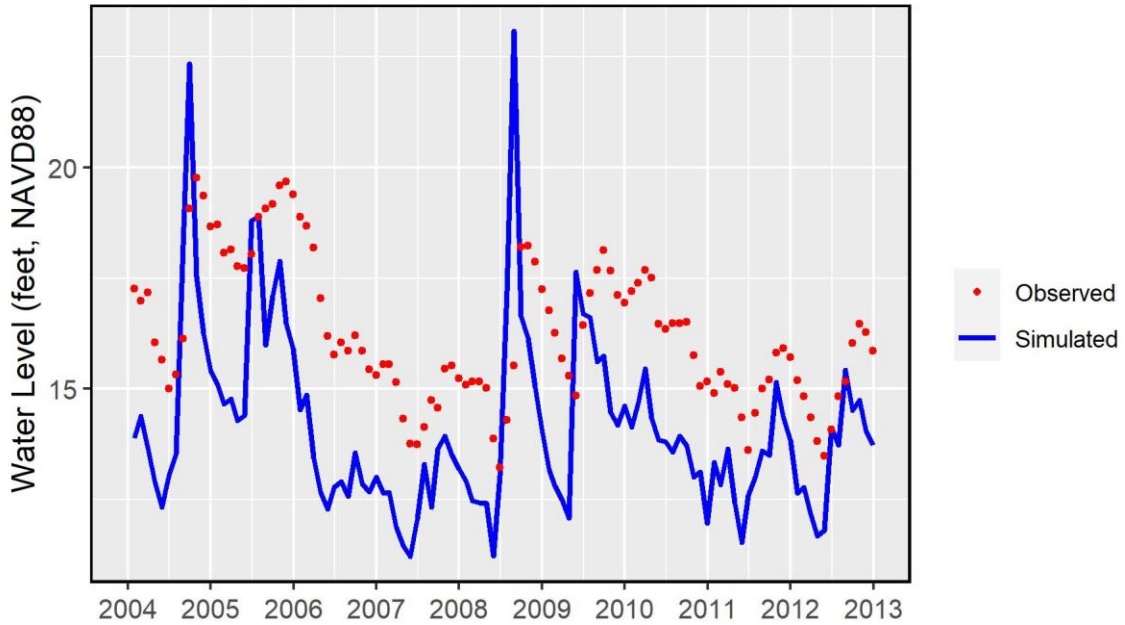
UFA: SJRWMD05701077
ME=0.172 MAE=1.468 nMAE=1.469 R2=0.598 NS=0.587



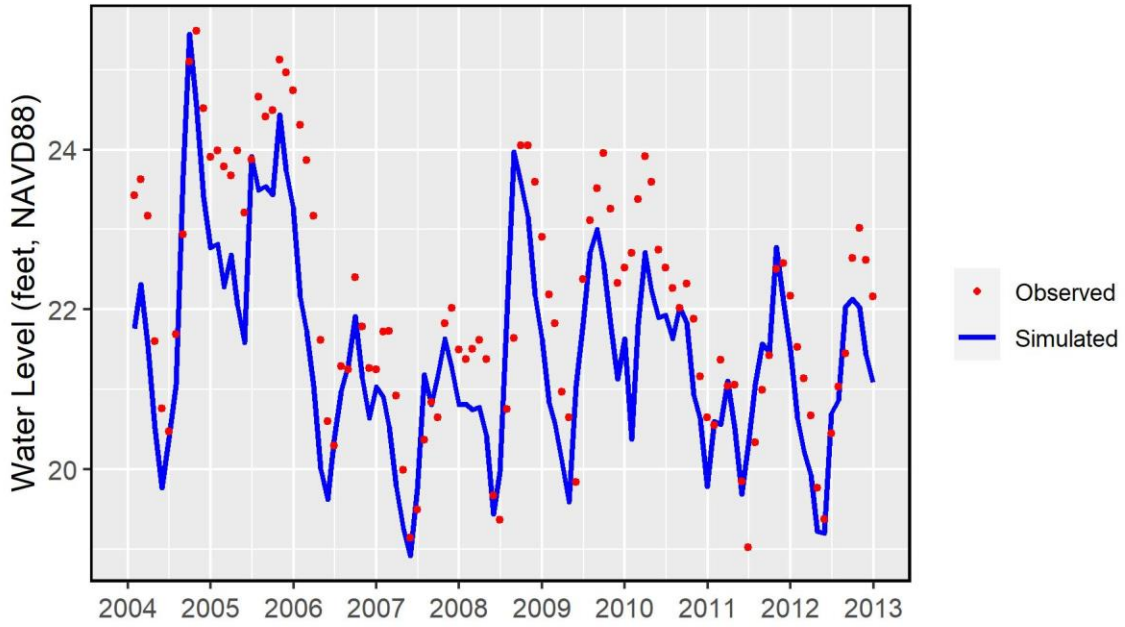
UFA: SJRWMD05781086
ME=3.359 MAE=3.41 nMAE=1.555 R2=0.136 NS=-4.544



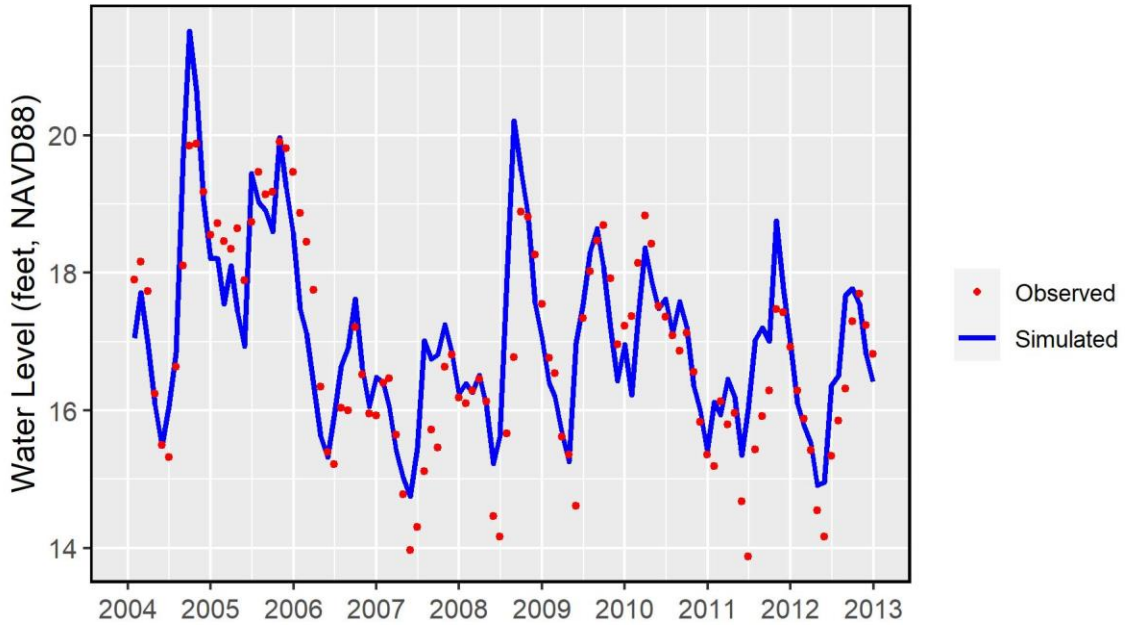
UFA: SJRWMD05791087
ME=-2.117 MAE=2.488 nMAE=1.091 R2=0.354 NS=-1.866



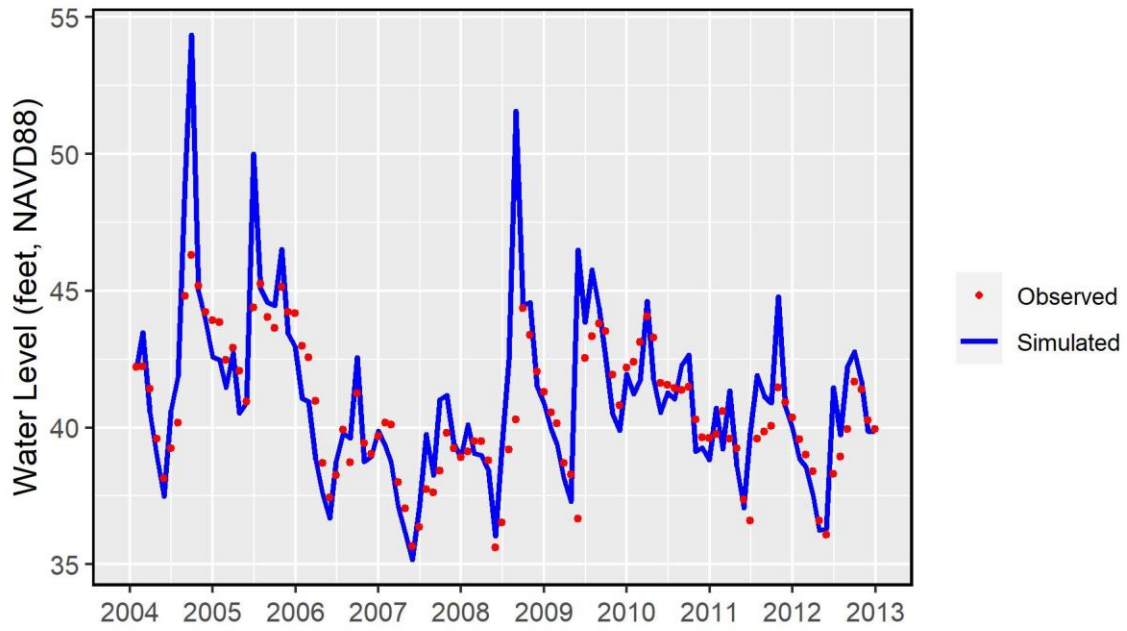
UFA: SJRWMD09532141
ME=-0.638 MAE=0.86 nMAE=0.598 R2=0.731 NS=0.554



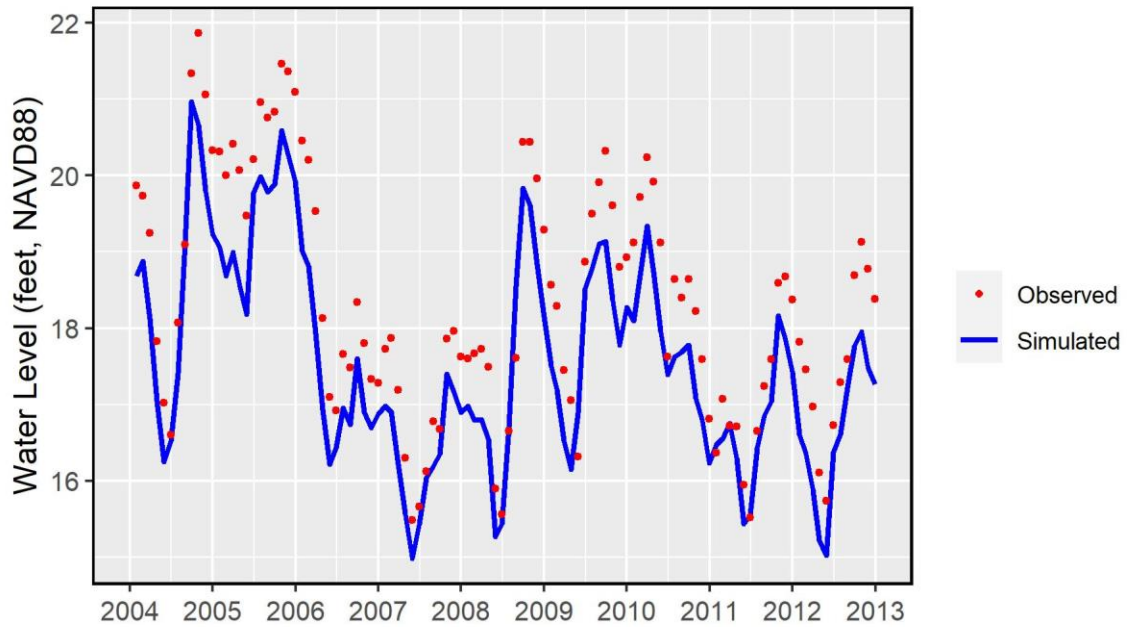
UFA: SJRWMD09532142
ME=0.191 MAE=0.628 nMAE=0.637 R2=0.684 NS=0.665



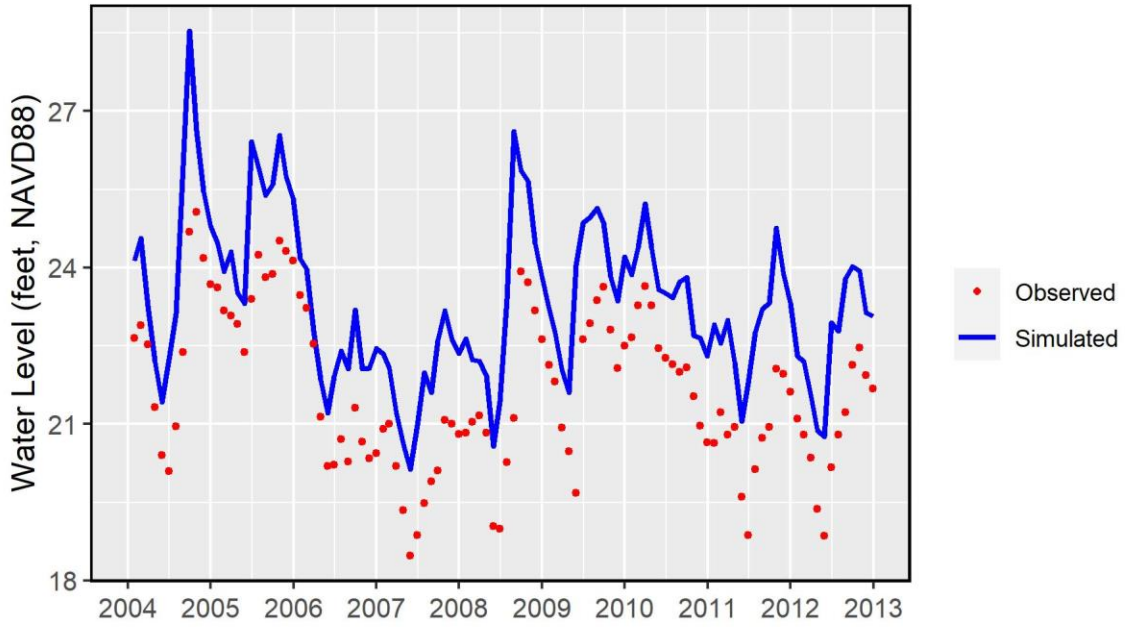
UFA: SJRWMD09991029
ME=0.441 MAE=1.291 nMAE=1.381 R2=0.556 NS=0.204



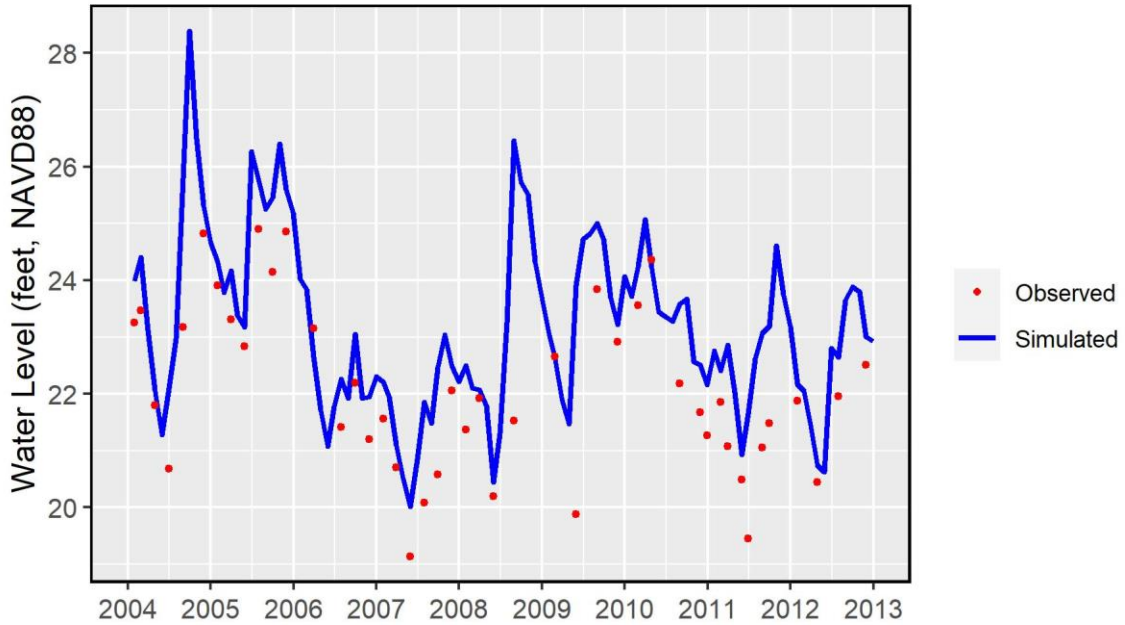
UFA: SJRWMD10291593
ME=-0.771 MAE=0.803 nMAE=0.333 R2=0.934 NS=0.684



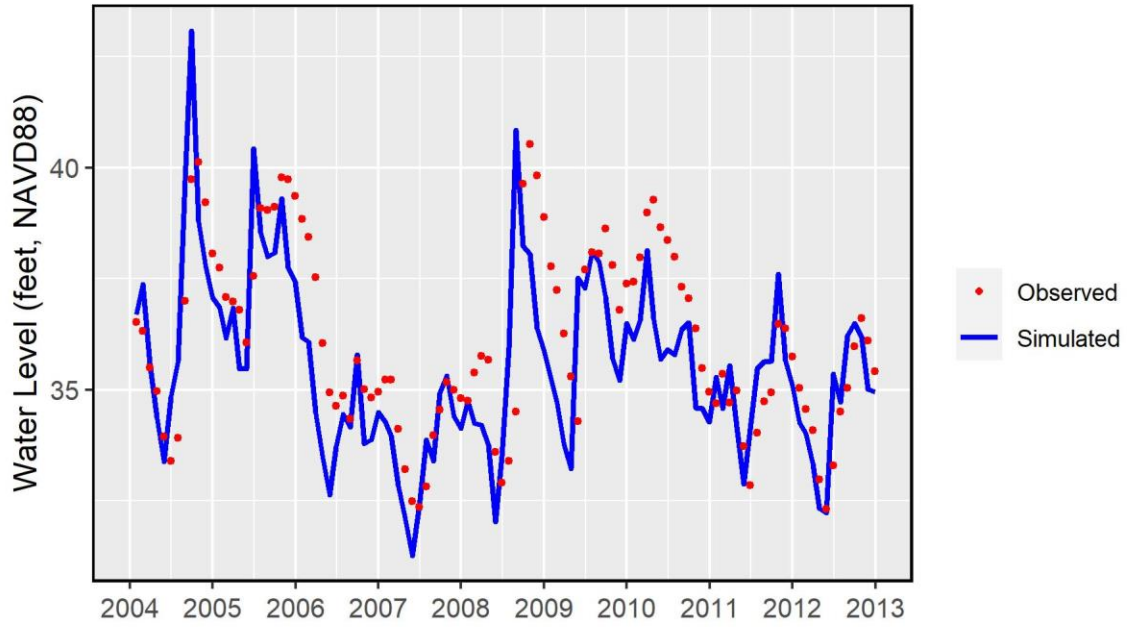
UFA: SJRWMD11501468
ME=1.675 MAE=1.675 nMAE=0.546 R2=0.762 NS=-0.49



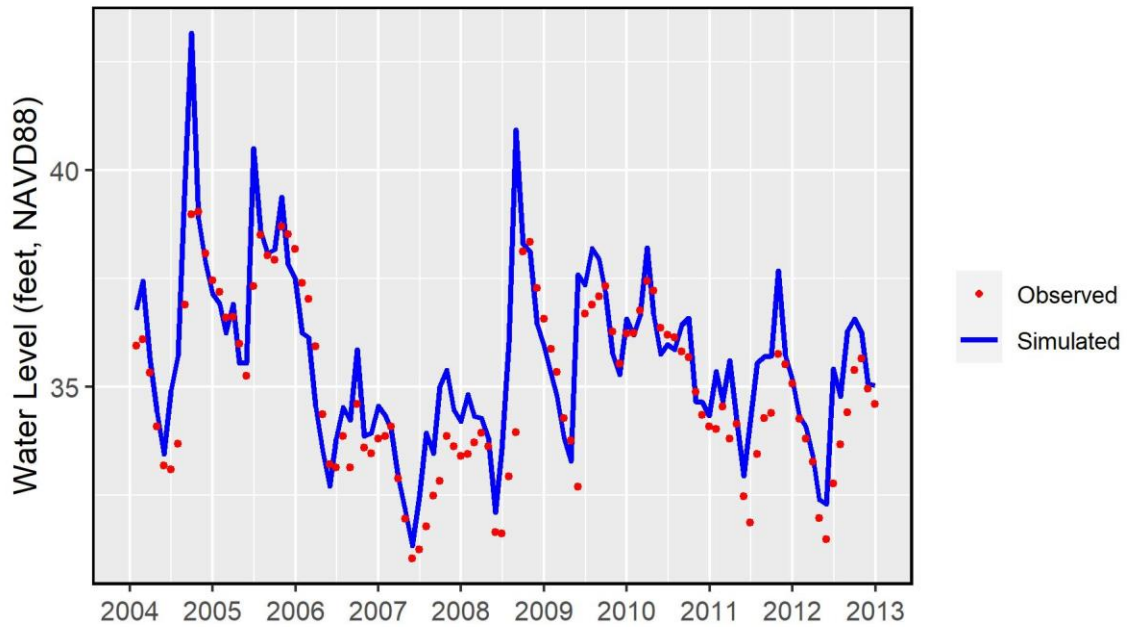
UFA: SJRWMD11502677
ME=1.001 MAE=1.03 nMAE=0.681 R2=0.602 NS=0.069



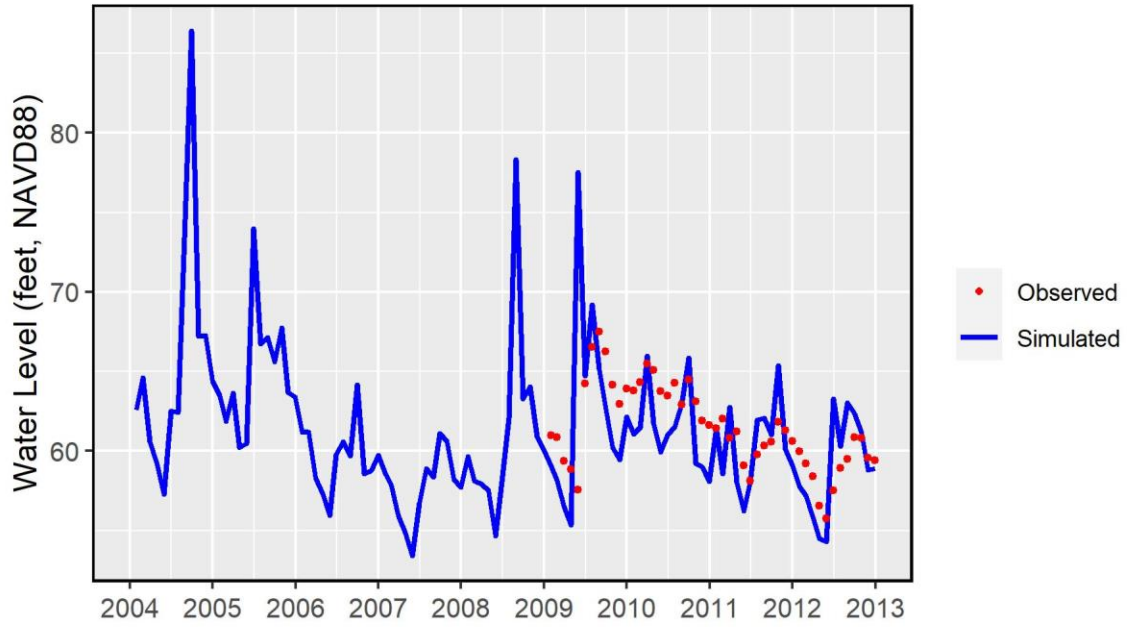
UFA: SJRWMD14652678
ME=-0.575 MAE=1.292 nMAE=1.084 R2=0.513 NS=0.368



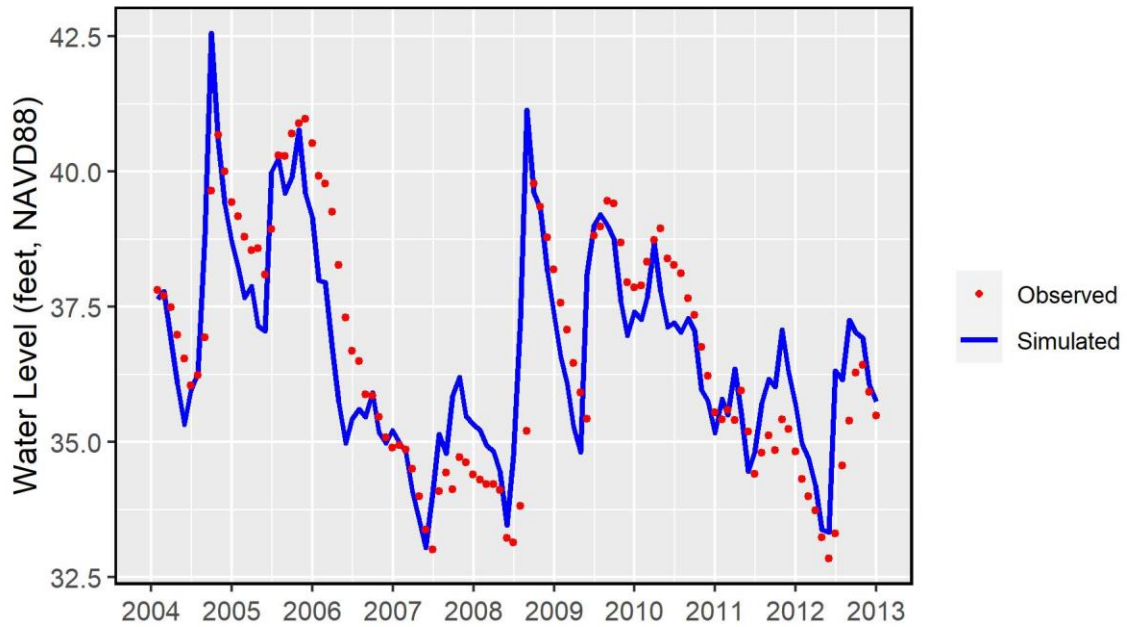
UFA: SJRWMD14652681
ME=0.641 MAE=0.91 nMAE=0.867 R2=0.652 NS=0.504



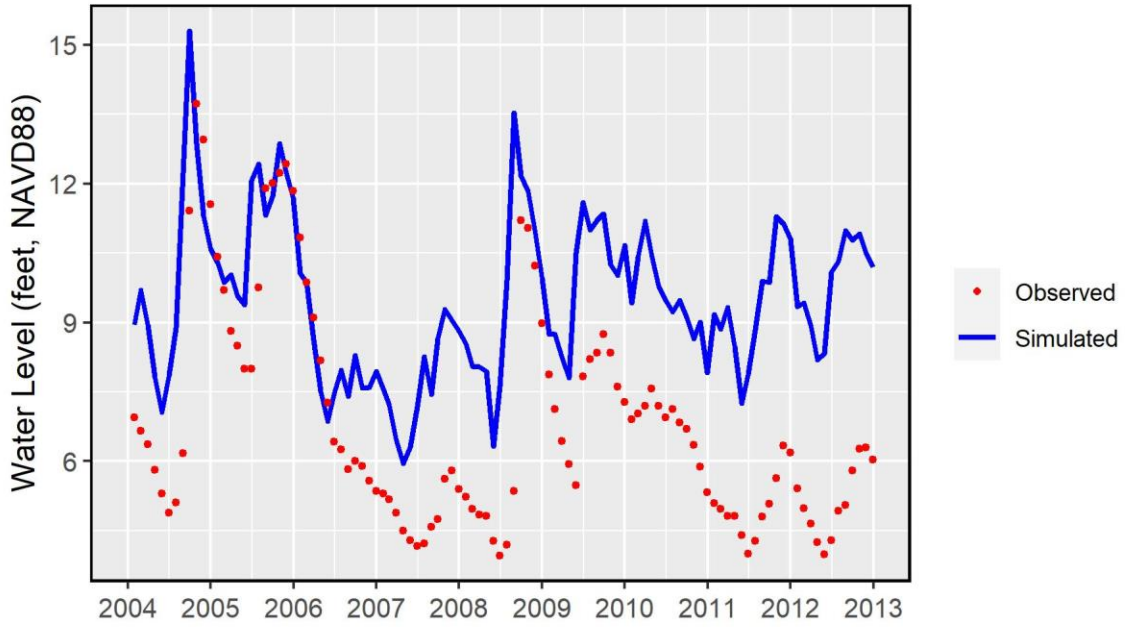
UFA: SJRWMD15282844
ME=-0.647 MAE=2.615 nMAE=2.455 R2=0.155 NS=-1.085



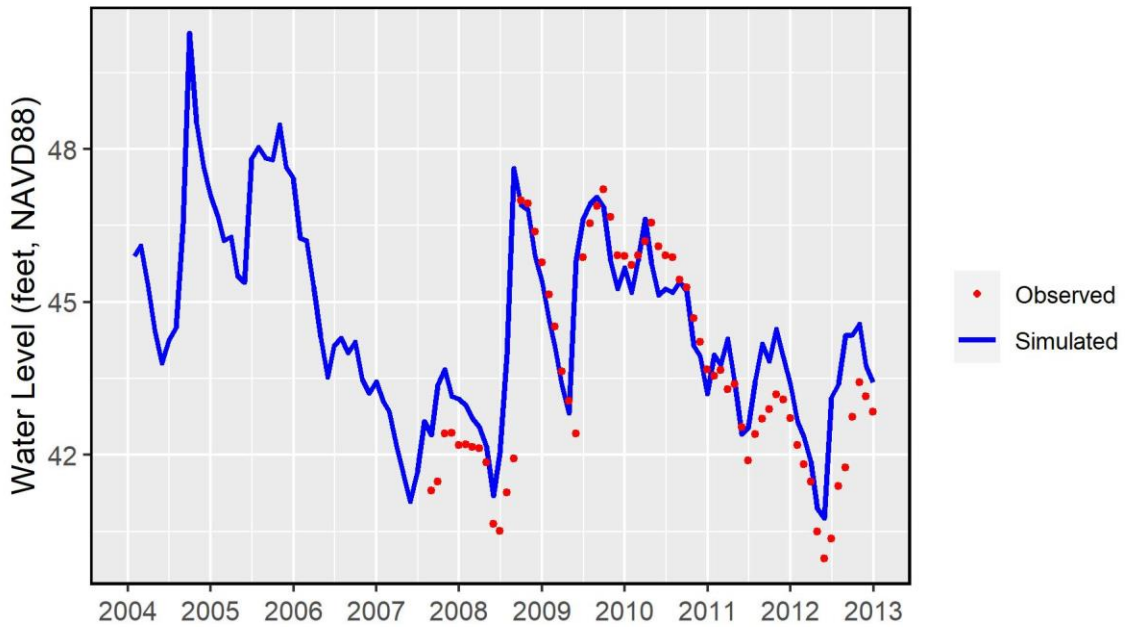
UFA: SJRWMD17043609
ME=0.014 MAE=0.908 nMAE=0.91 R2=0.678 NS=0.676



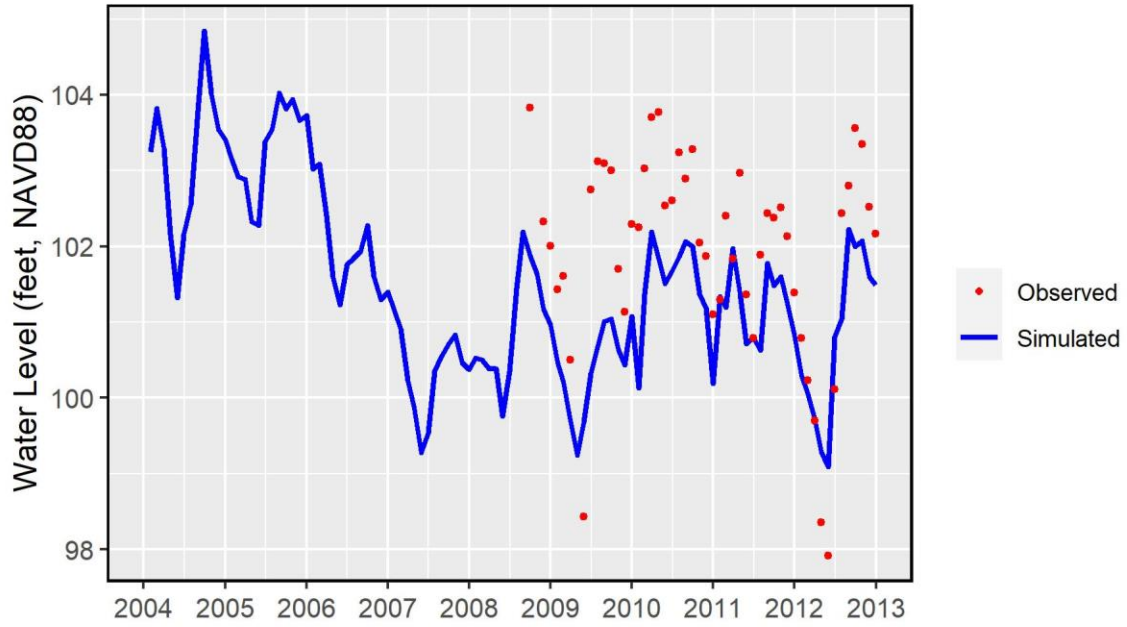
UFA: SJRWMD17453563
ME=2.656 MAE=2.788 nMAE=1.43 R2=0.419 NS=-0.856



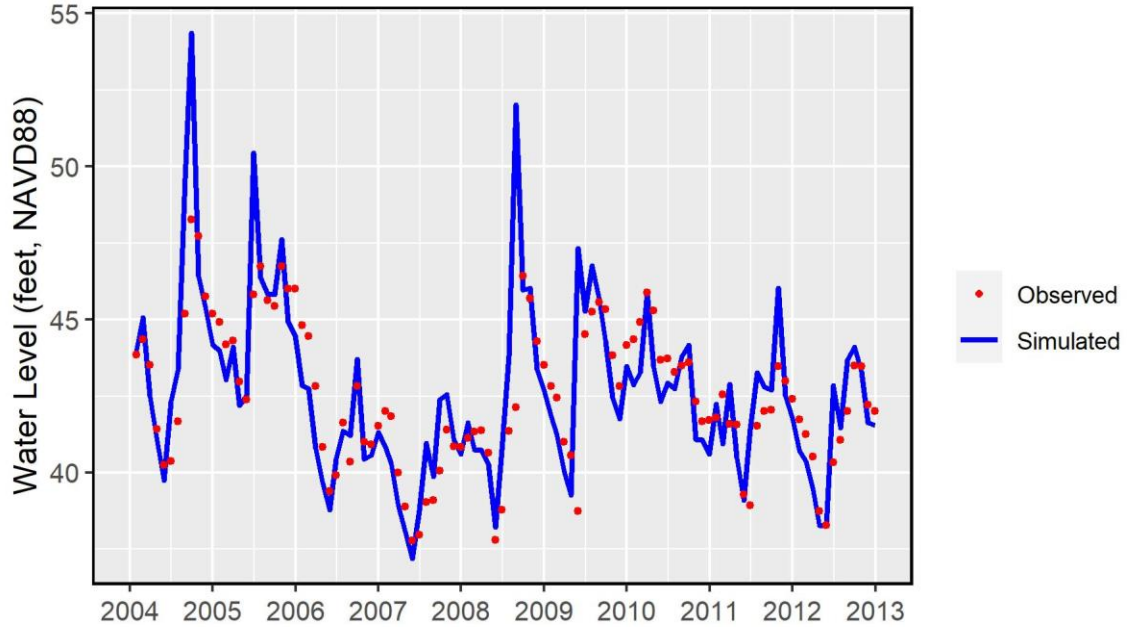
UFA: SJRWMD19414941
ME=0.555 MAE=0.858 nMAE=0.791 R2=0.683 NS=0.605



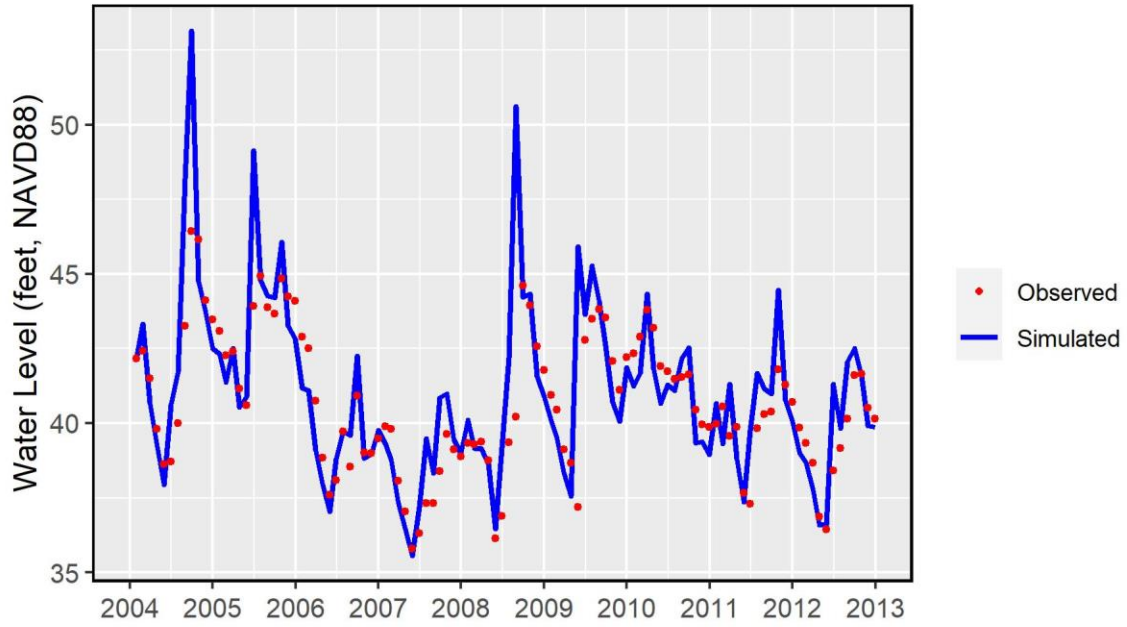
UFA: SJRWMD19535110
ME=-0.911 MAE=1.081 nMAE=0.601 R2=0.682 NS=0.158



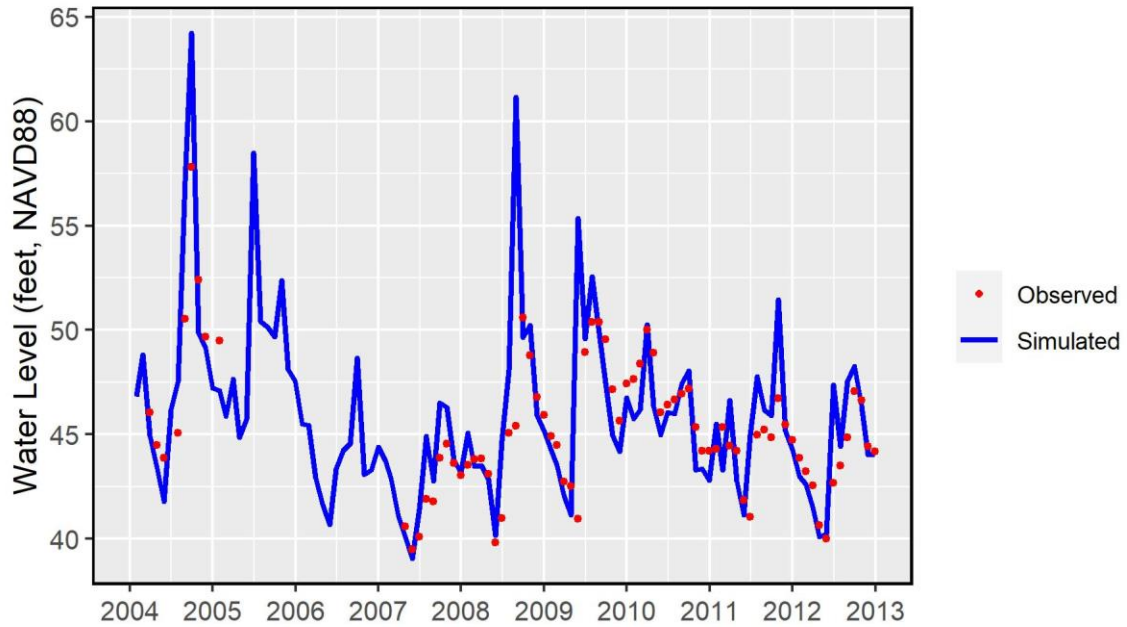
UFA: SJRWMD22752271
ME=0.152 MAE=1.203 nMAE=1.235 R2=0.571 NS=0.353



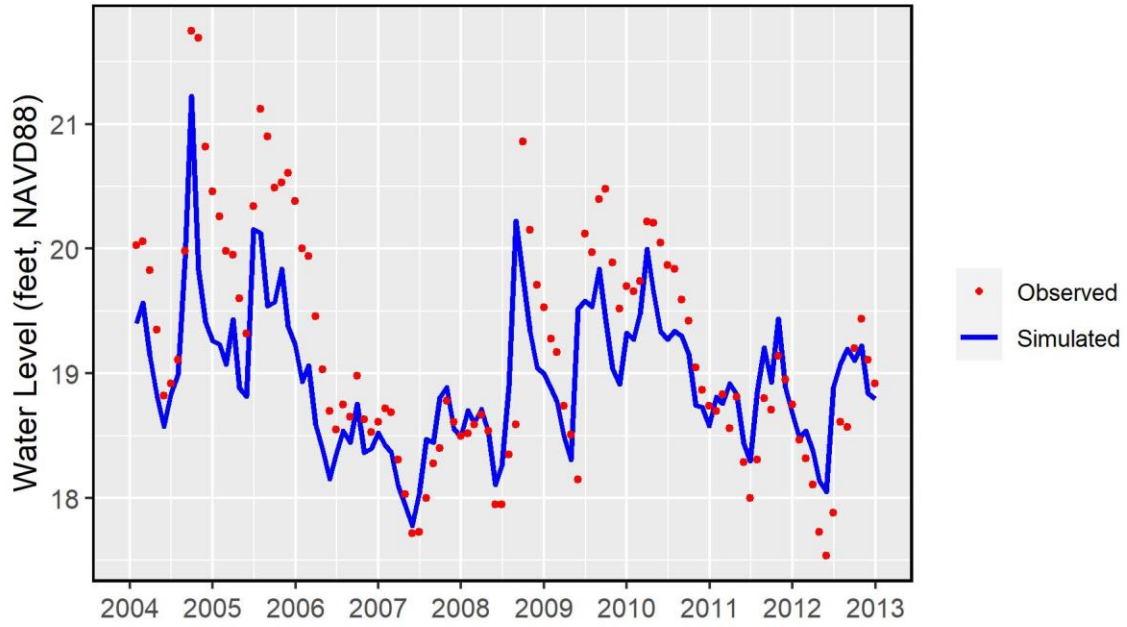
UFA: SJRWMD22752276
ME=0.356 MAE=1.191 nMAE=1.257 R2=0.566 NS=0.299



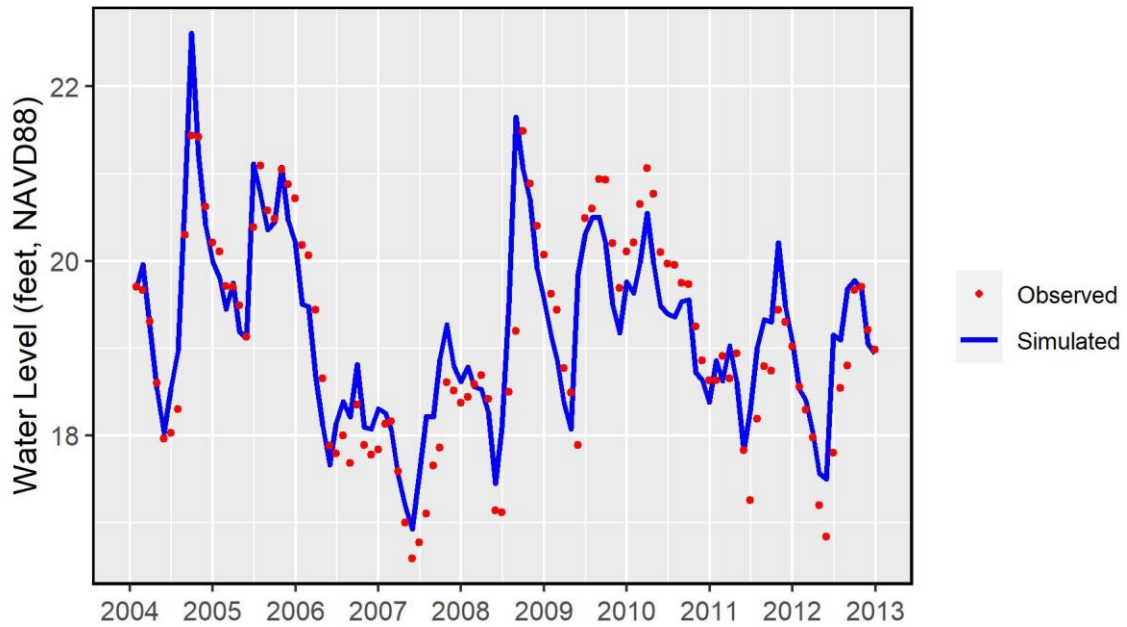
UFA: SJRWMD29674177
ME=0.638 MAE=1.839 nMAE=1.978 R2=0.496 NS=0.051



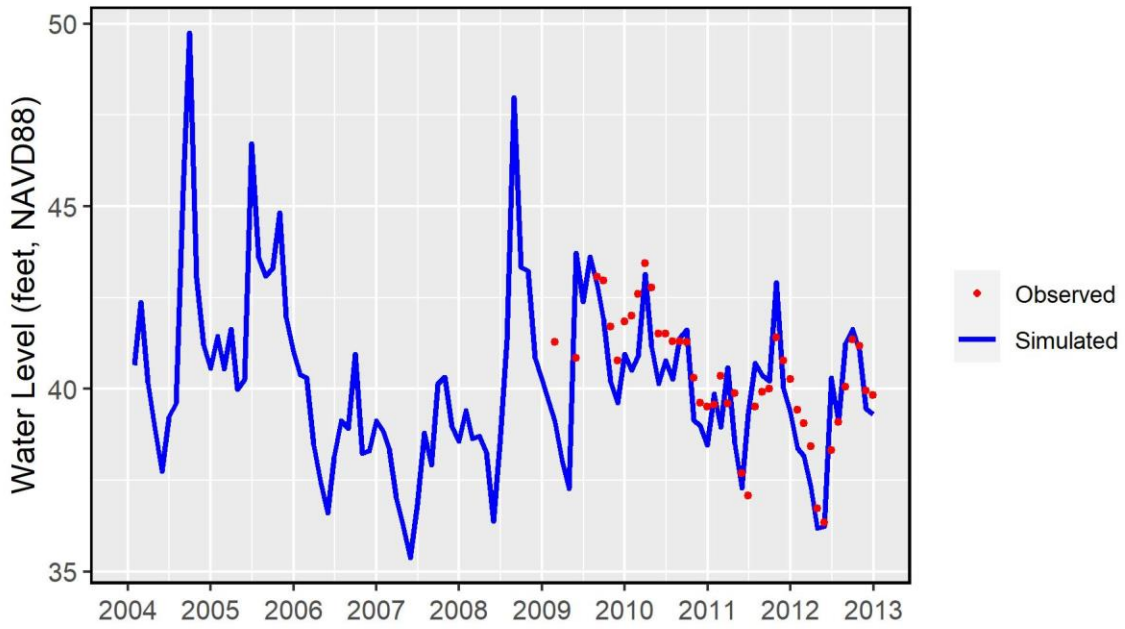
UFA: SJRWMD30063052
ME=-0.247 MAE=0.464 nMAE=0.413 R2=0.663 NS=0.553



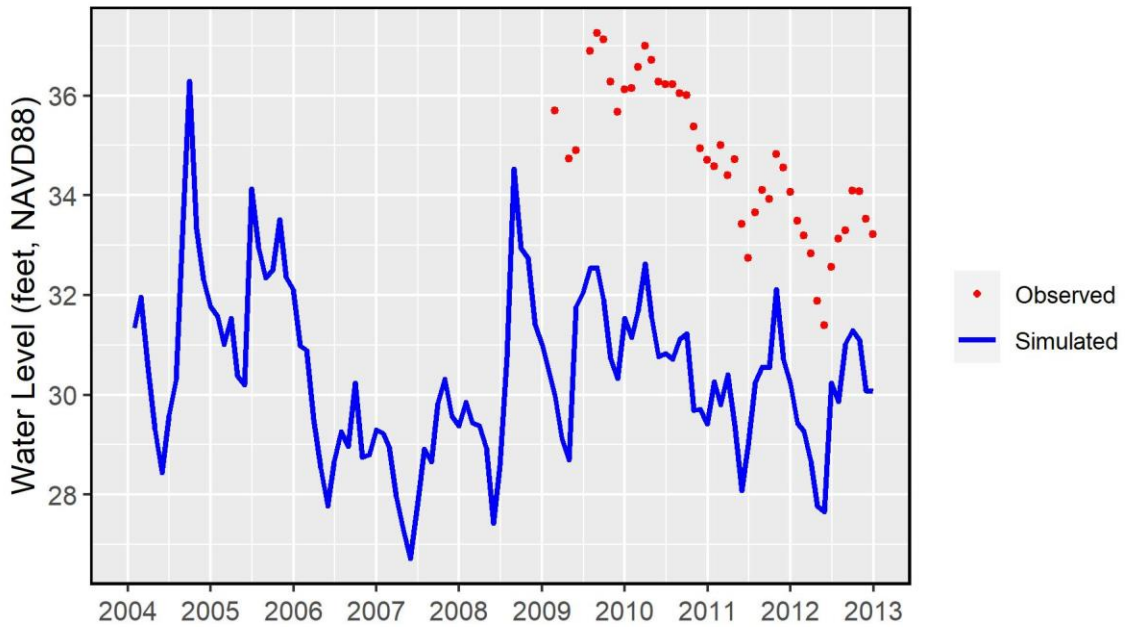
UFA: SJRWMD30072619
ME=0.076 MAE=0.437 nMAE=0.446 R2=0.772 NS=0.768



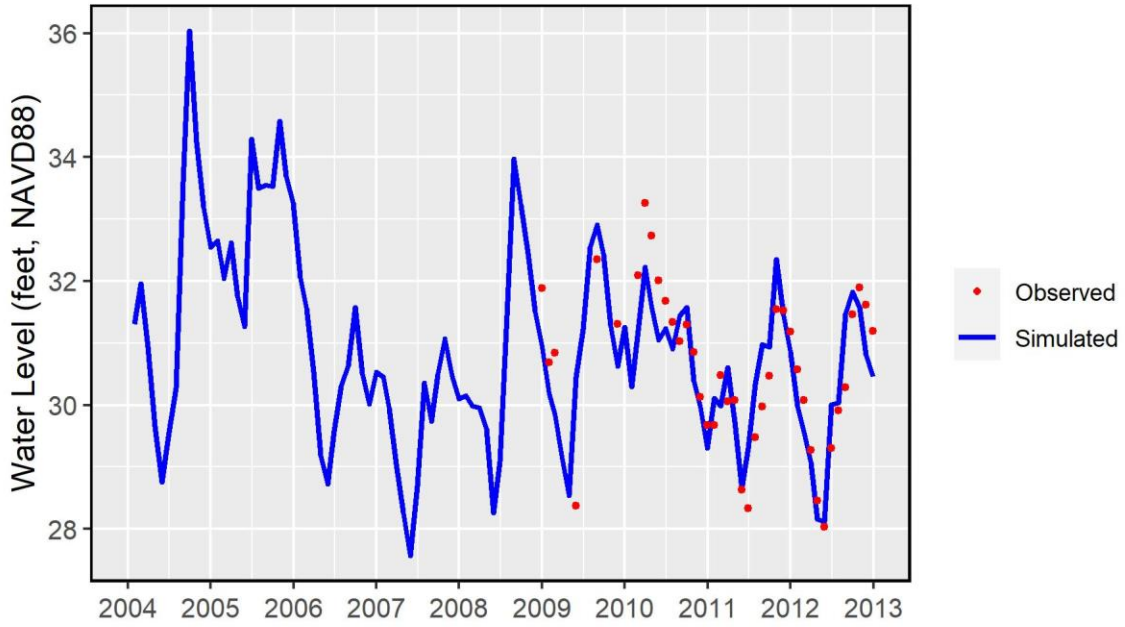
UFA: SJRWMD30332850
ME=-0.33 MAE=0.965 nMAE=0.874 R2=0.597 NS=0.494



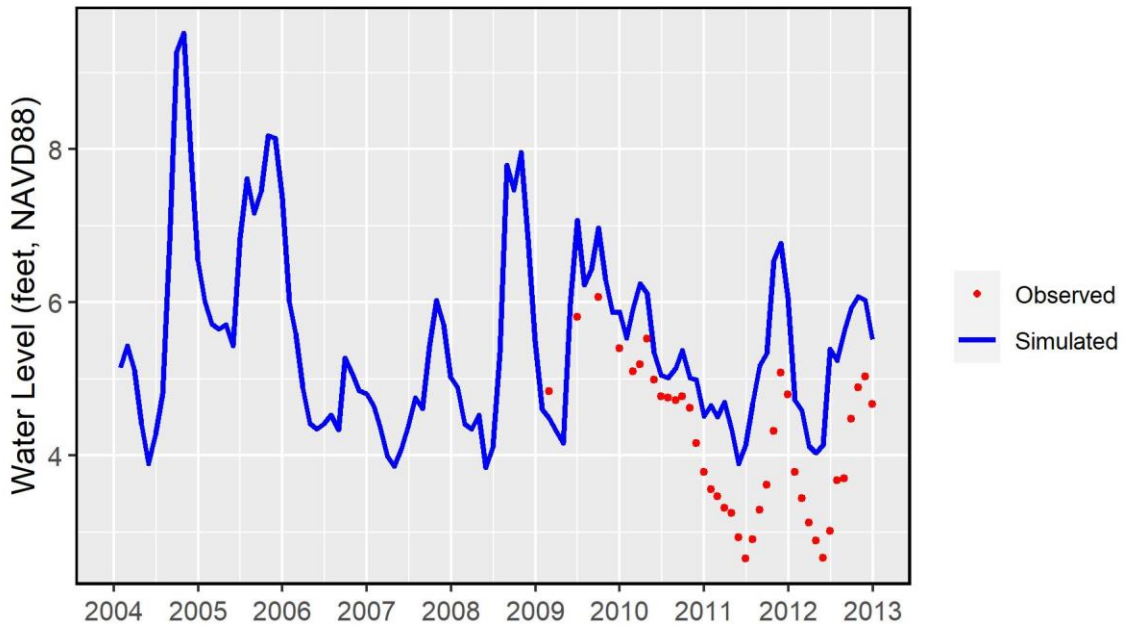
UFA: SJRWMD30342858
ME=-4.341 MAE=4.341 nMAE=0.859 R2=0.548 NS=-8.169



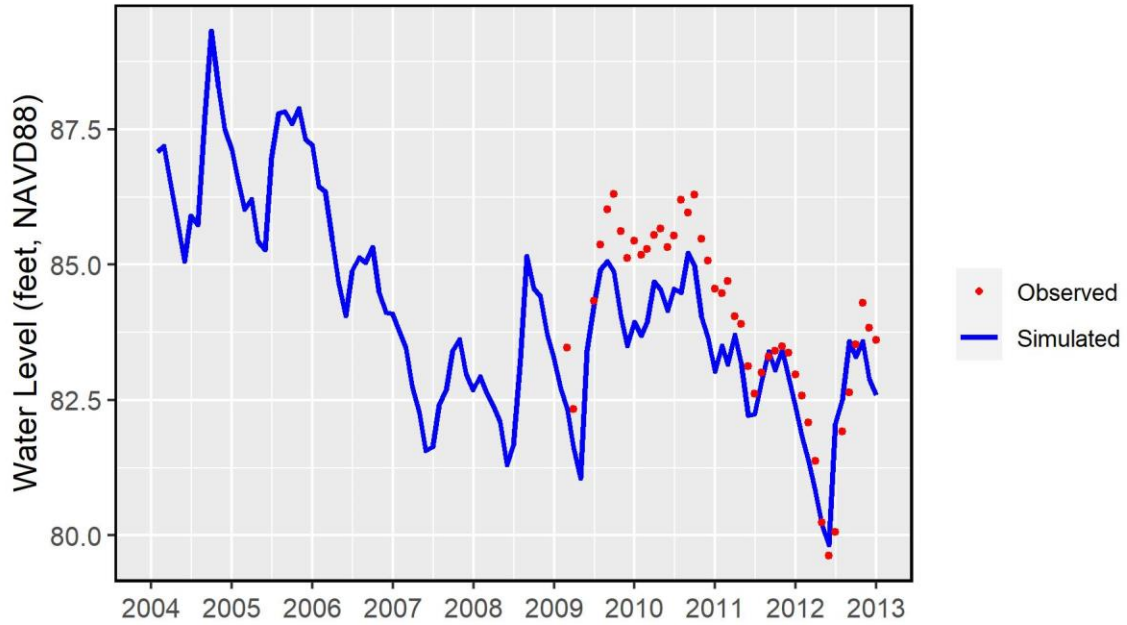
UFA: SJRWMD30362864
ME=-0.068 MAE=0.601 nMAE=0.59 R2=0.672 NS=0.668



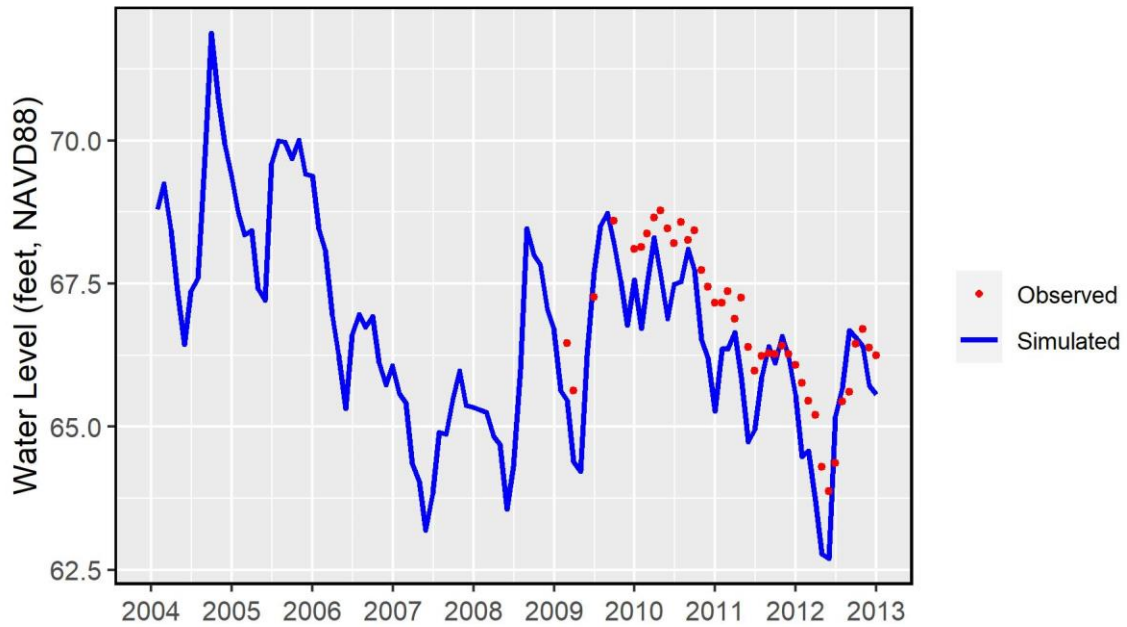
UFA: SJRWMD30432902
ME=1.084 MAE=1.102 nMAE=0.438 R2=0.637 NS=-0.736



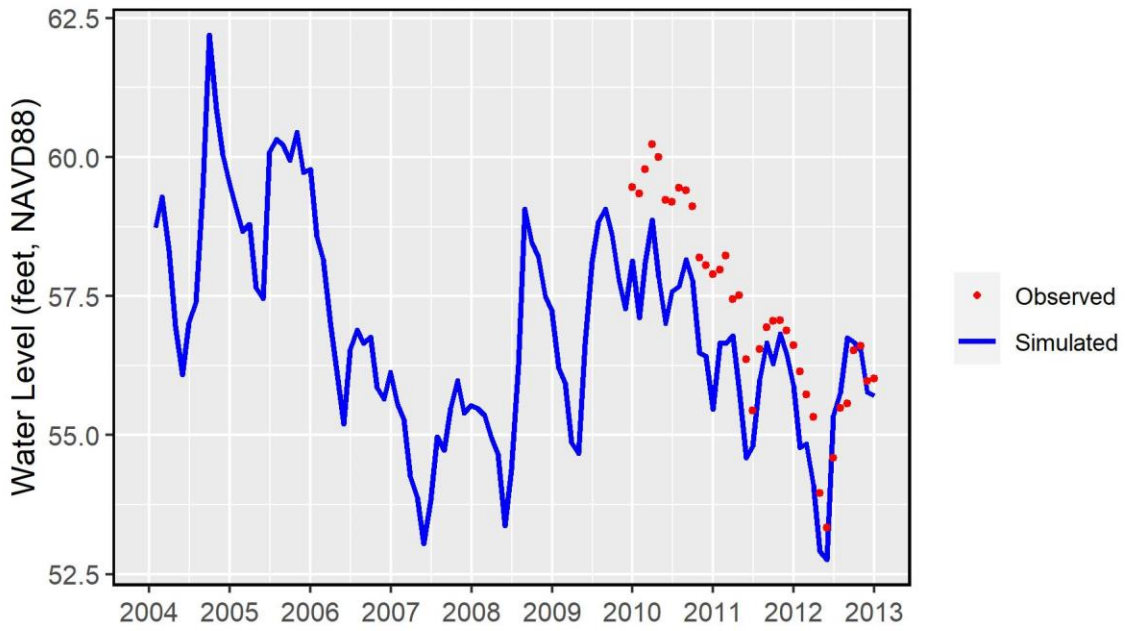
UFA: SJRWMD30442913
ME=-0.716 MAE=0.886 nMAE=0.547 R2=0.837 NS=0.624



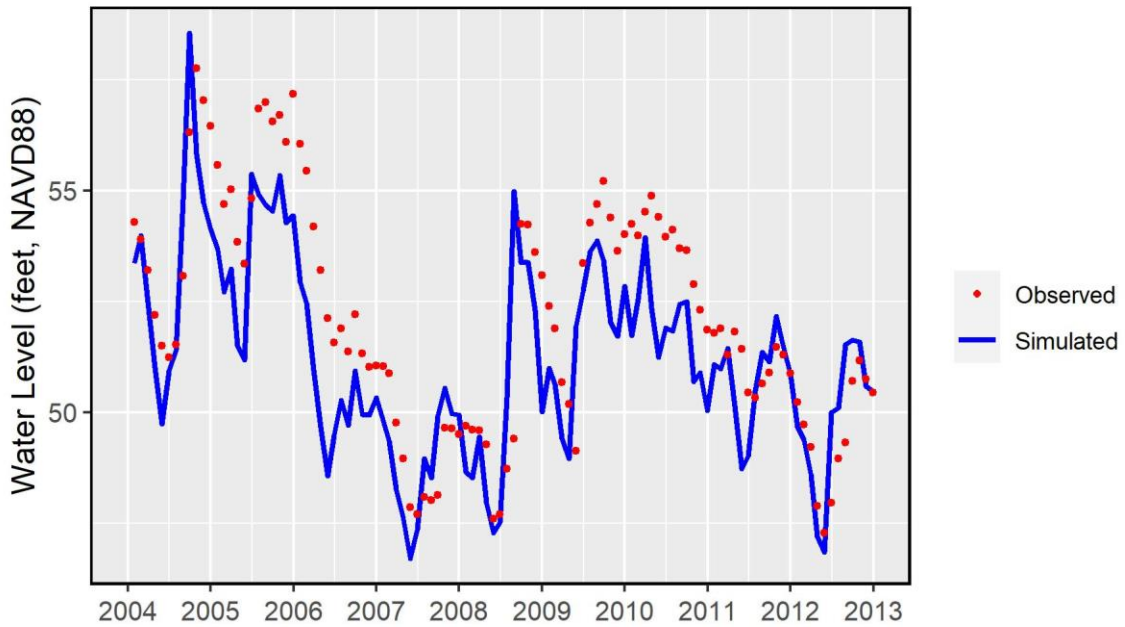
UFA: SJRWMD30452912
ME=-0.677 MAE=0.822 nMAE=0.556 R2=0.75 NS=0.426



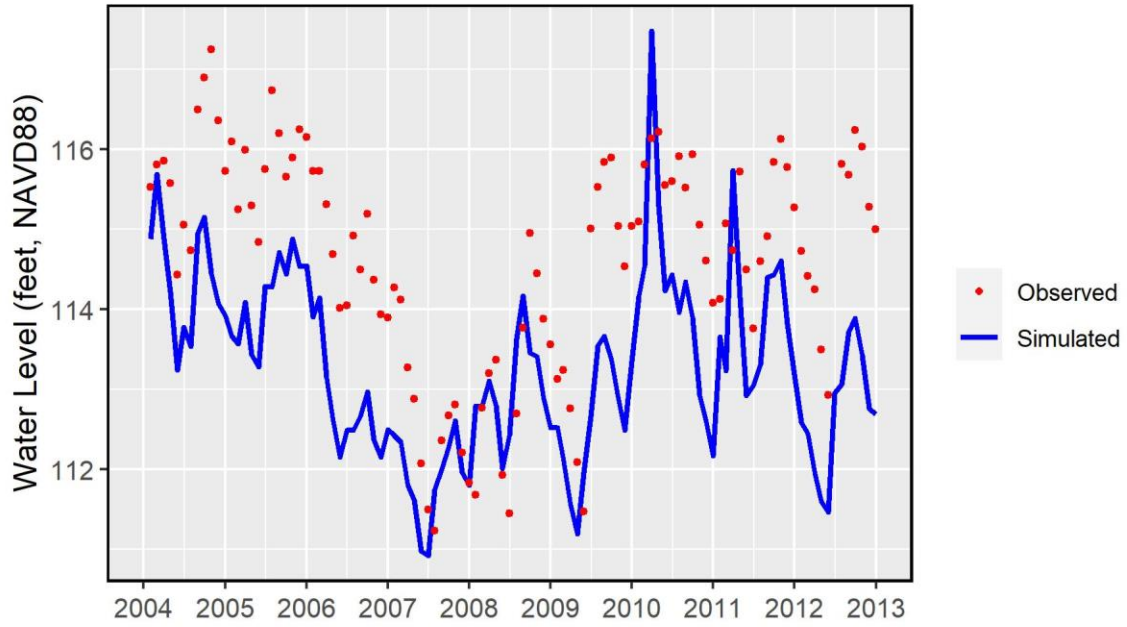
UFA: SJRWMD31043397
ME=-0.992 MAE=1.12 nMAE=0.696 R2=0.779 NS=0.444



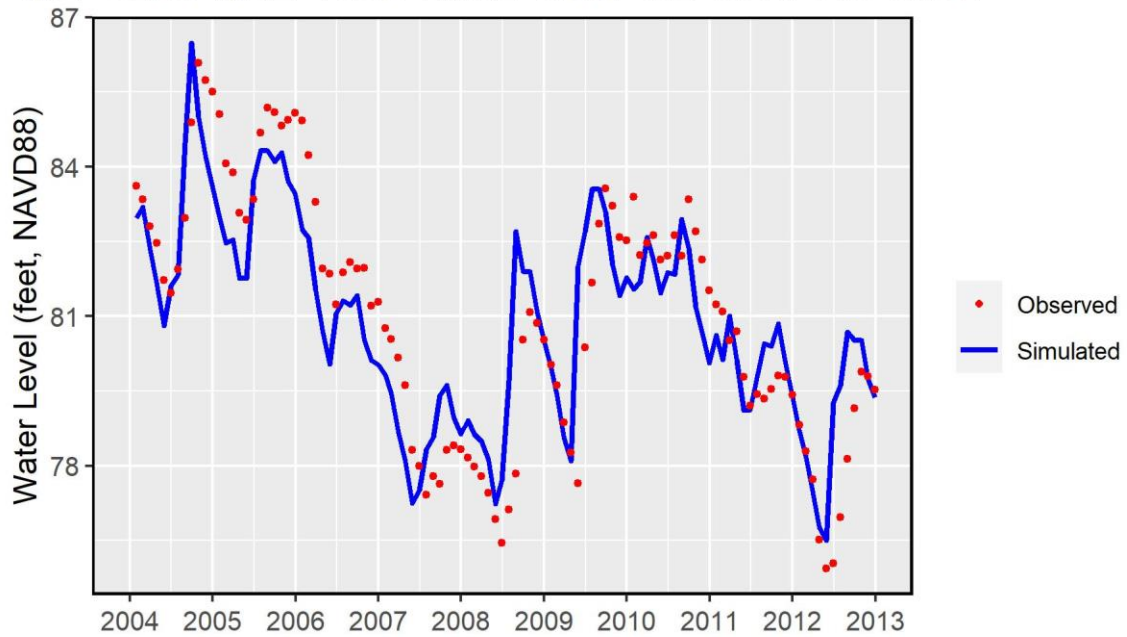
UFA: SJRWMD51005099
ME=-0.893 MAE=1.417 nMAE=1.102 R2=0.688 NS=0.57



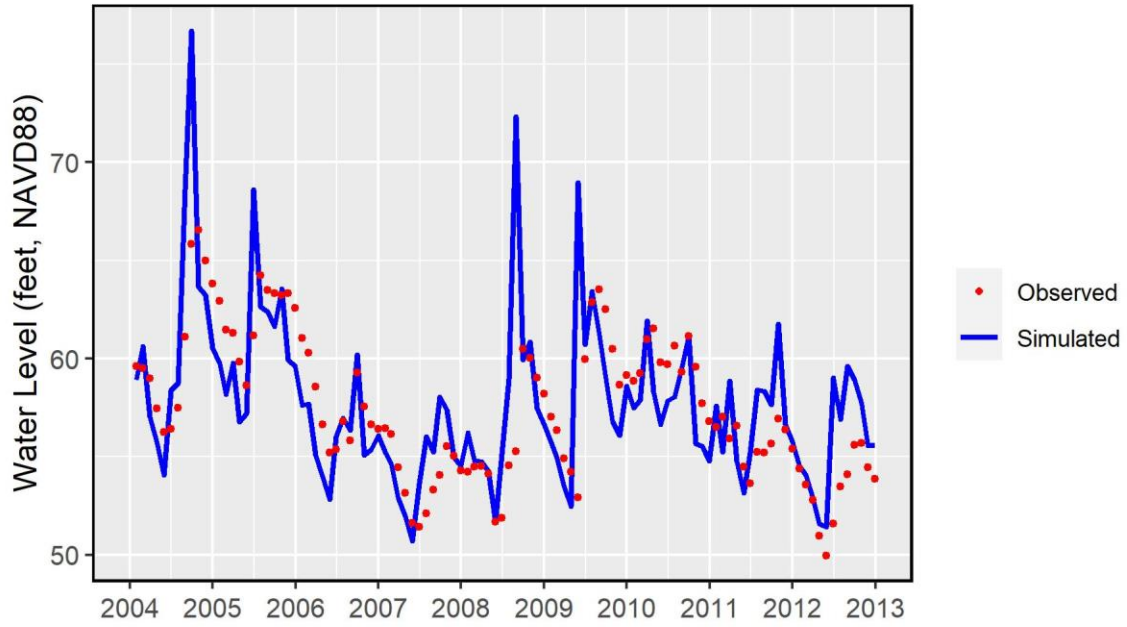
UFA: USGS282241081443901
ME=-1.331 MAE=1.46 nMAE=0.704 R2=0.585 NS=-0.318



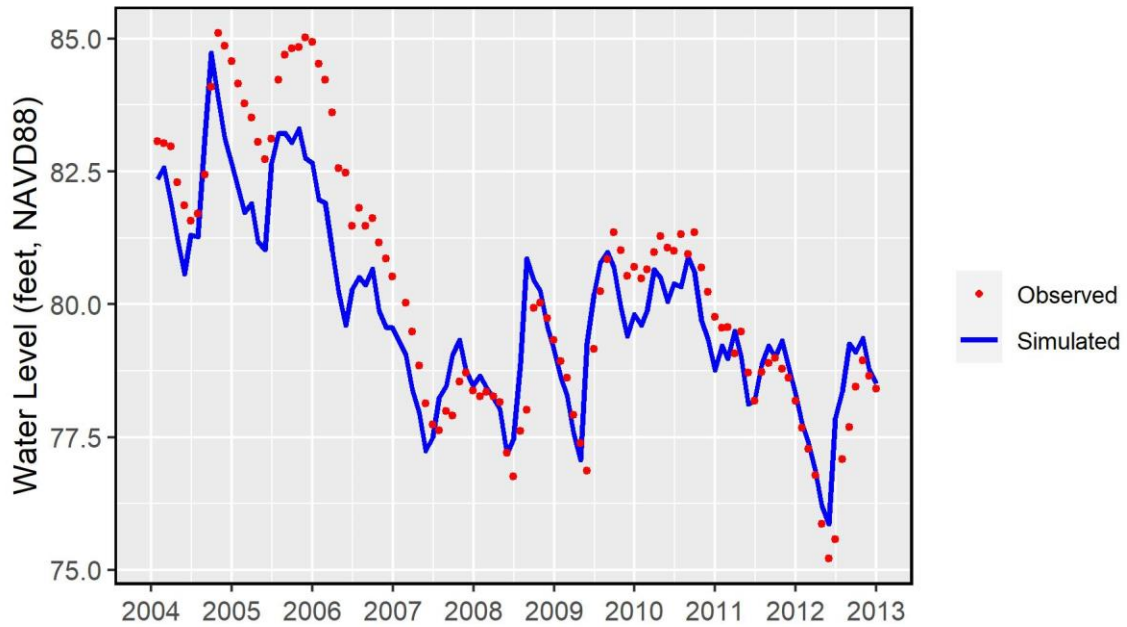
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ME=-0.103 MAE=1.009 nMAE=0.987 R2=0.737 NS=0.732



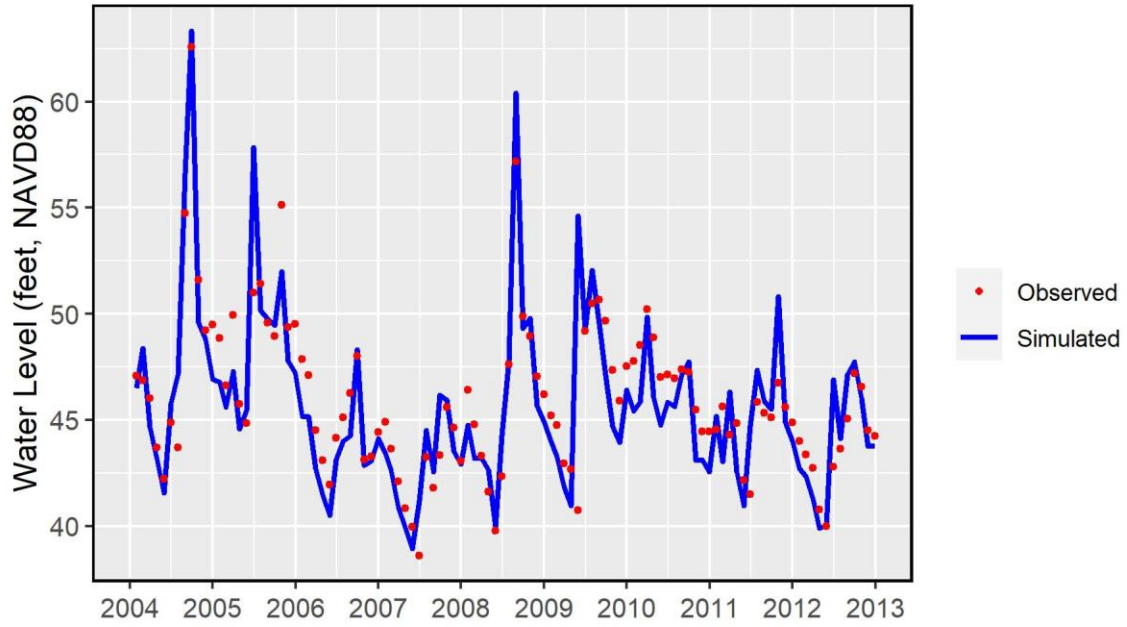
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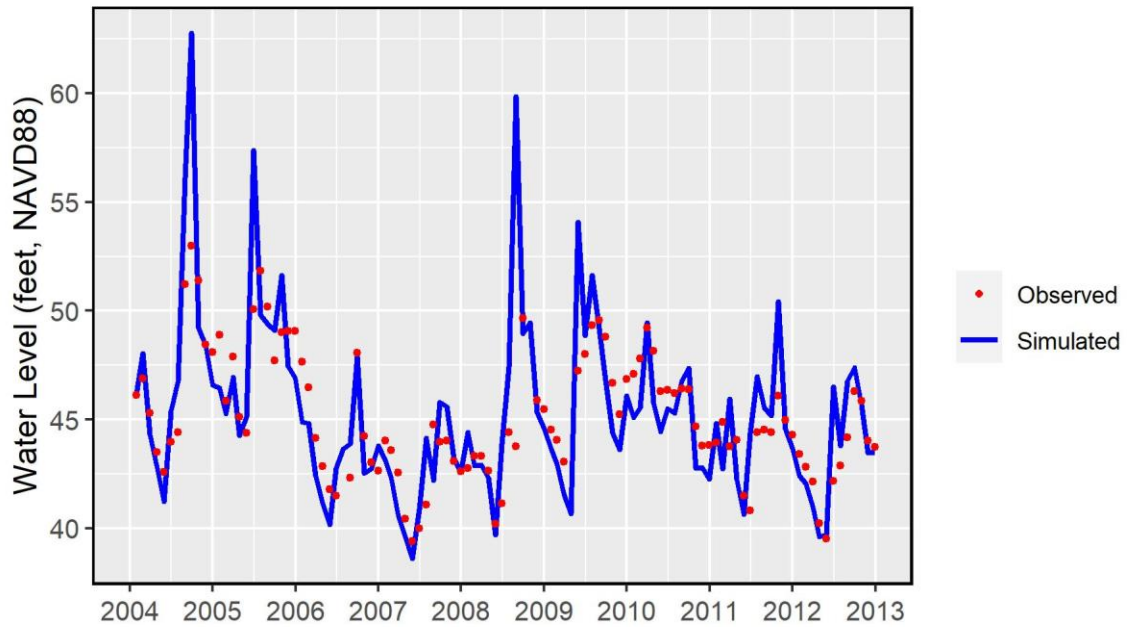
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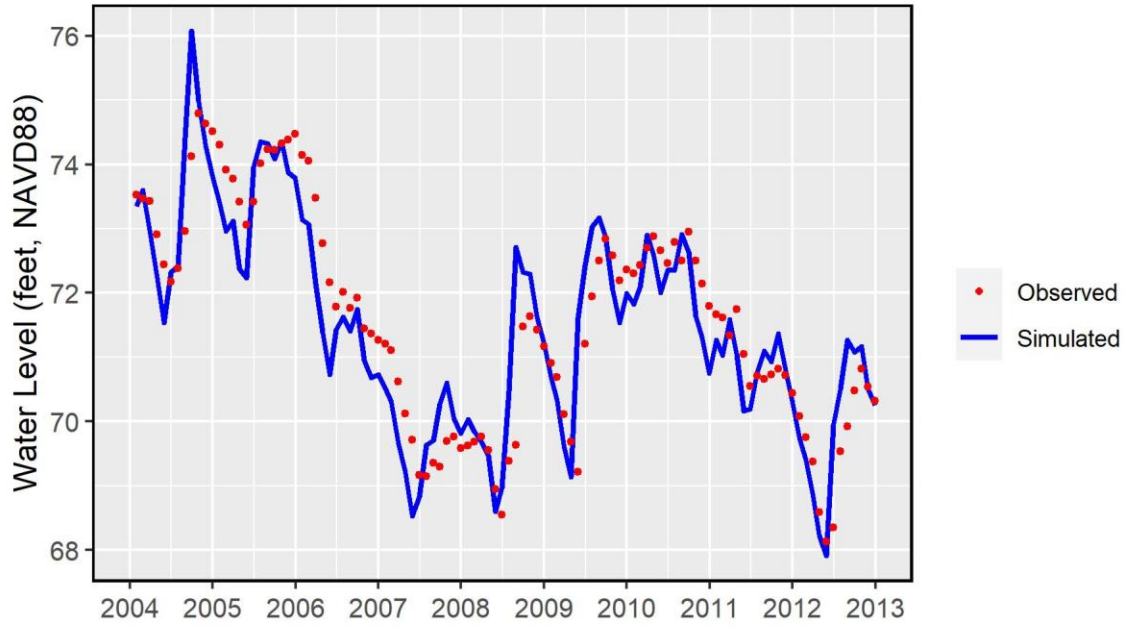
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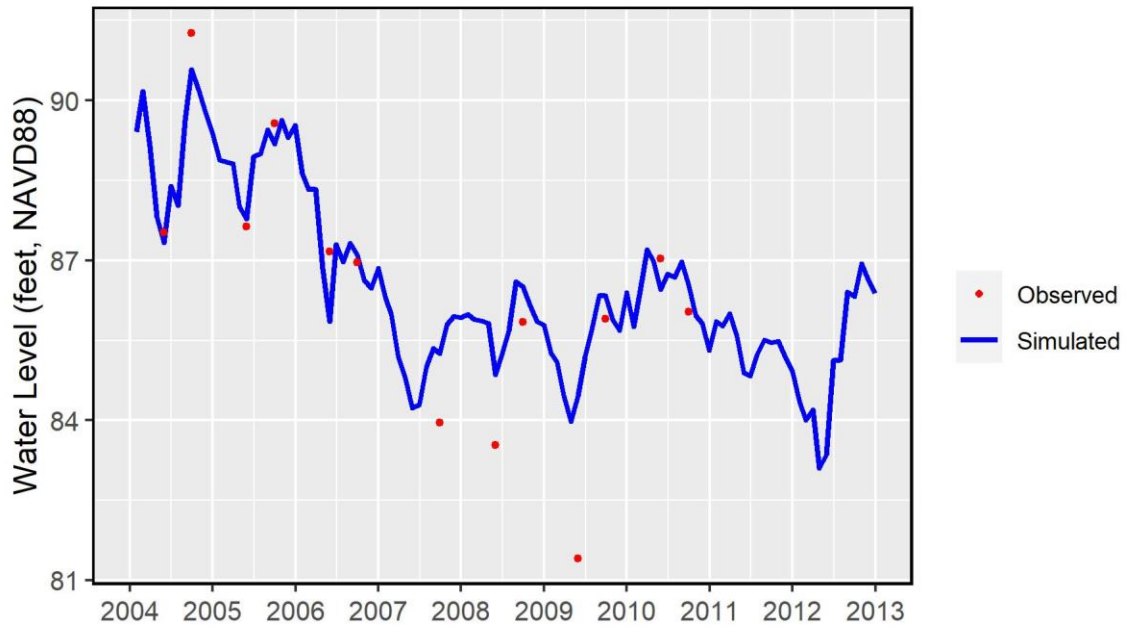
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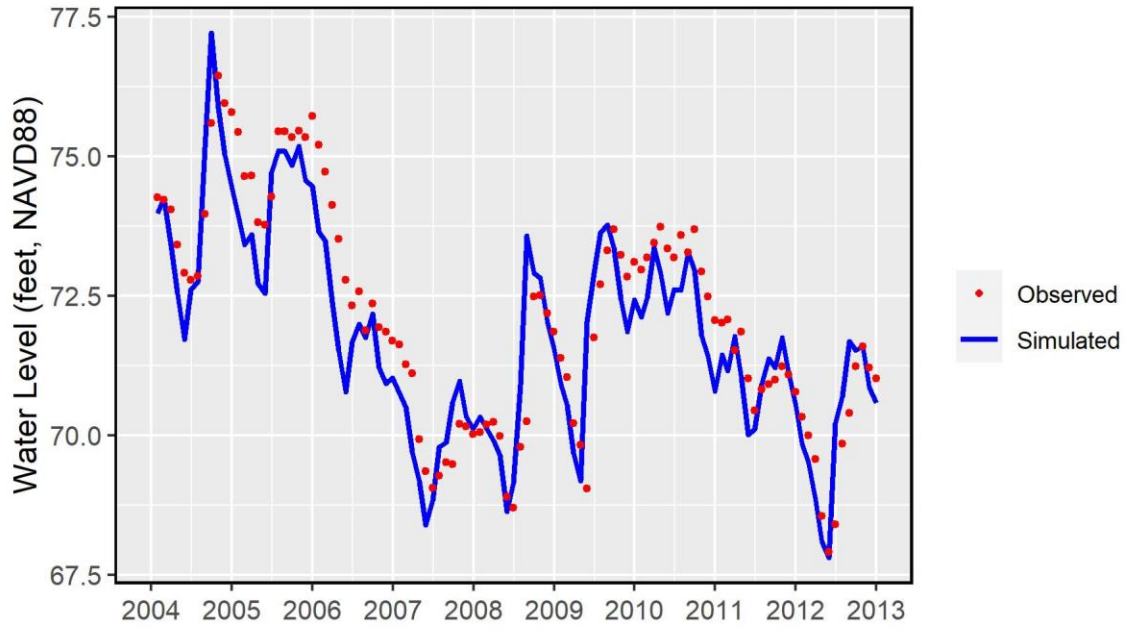
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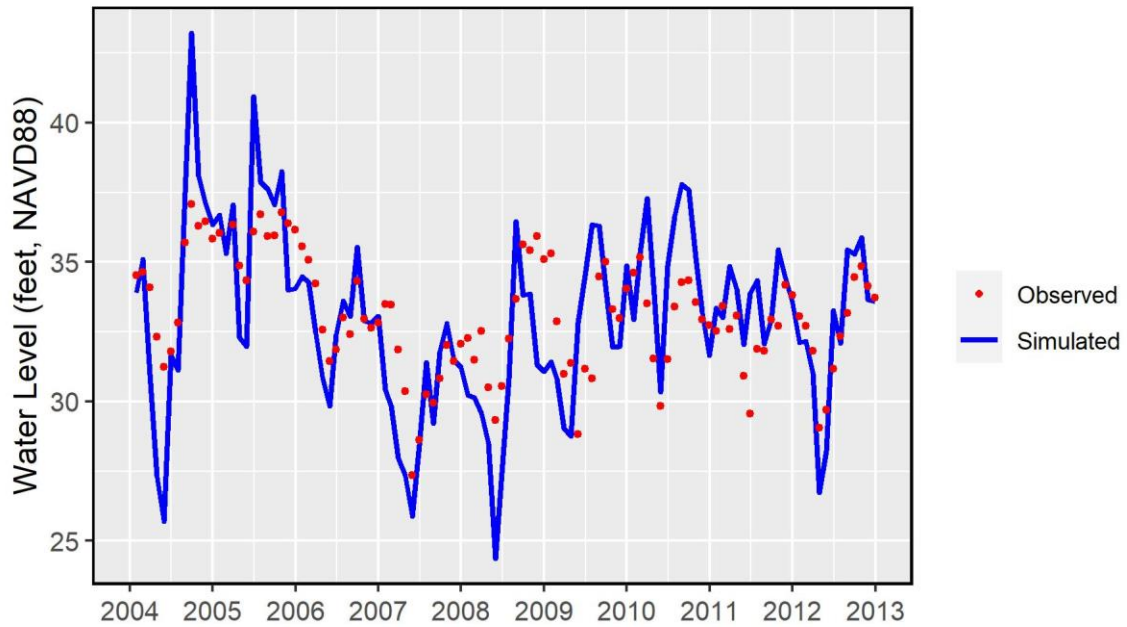
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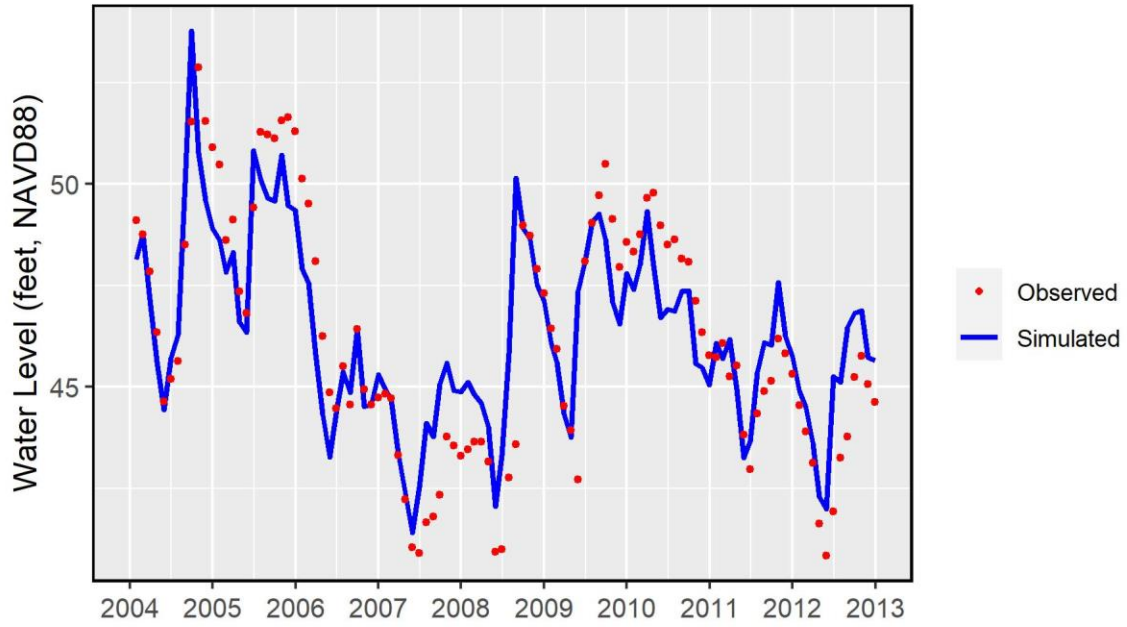
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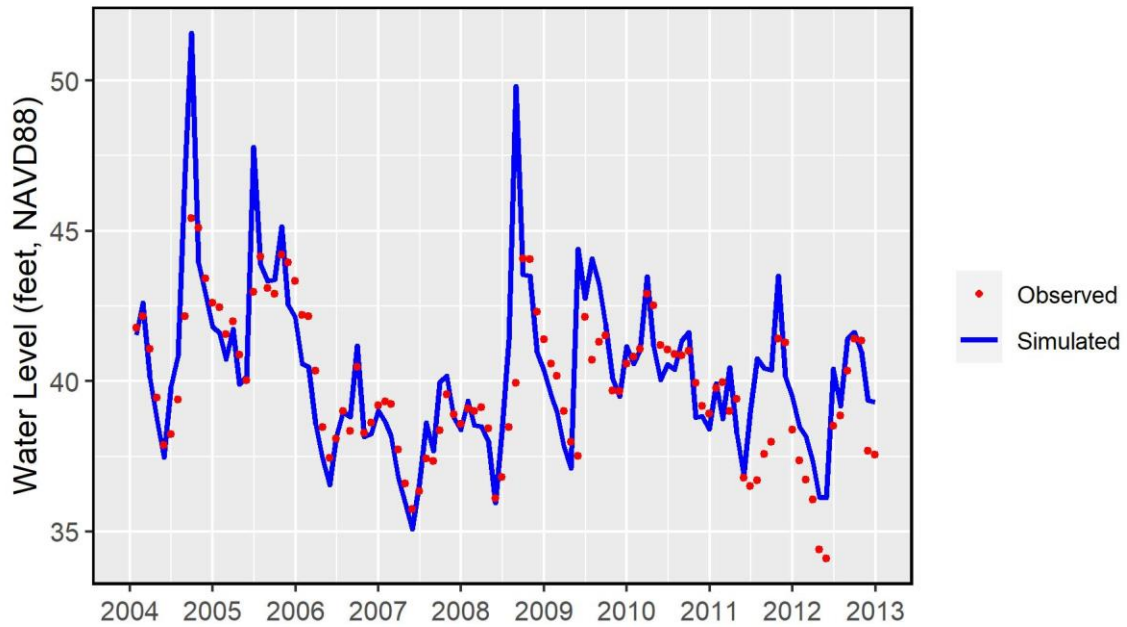
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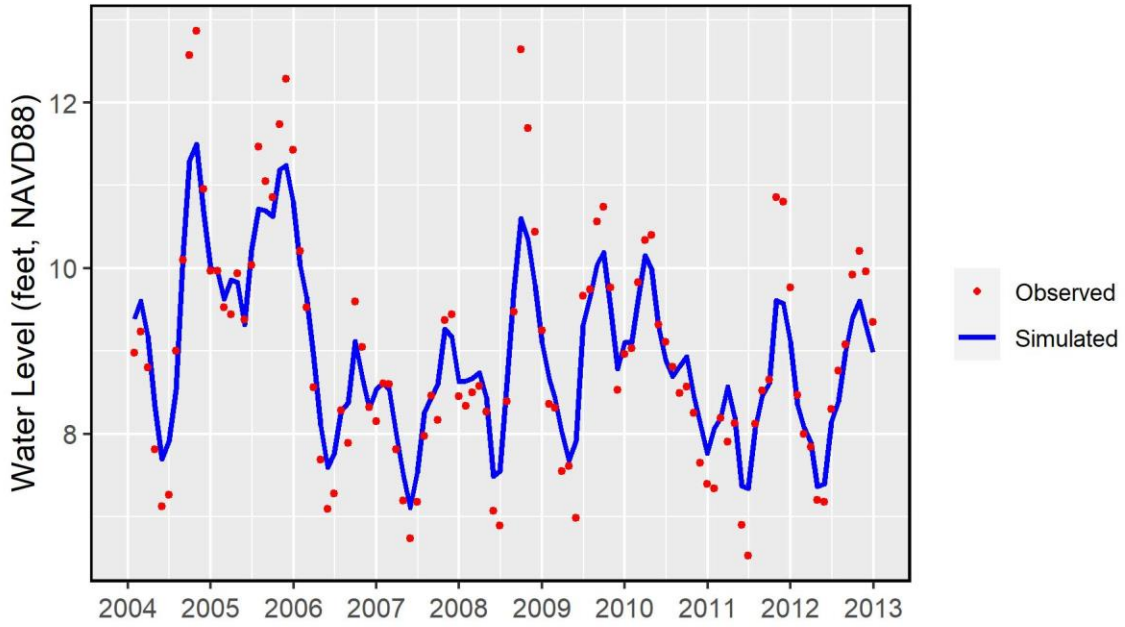
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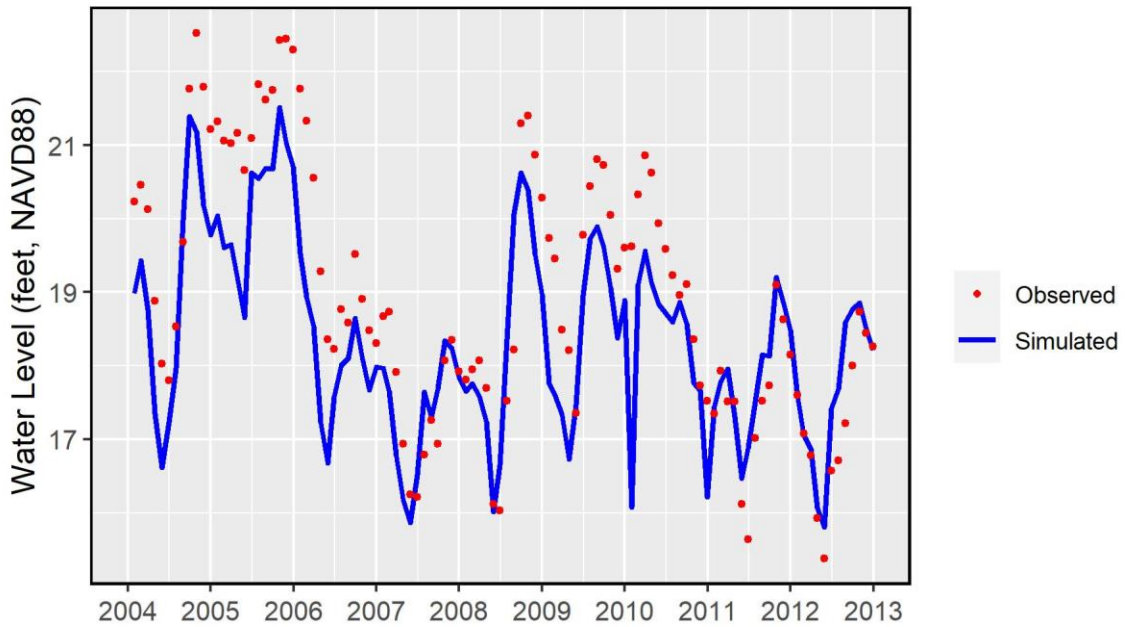
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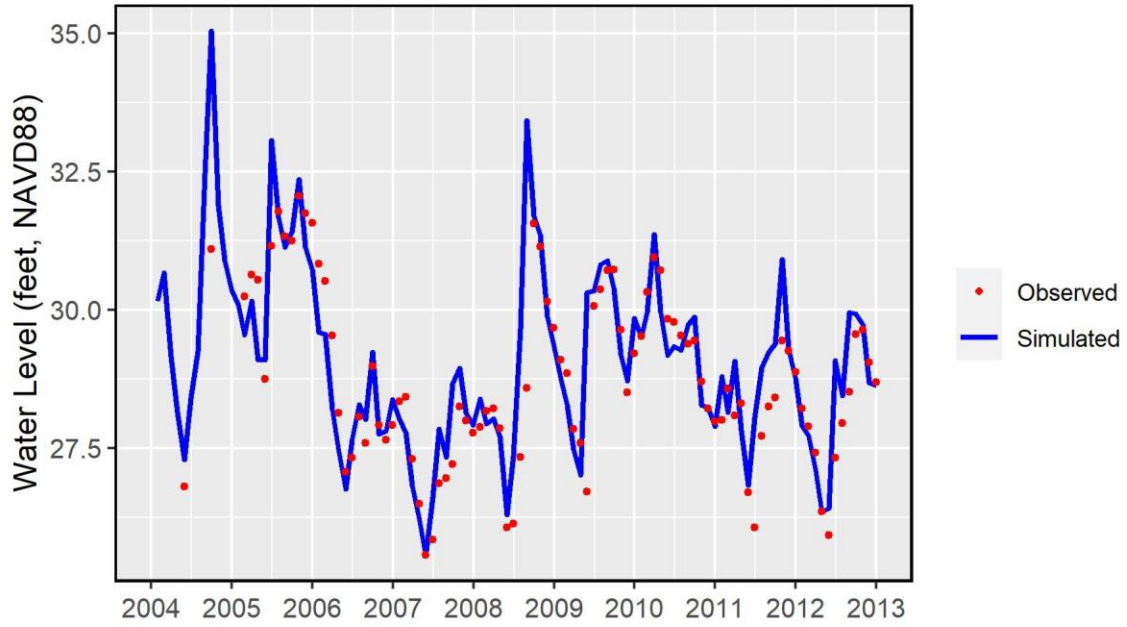
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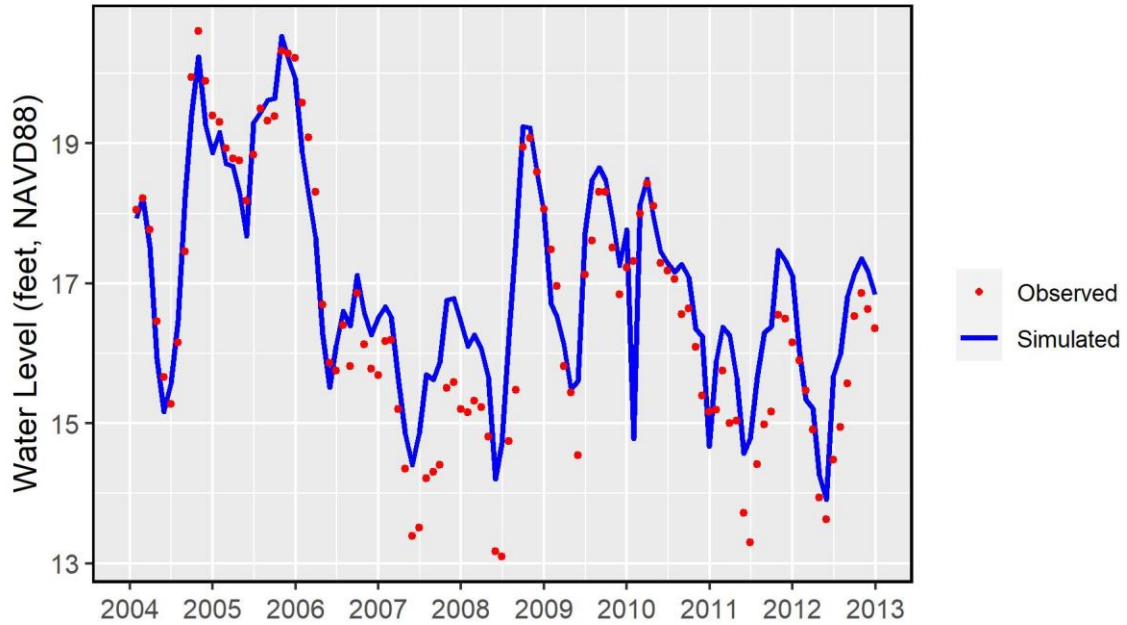
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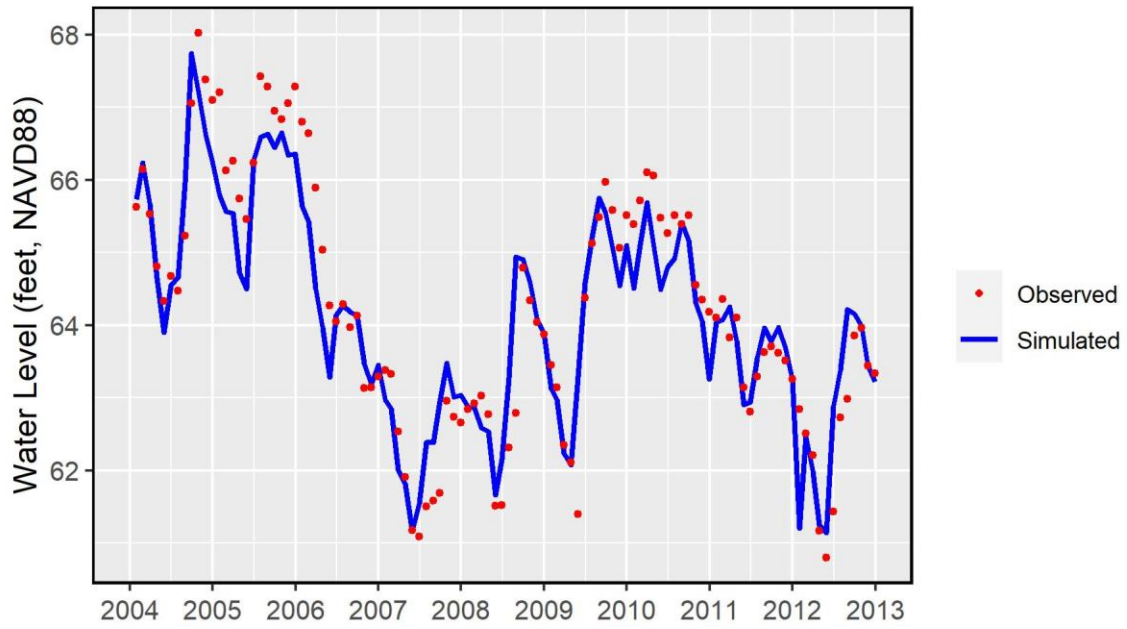
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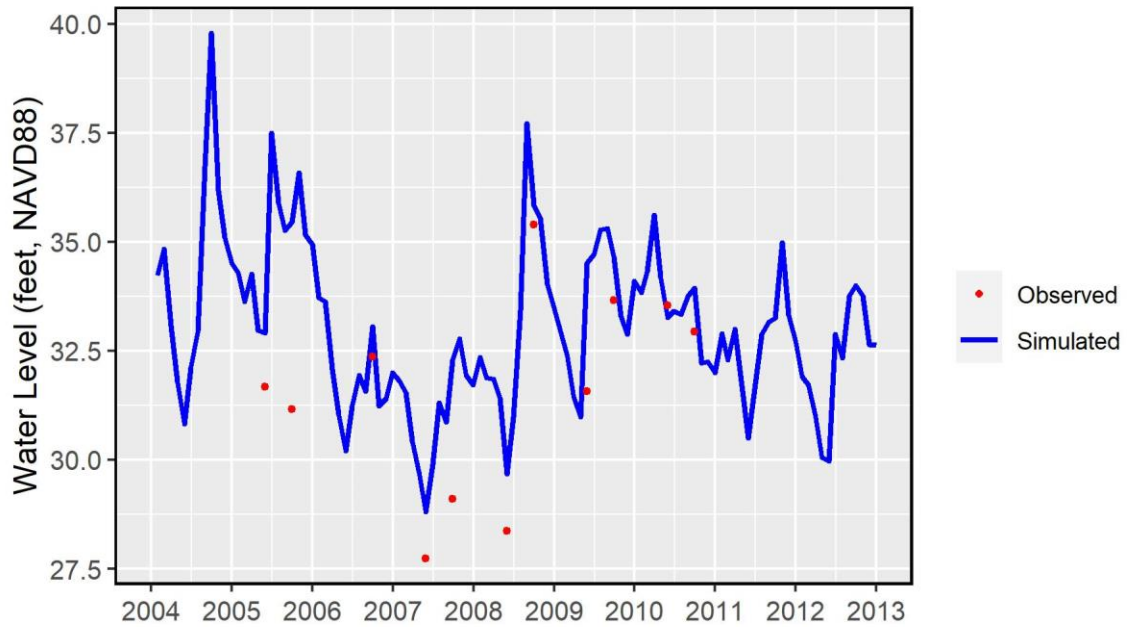
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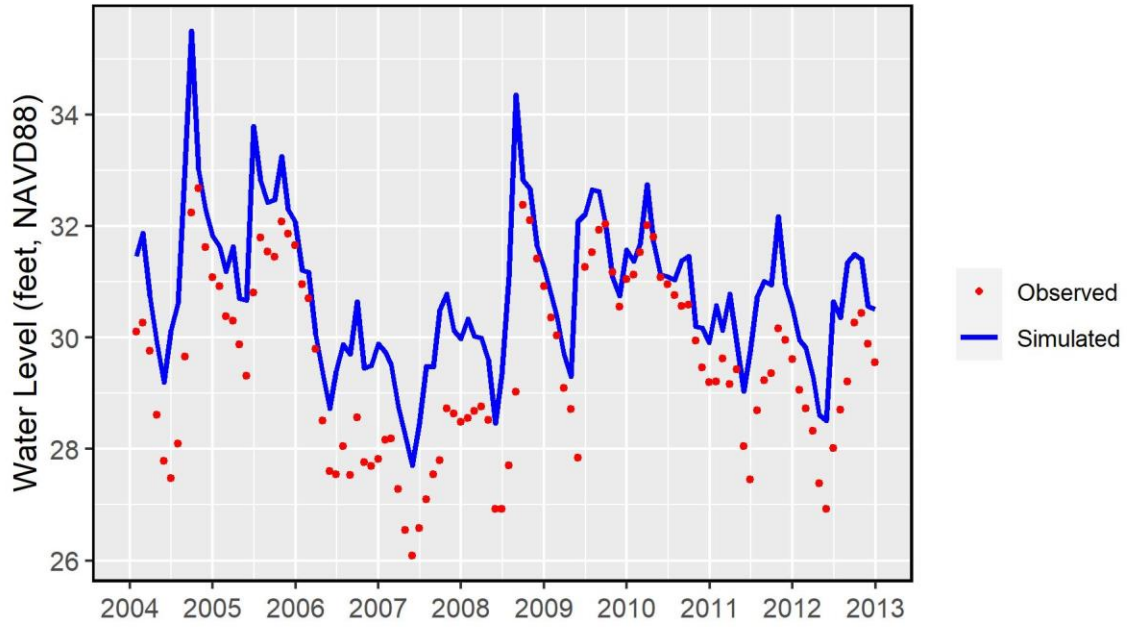
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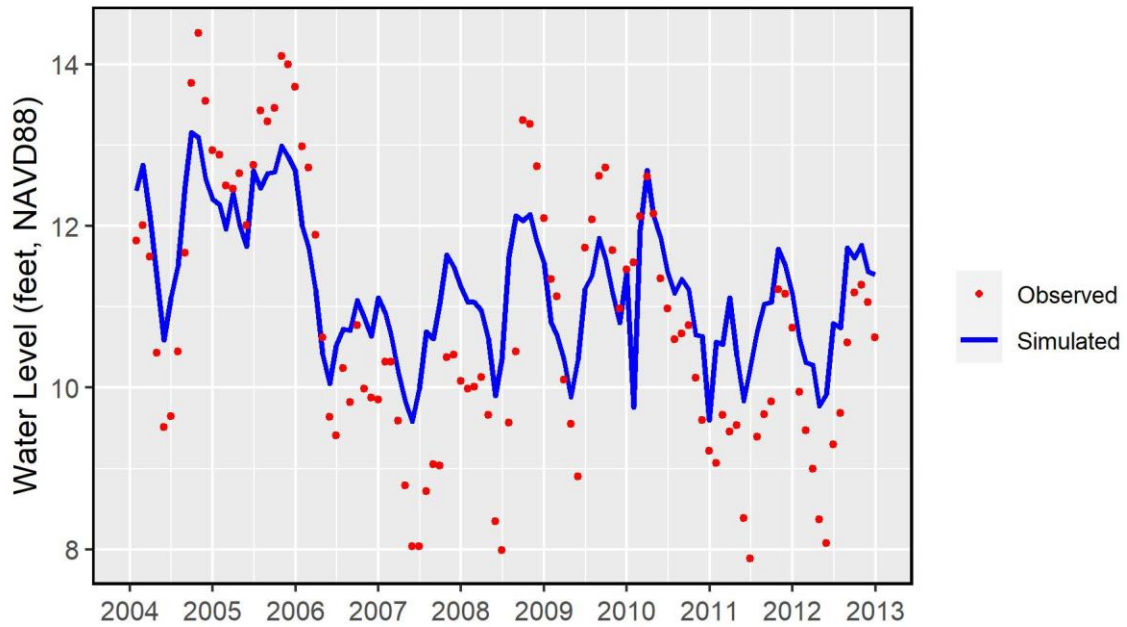
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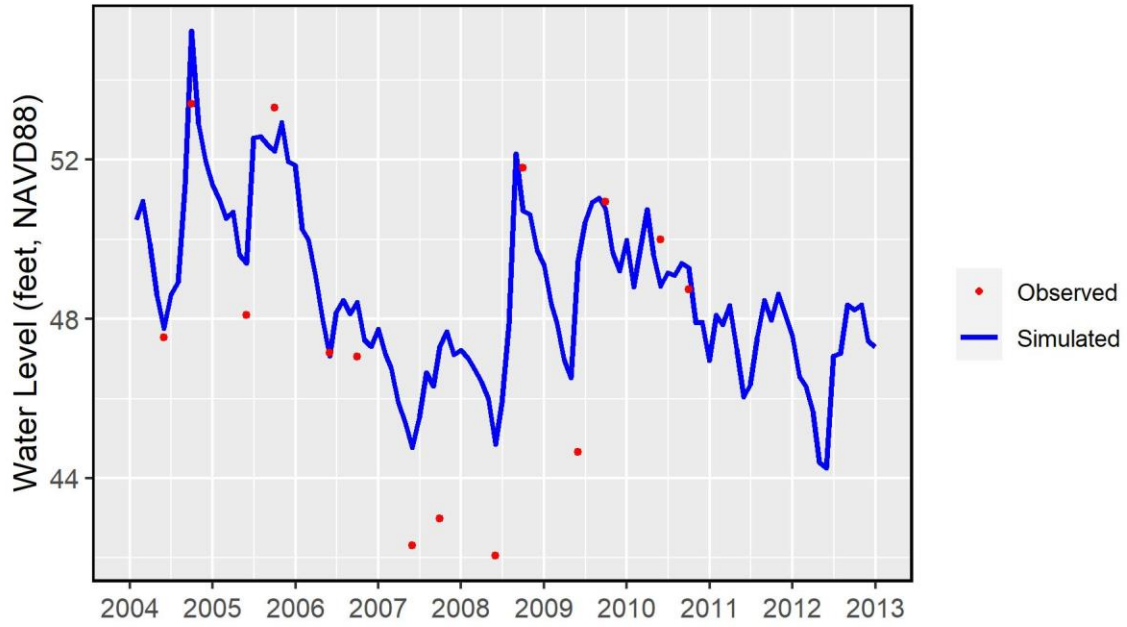
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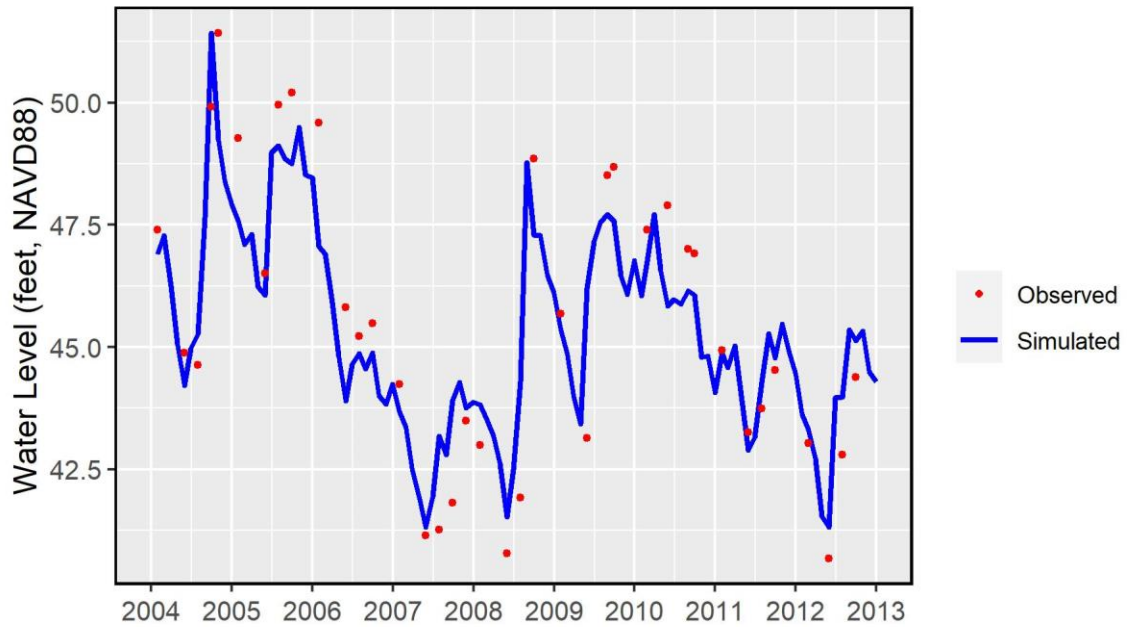
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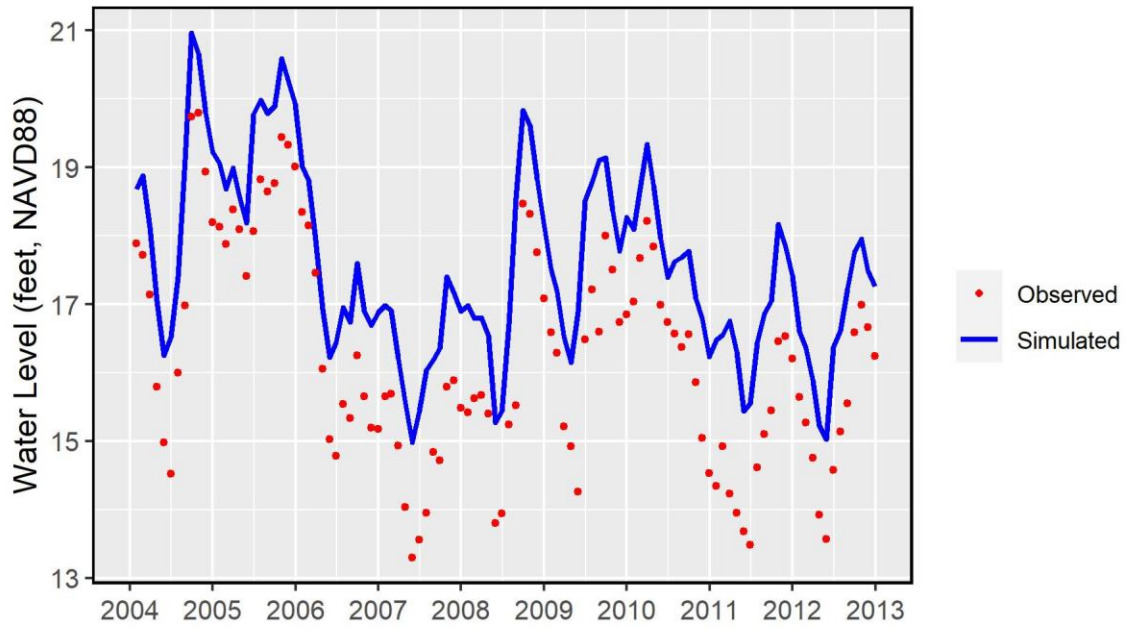
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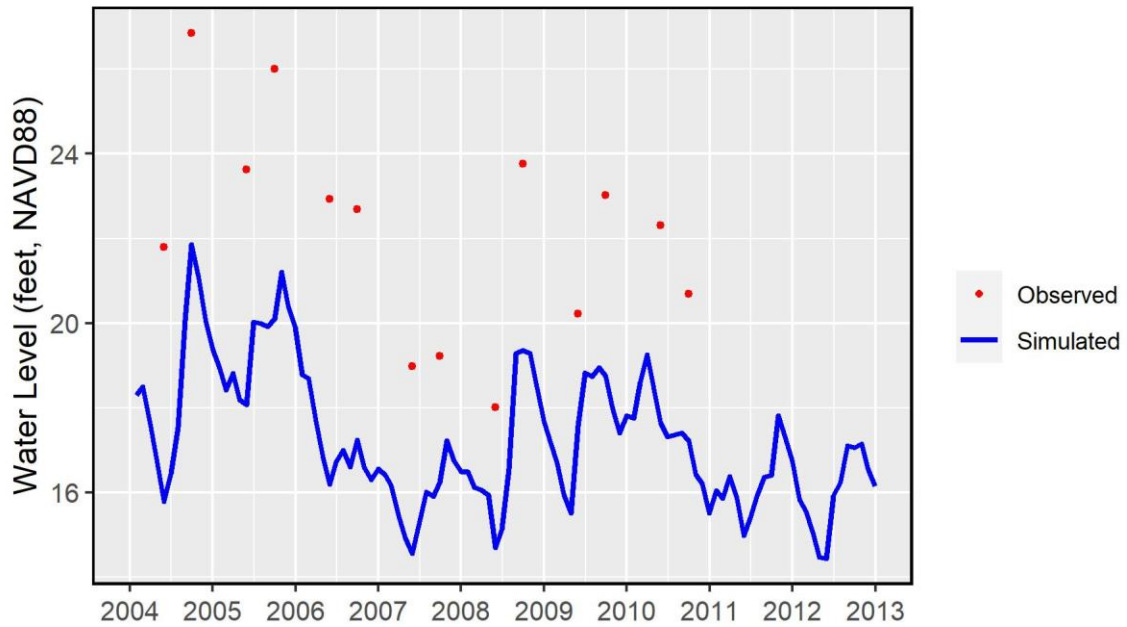
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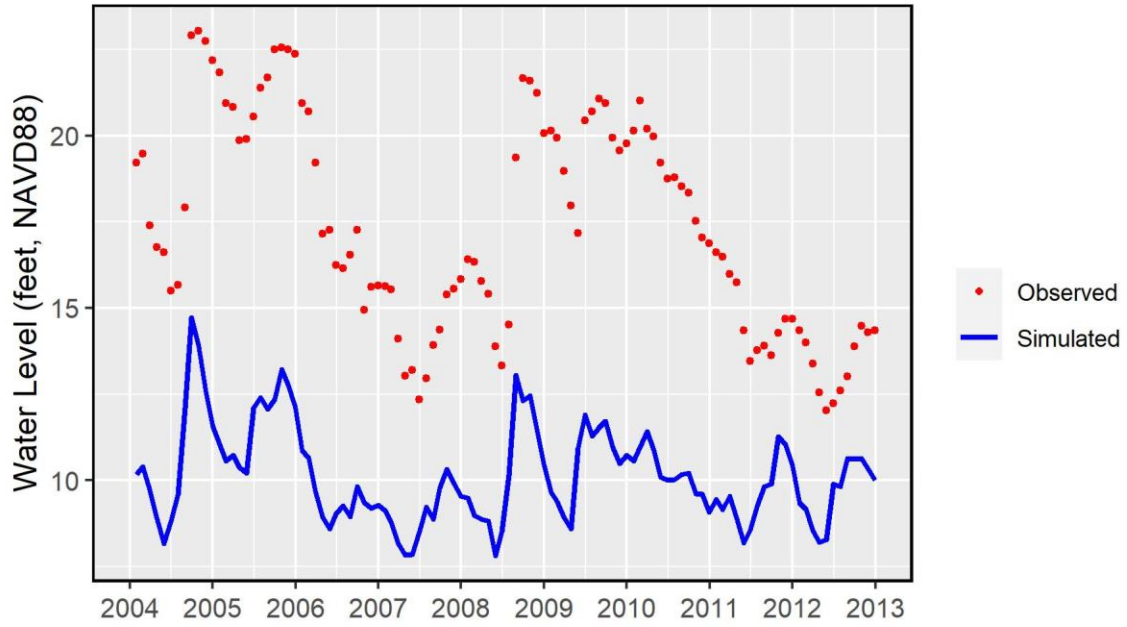
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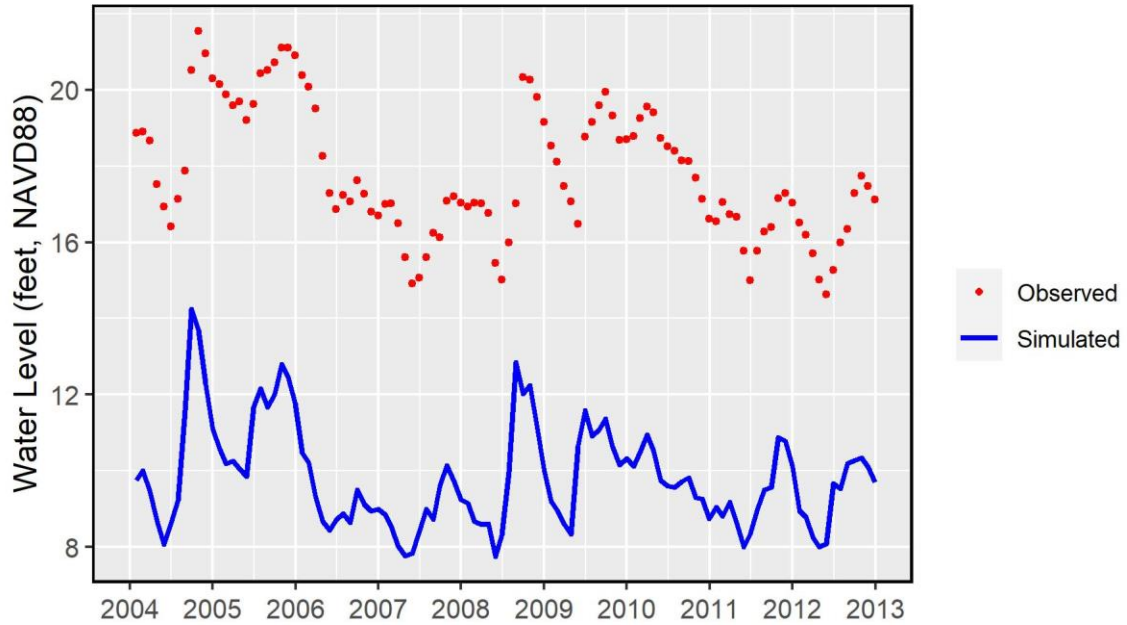
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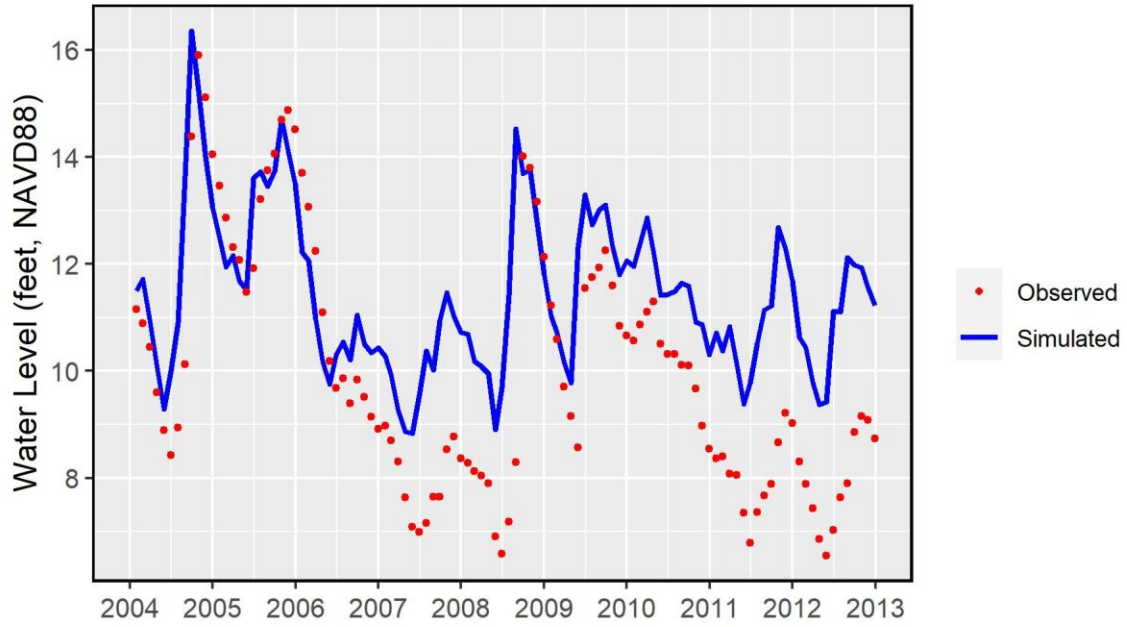
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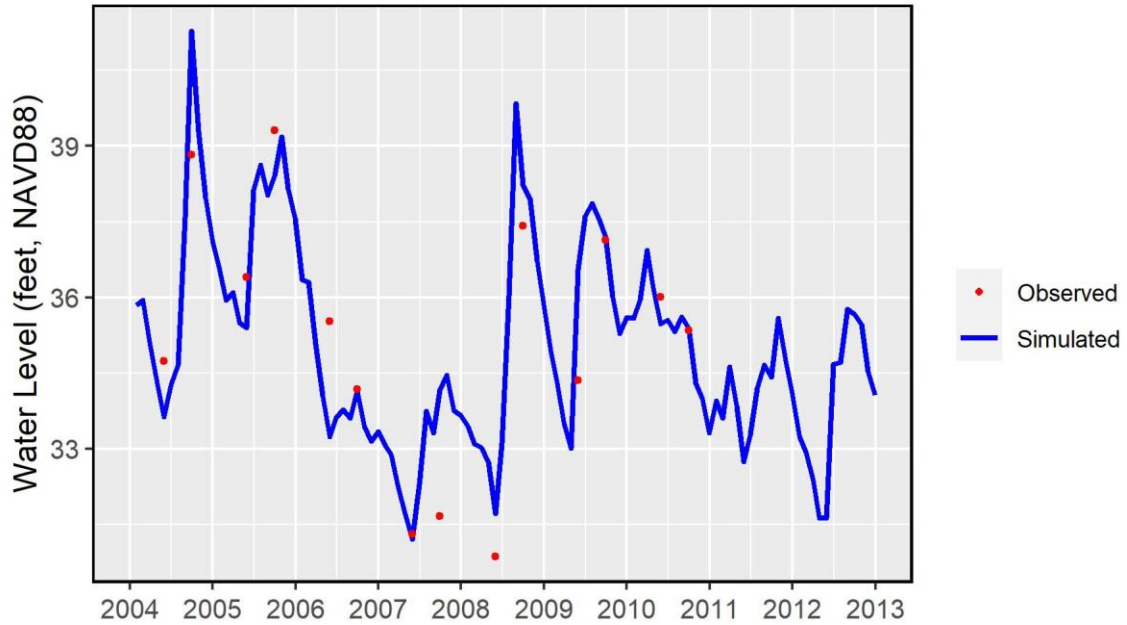
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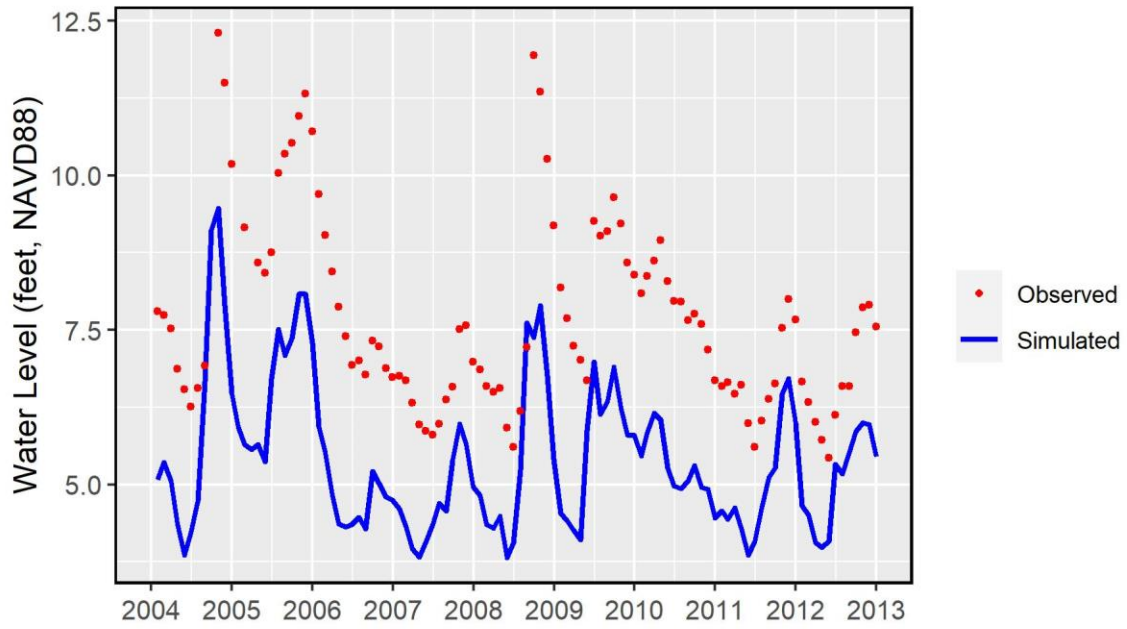
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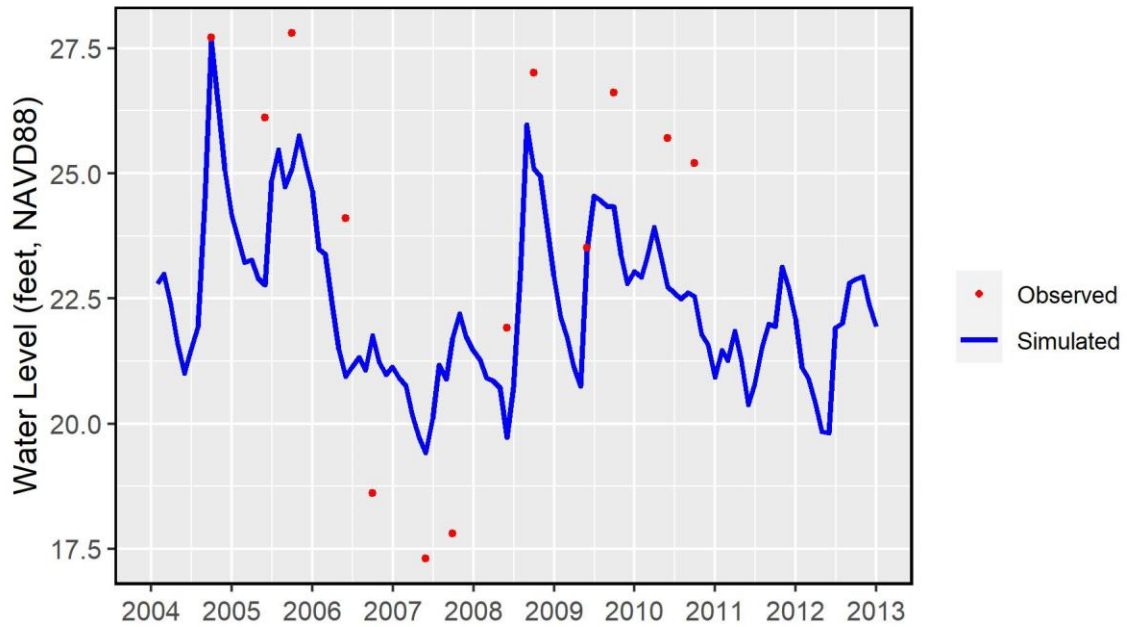
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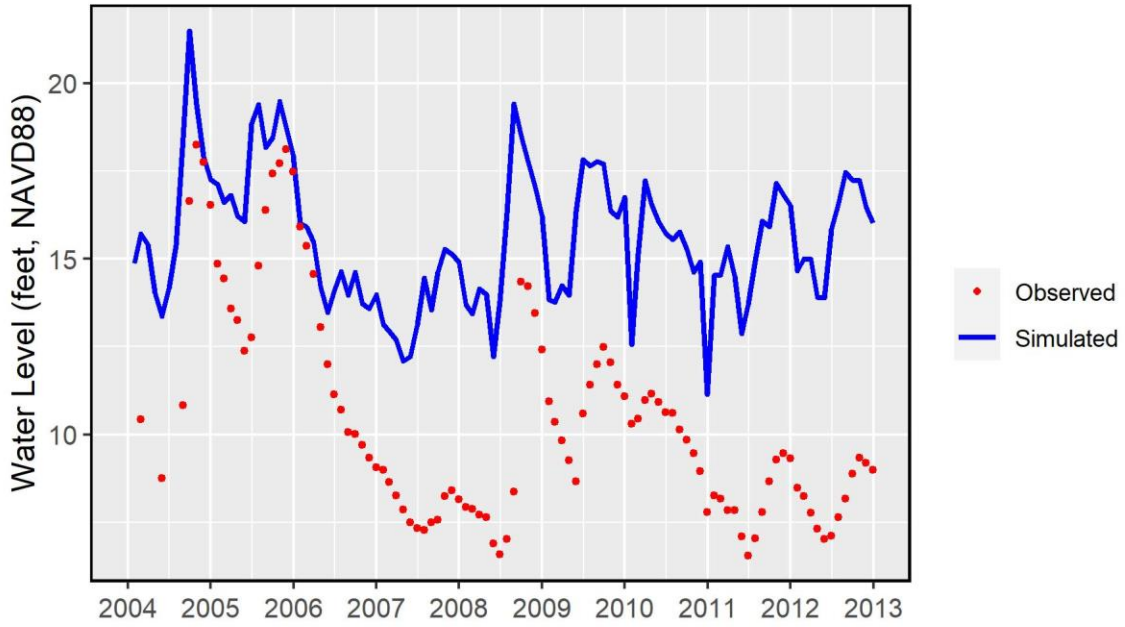
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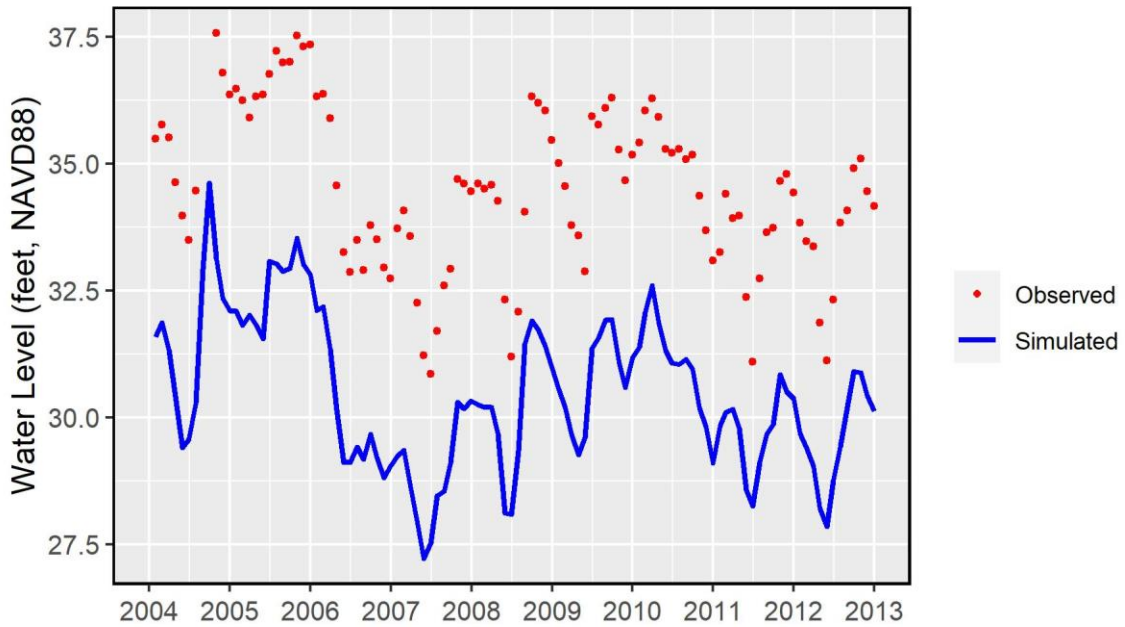
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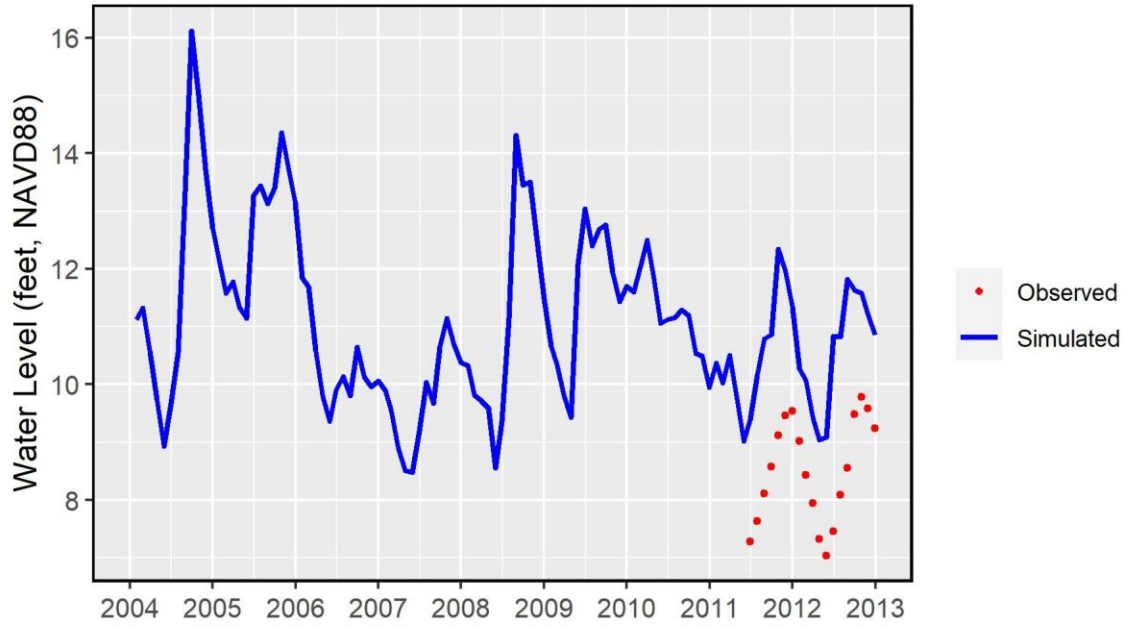
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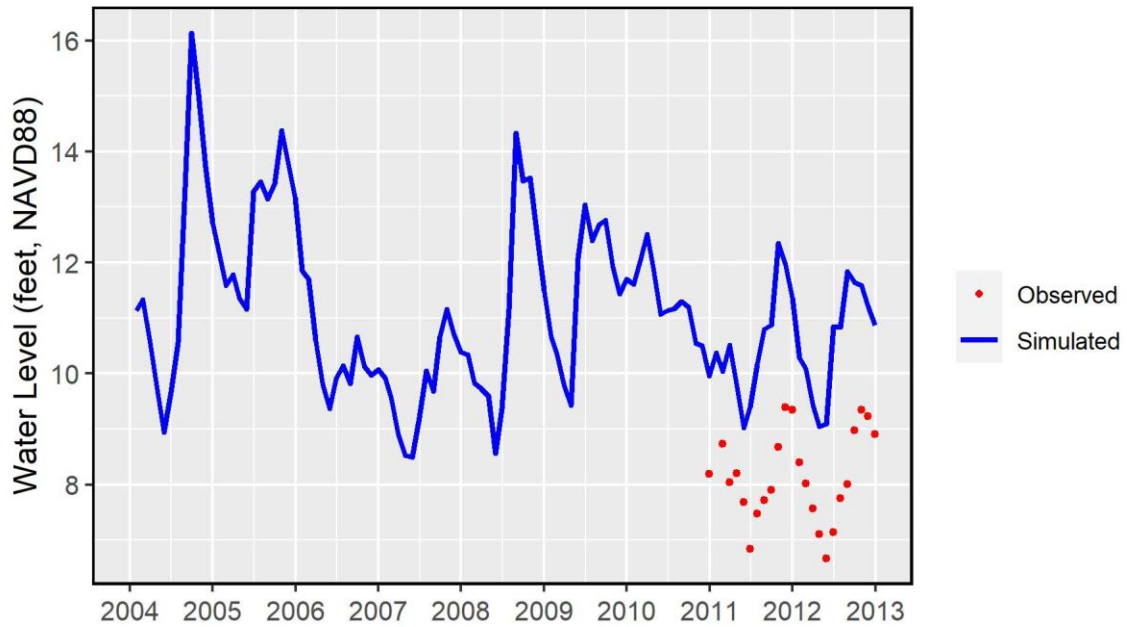
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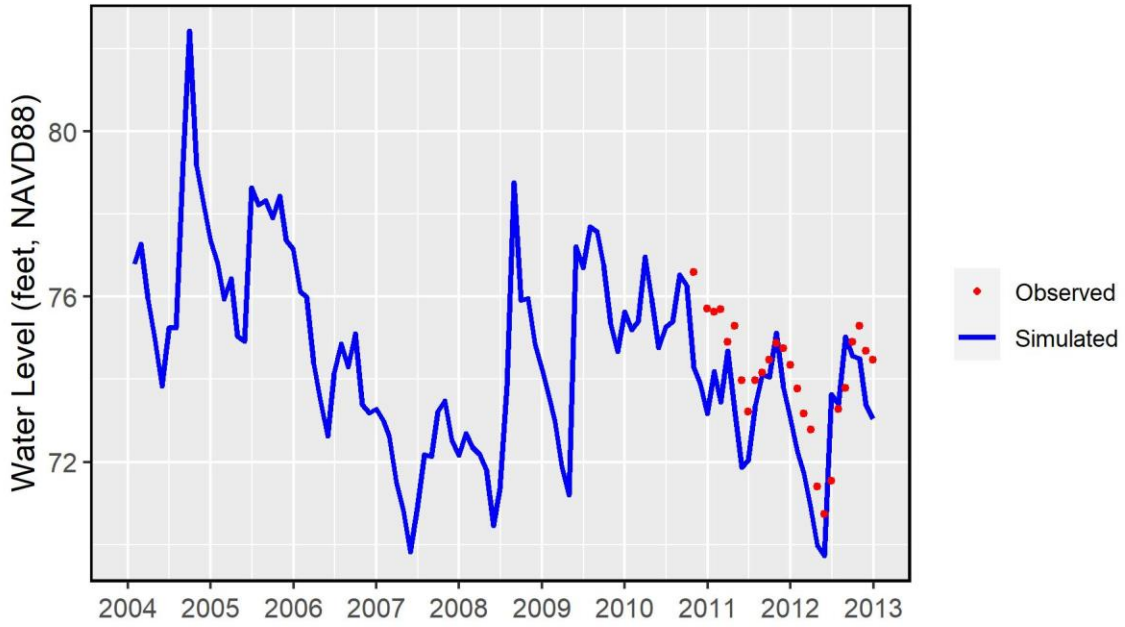
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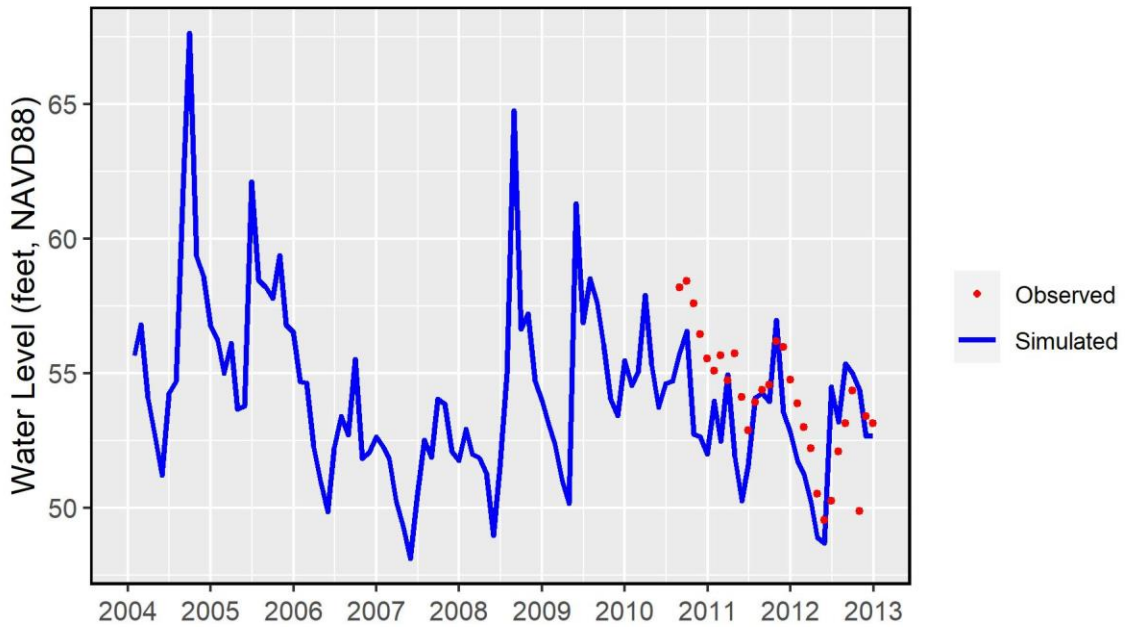
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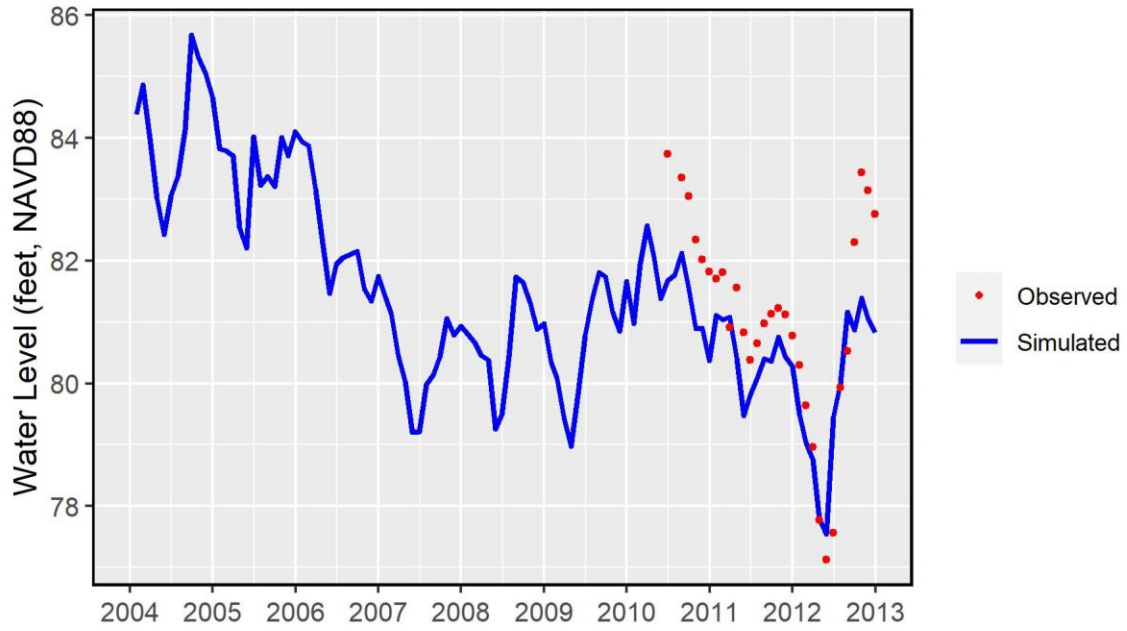
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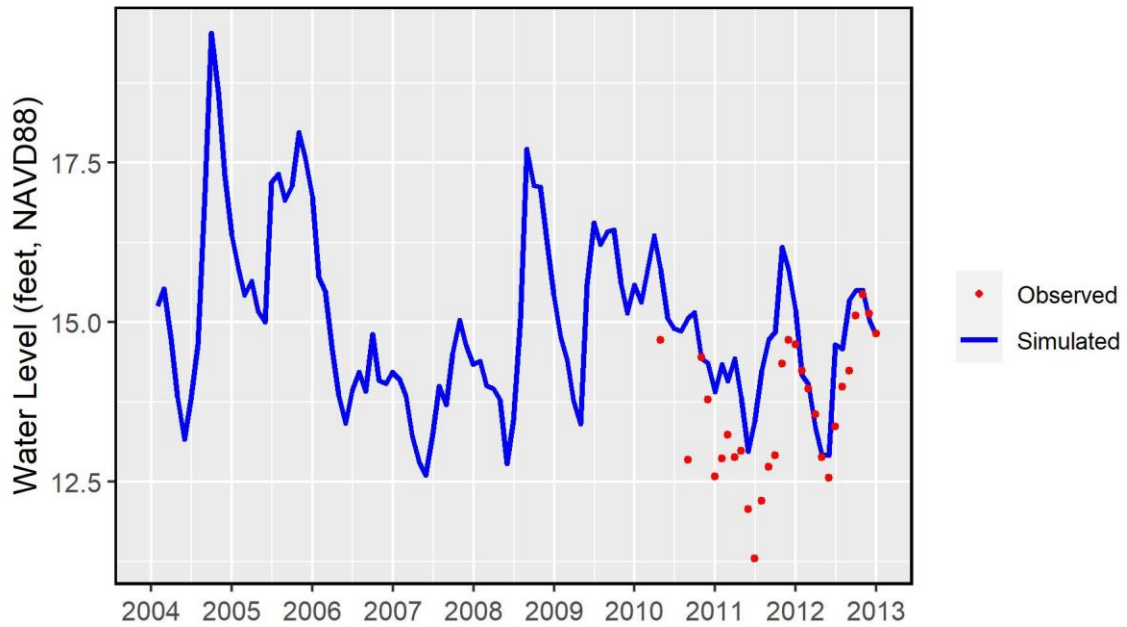
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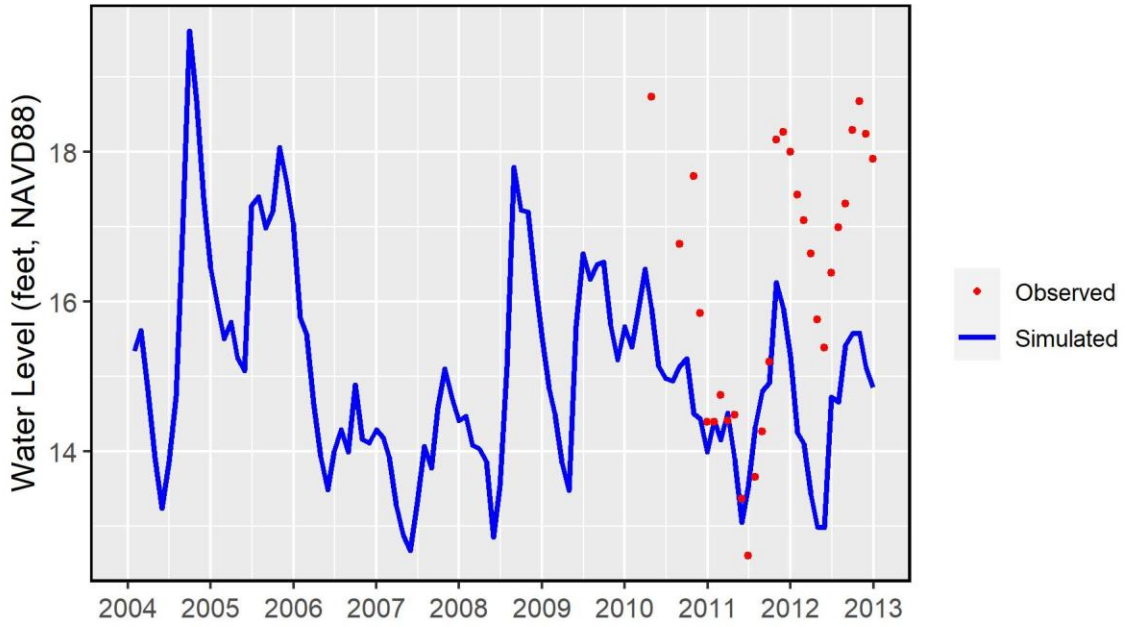
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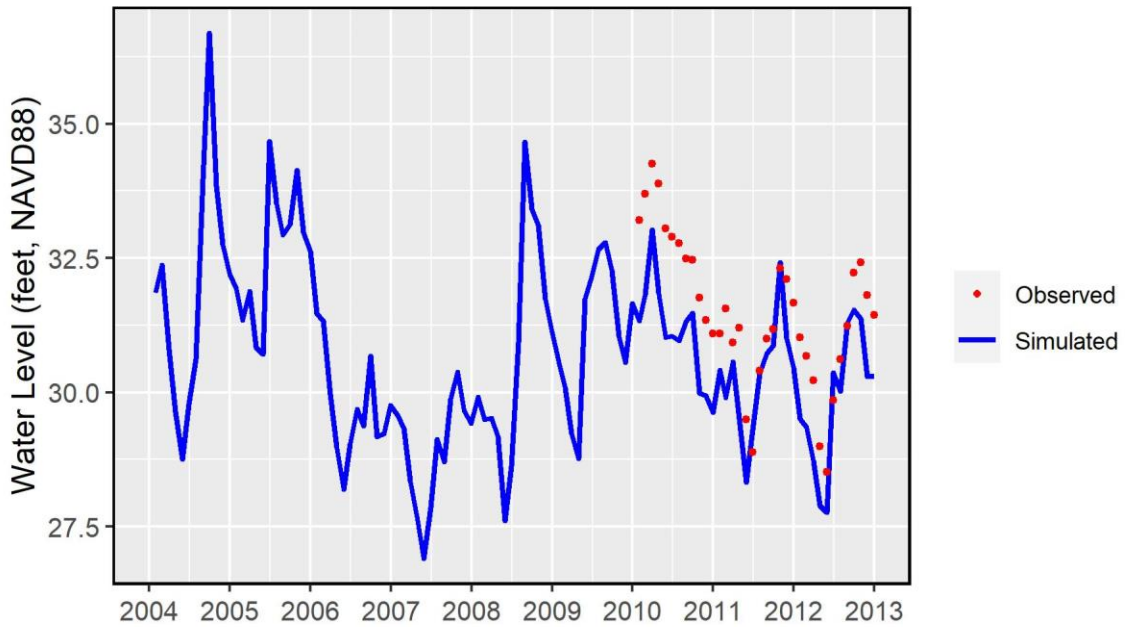
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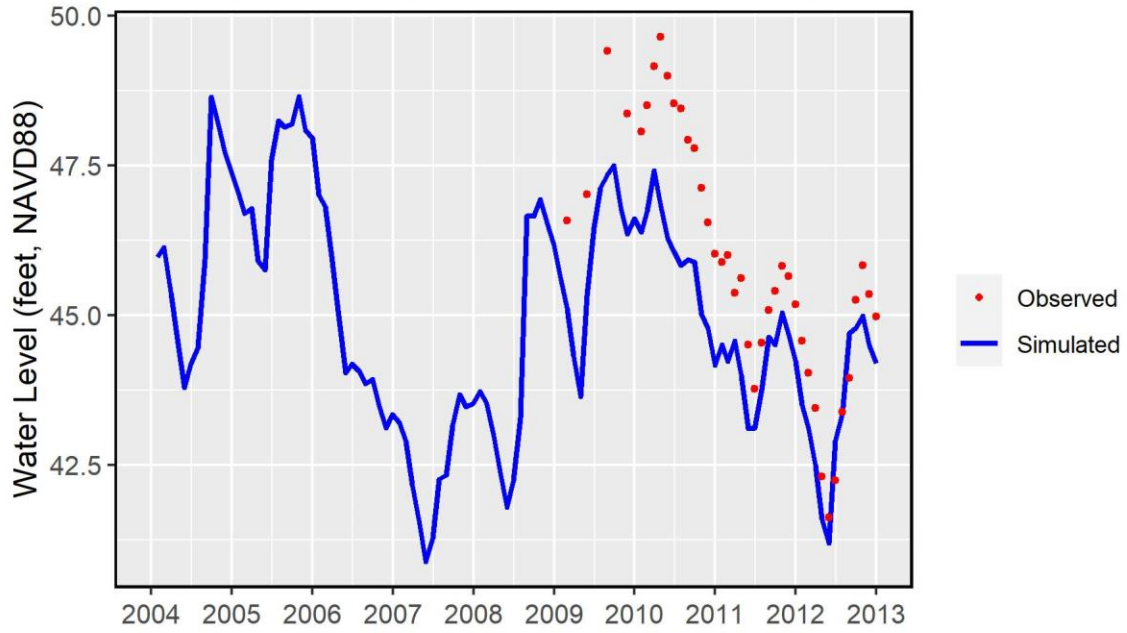
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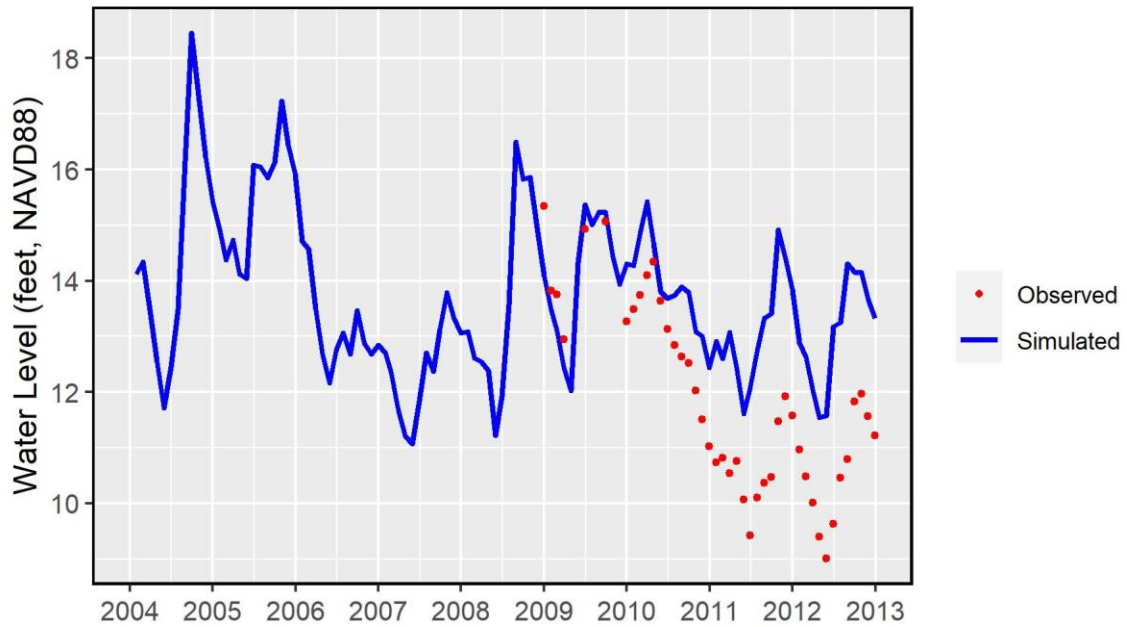
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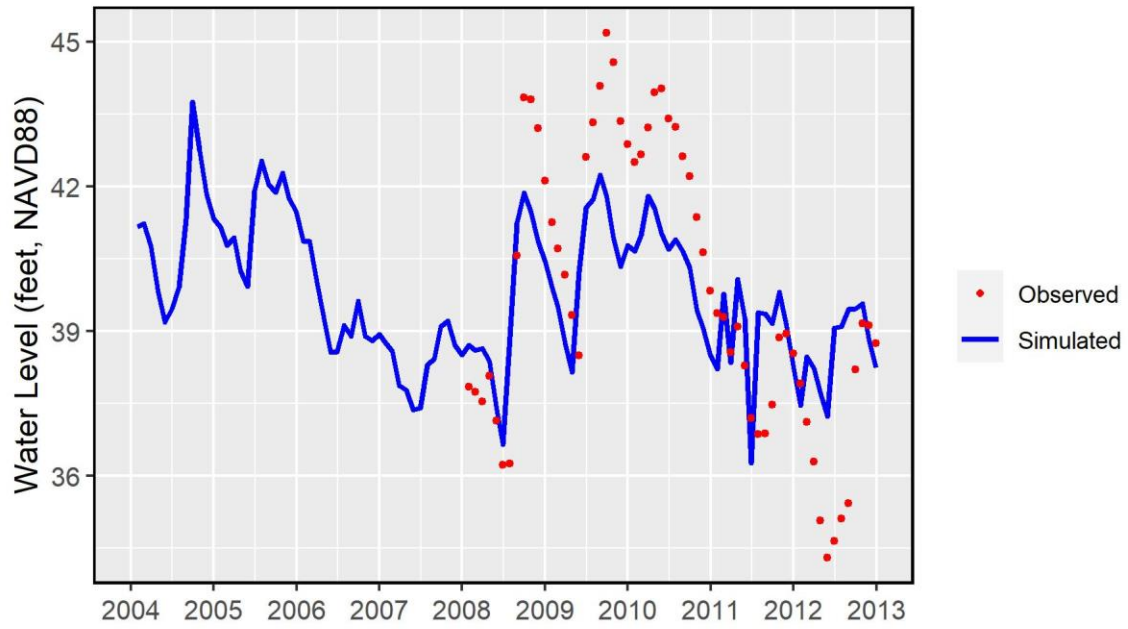
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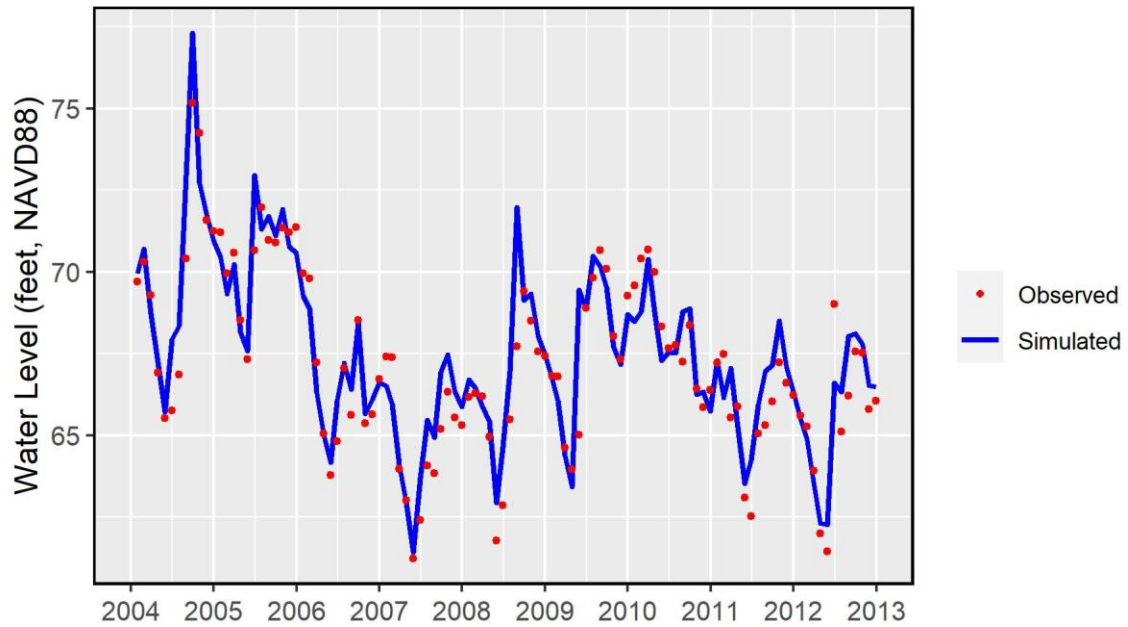
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ME=-0.265 MAE=1.669 nMAE=1.636 R2=0.666 NS=0.544

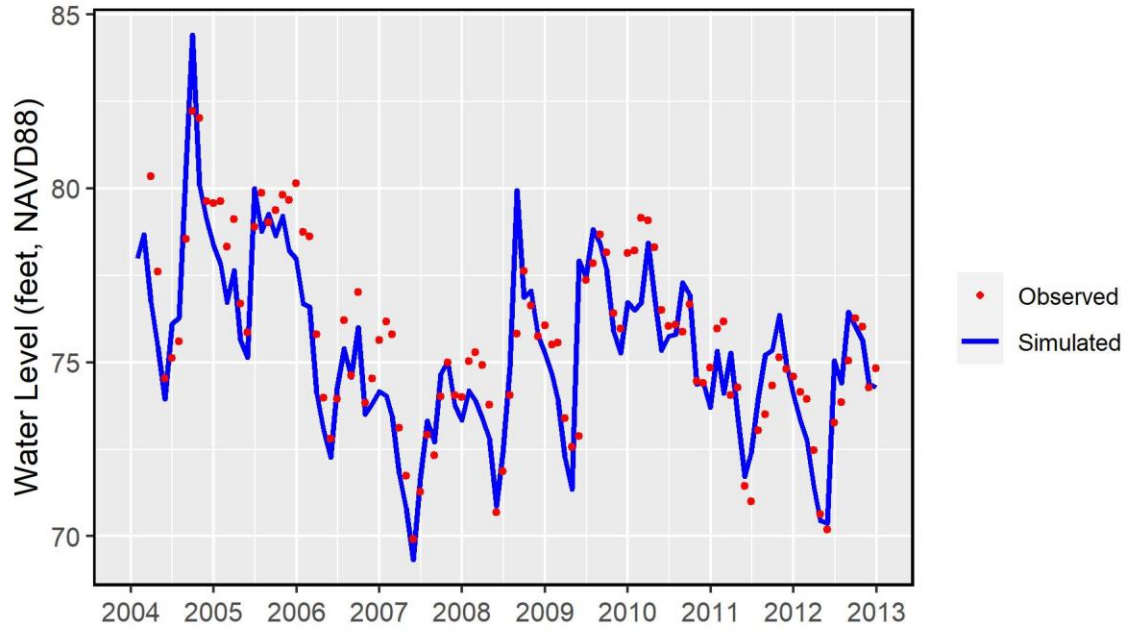


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UFA: ORF-61

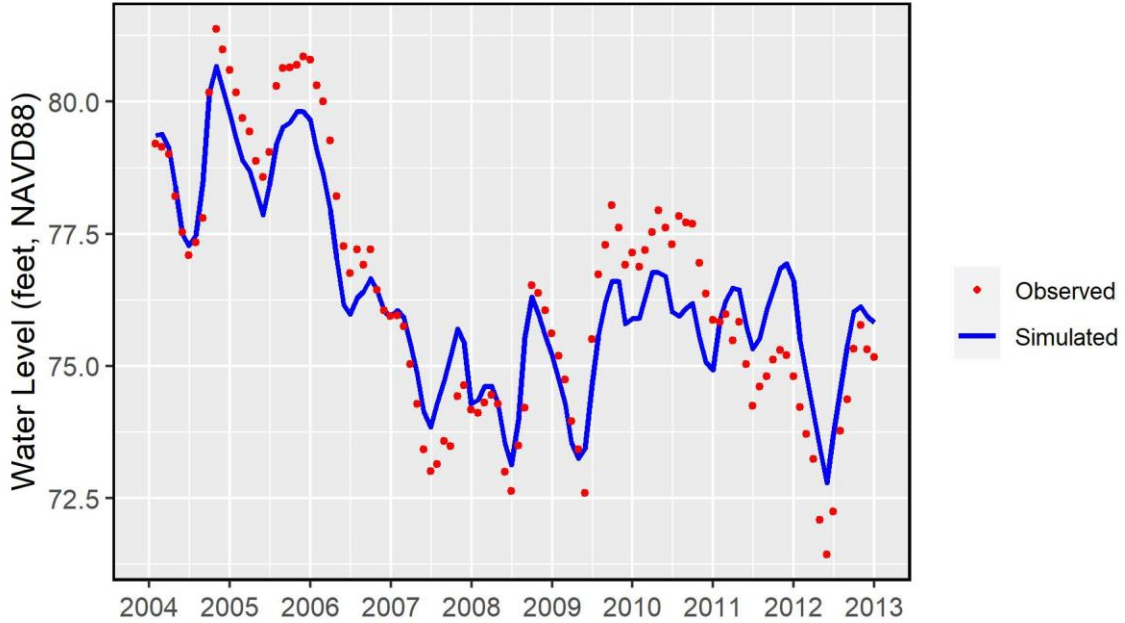
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Lower Floridan Aquifer (LFA) Target Wells

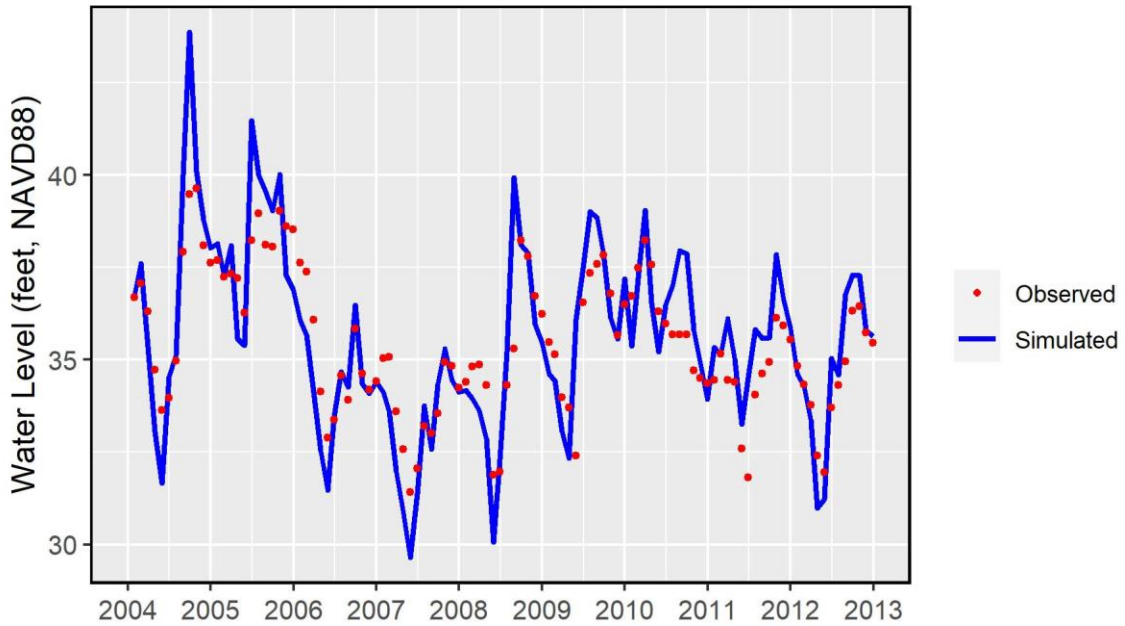
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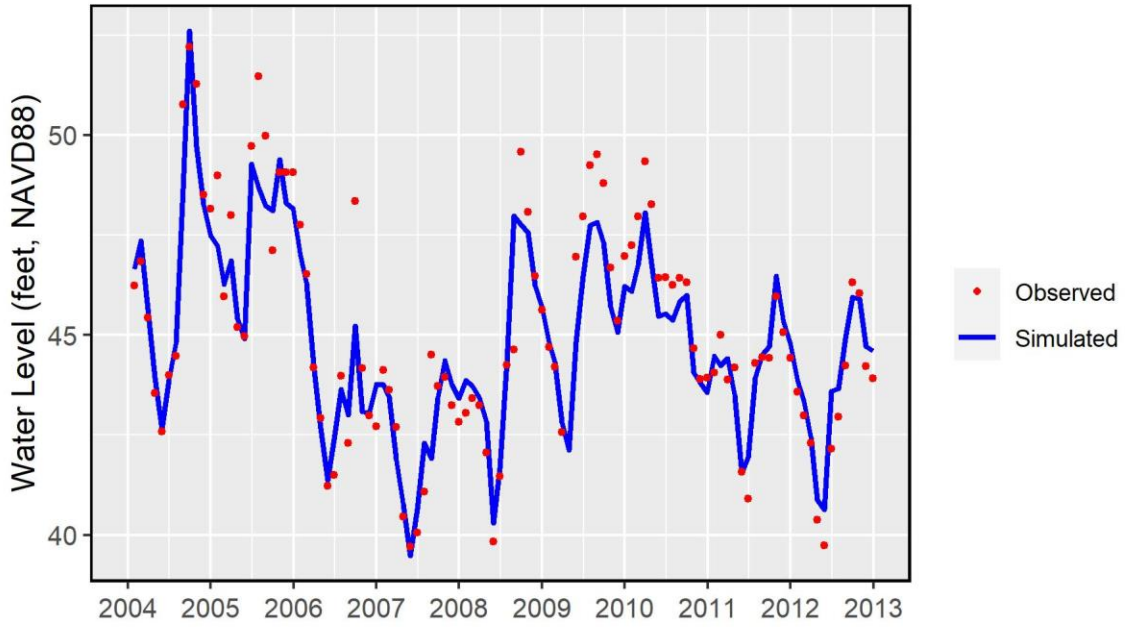


LFA: SJRWMD05320993

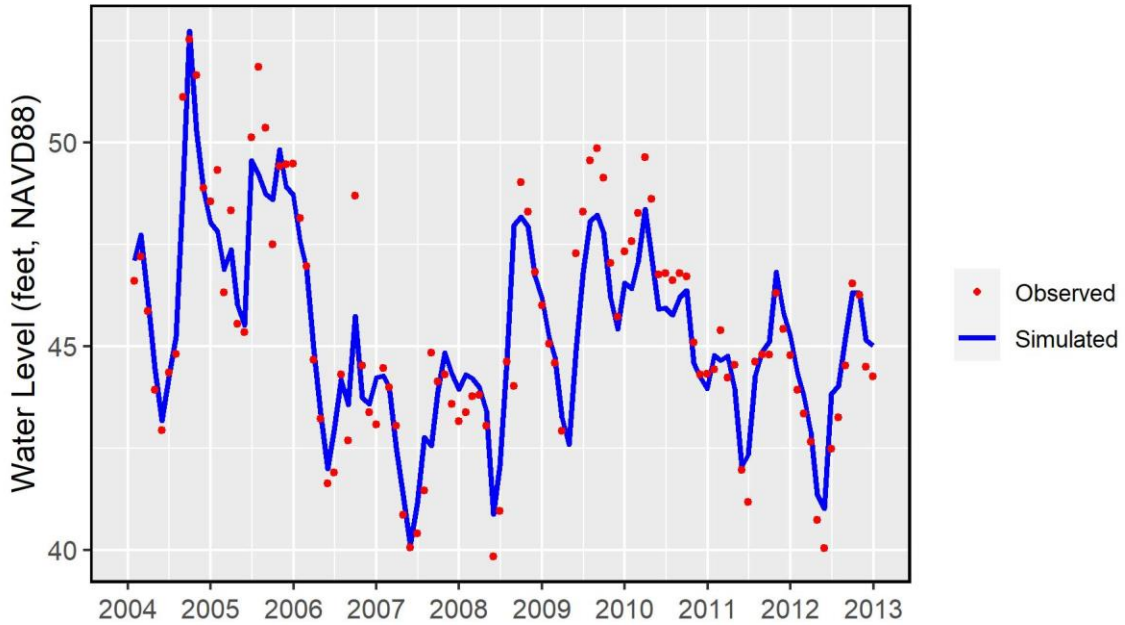
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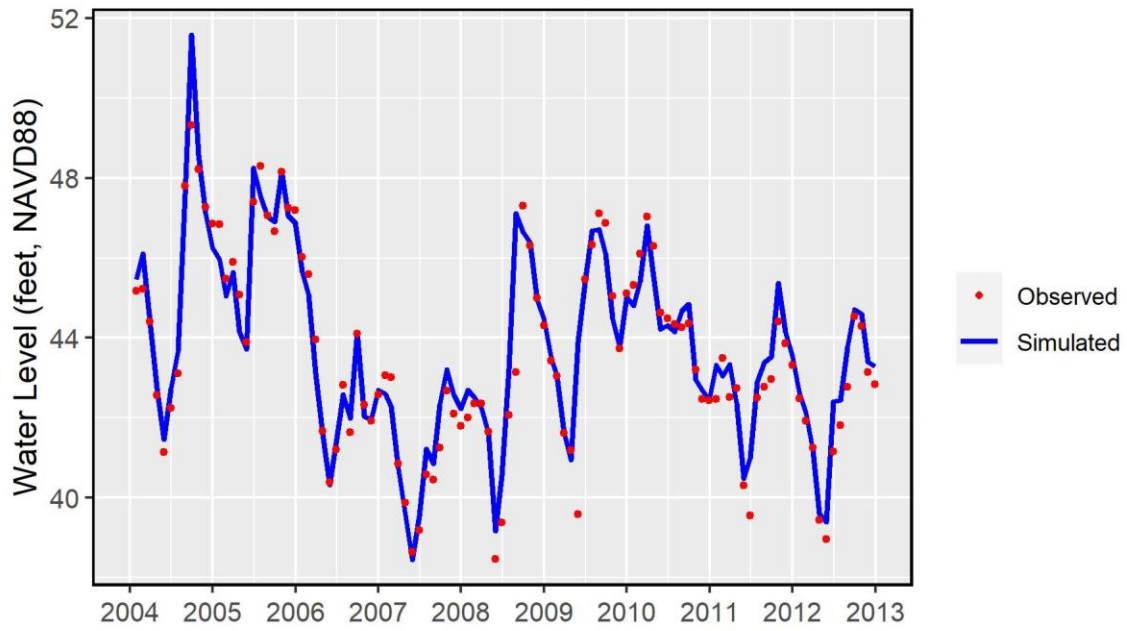
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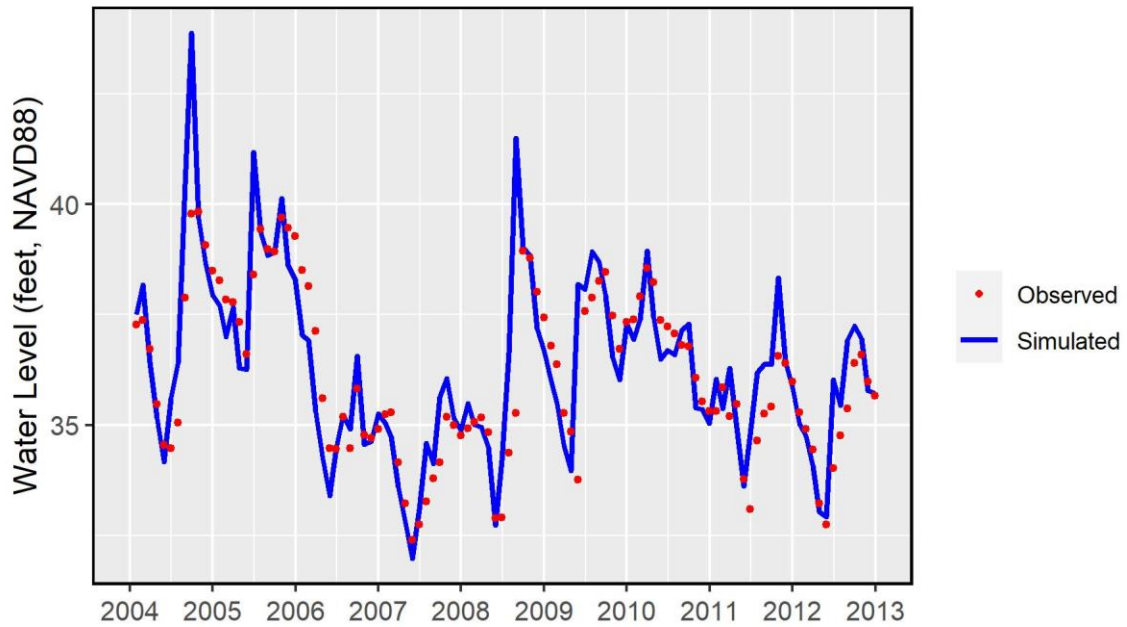
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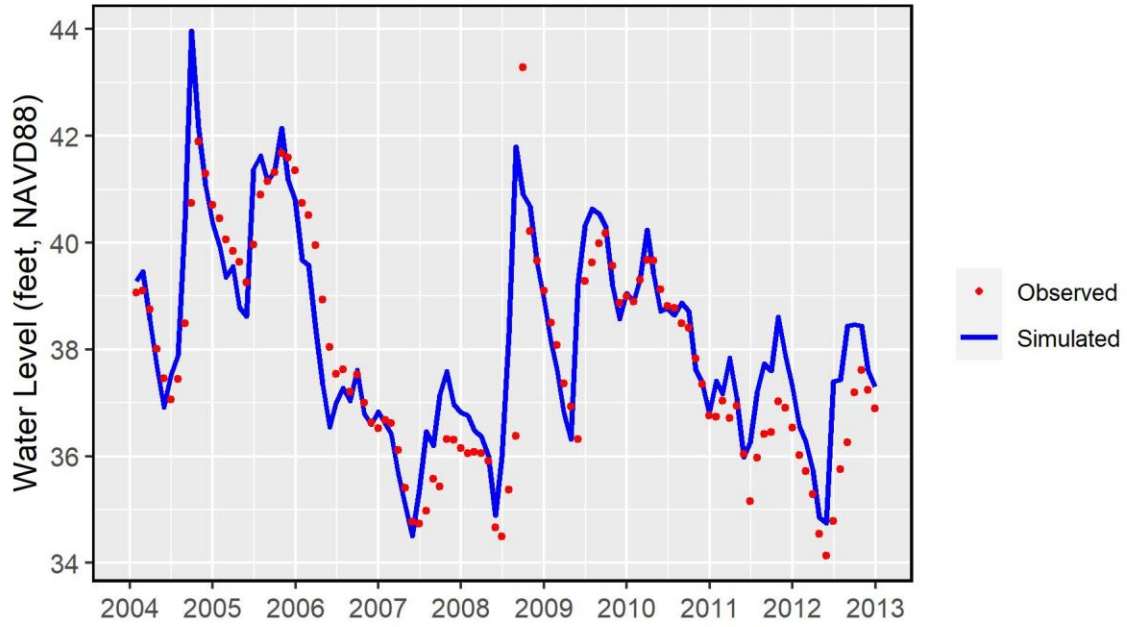
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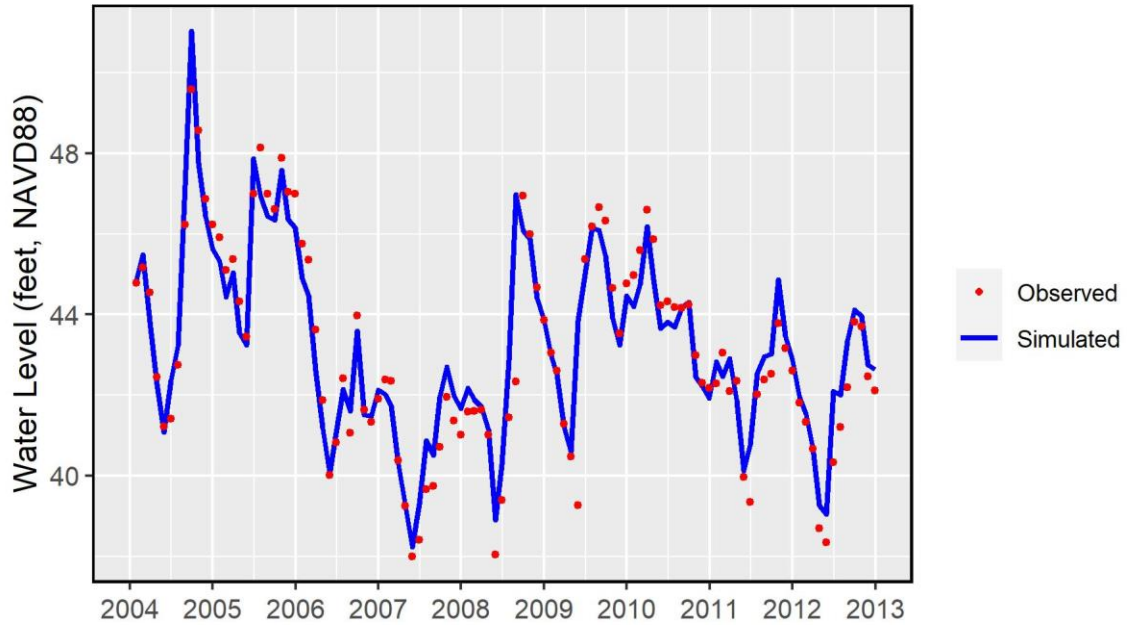
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ME=0.192 MAE=0.777 nMAE=0.817 R2=0.667 NS=0.583



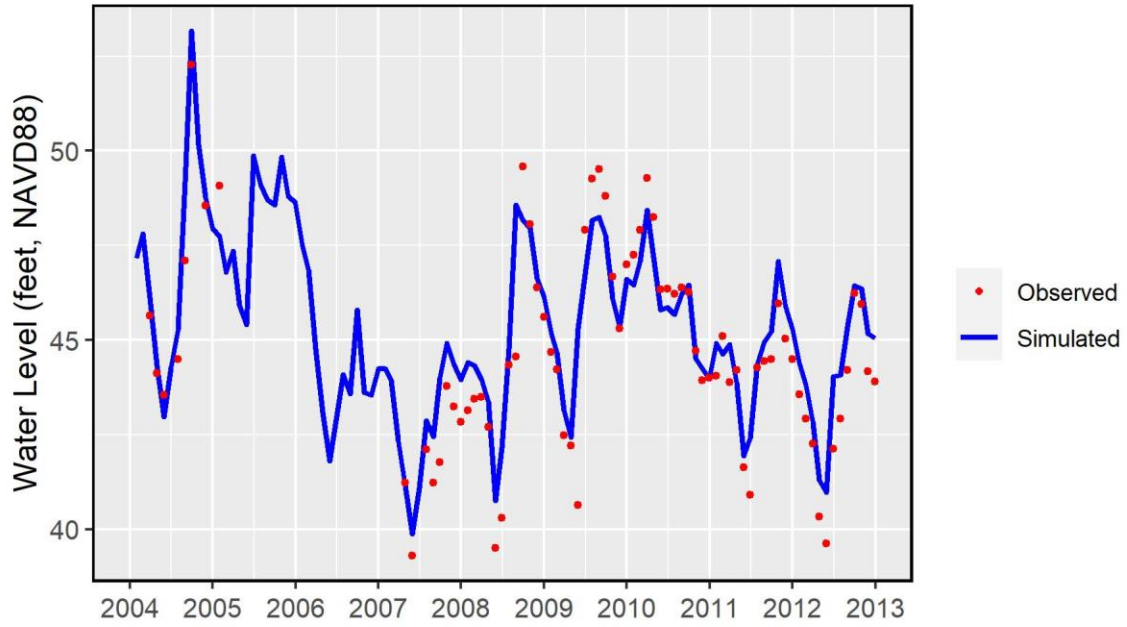
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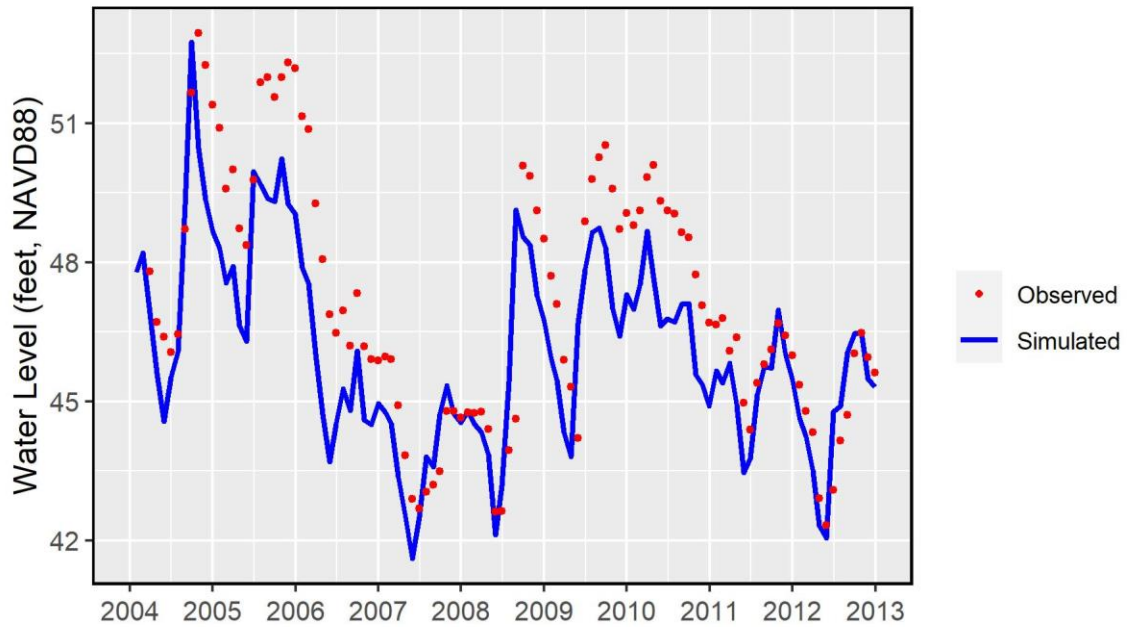
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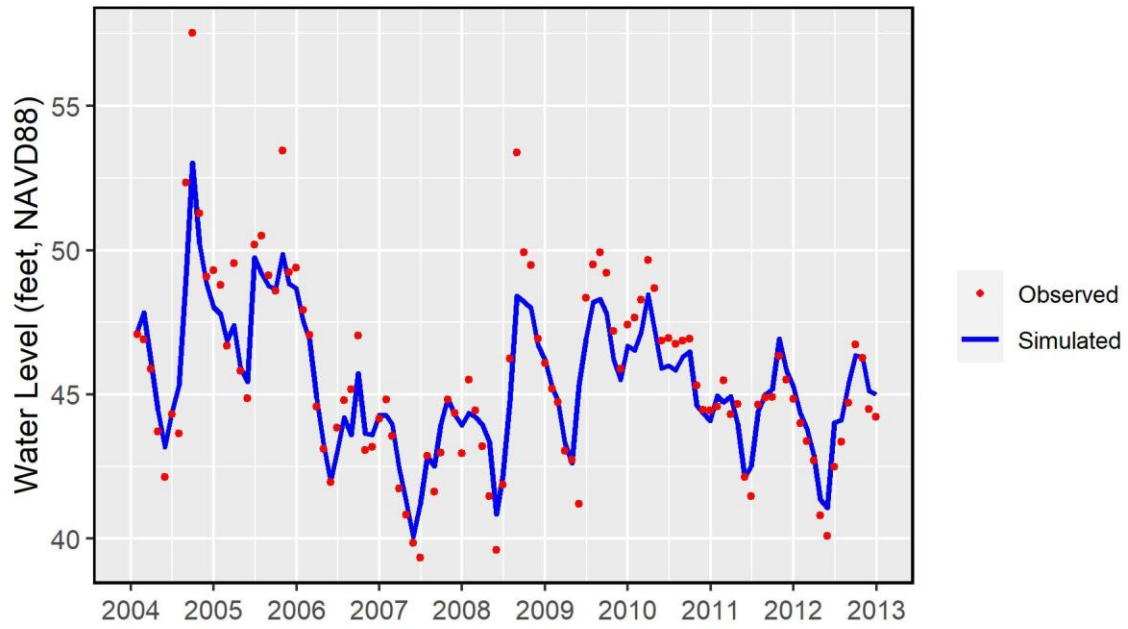
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LFA: SJRWMD51004173
ME=-1.096 MAE=1.44 nMAE=1.028 R2=0.767 NS=0.591



LFA: USGS283333081233501
ME=-0.218 MAE=0.835 nMAE=0.85 R2=0.91 NS=0.854



Appendix C – Simulated Versus Observed Flow Hydrographs for Continuously Measured Springs

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Figure 5. Simulated and observed flow at Alexander Spring for the model simulation period.

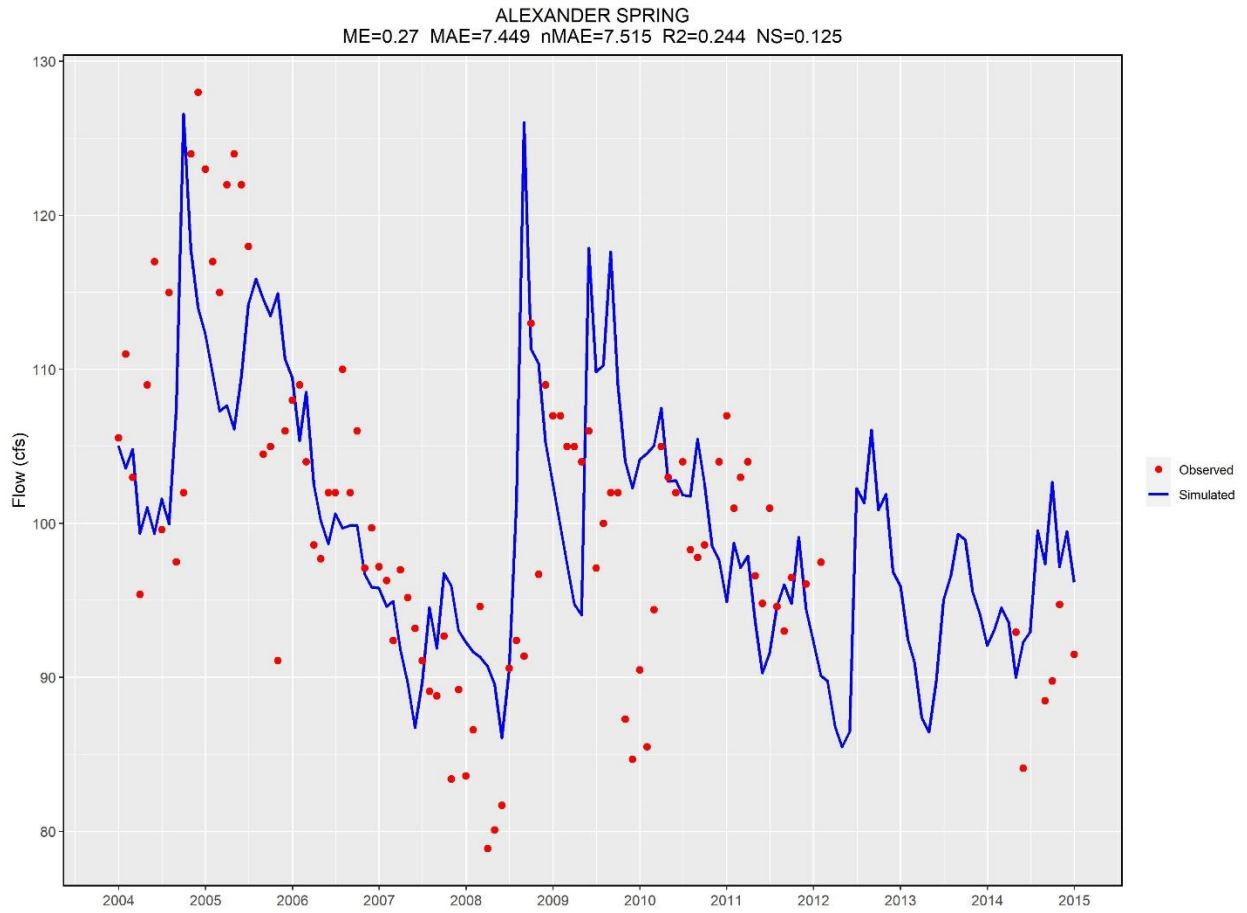


Figure 6. Simulated and observed flow at Apopka Spring for the model simulation period.

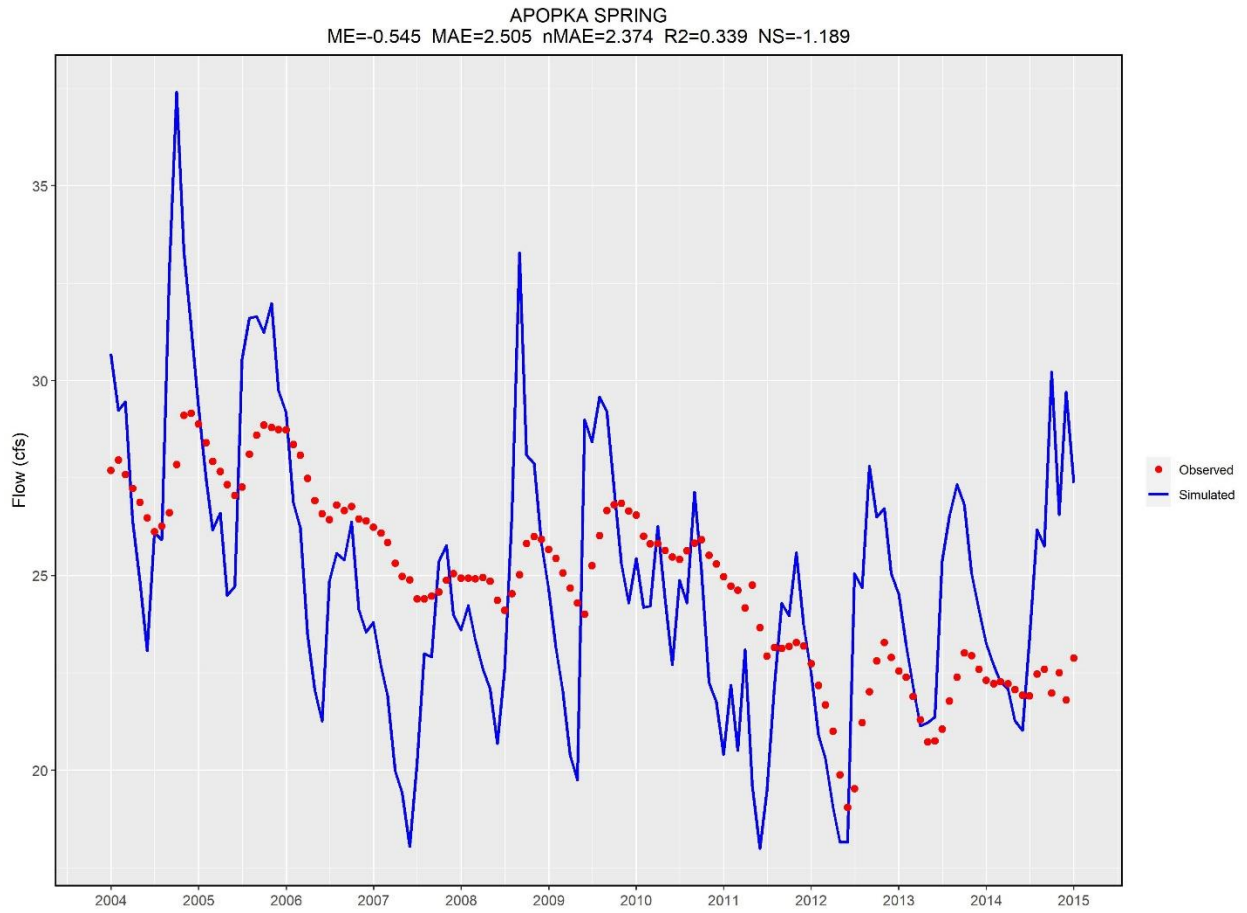


Figure 7. Simulated and observed flow at Buckhorn Main Spring for the model simulation period.

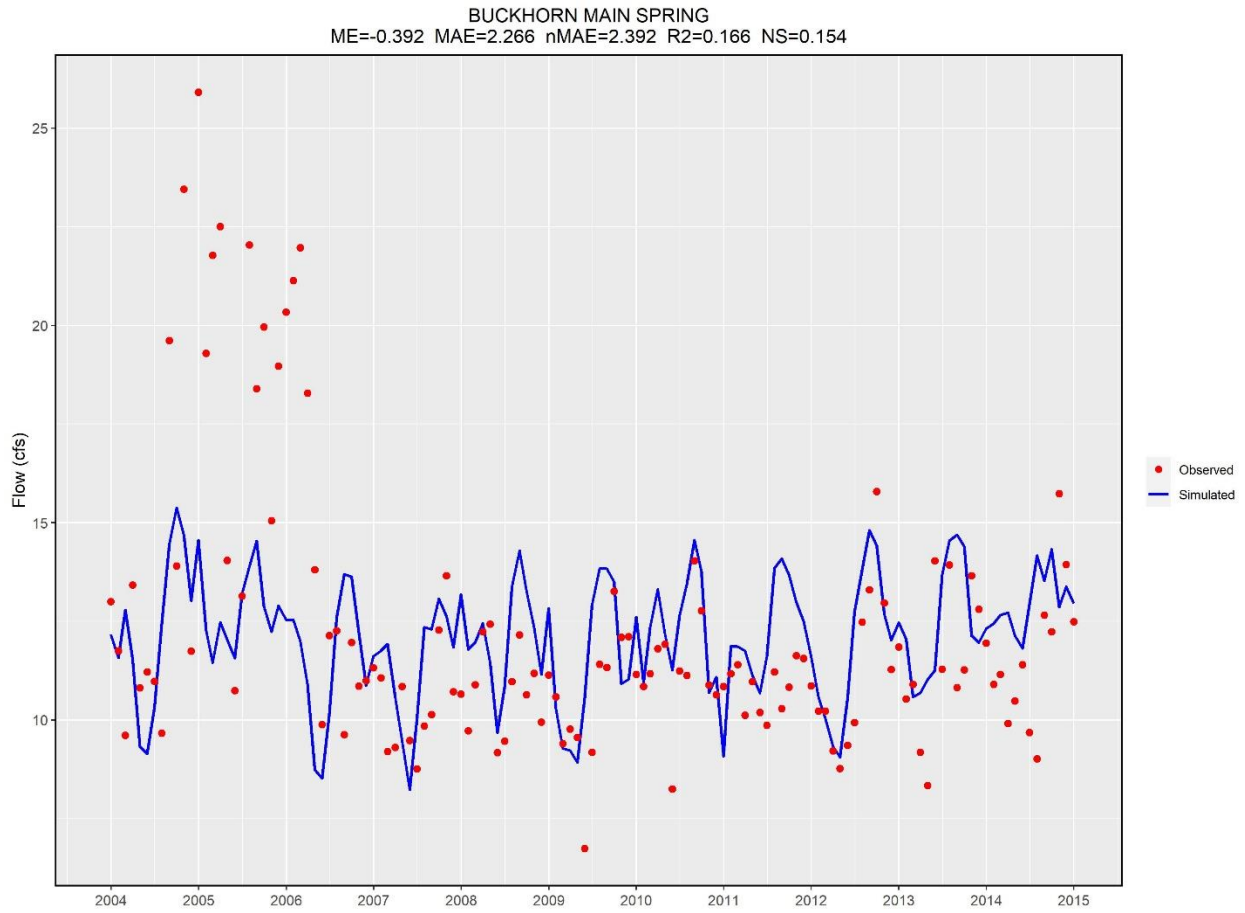


Figure 8. Simulated and observed flow at Bugg Spring for the model simulation period.

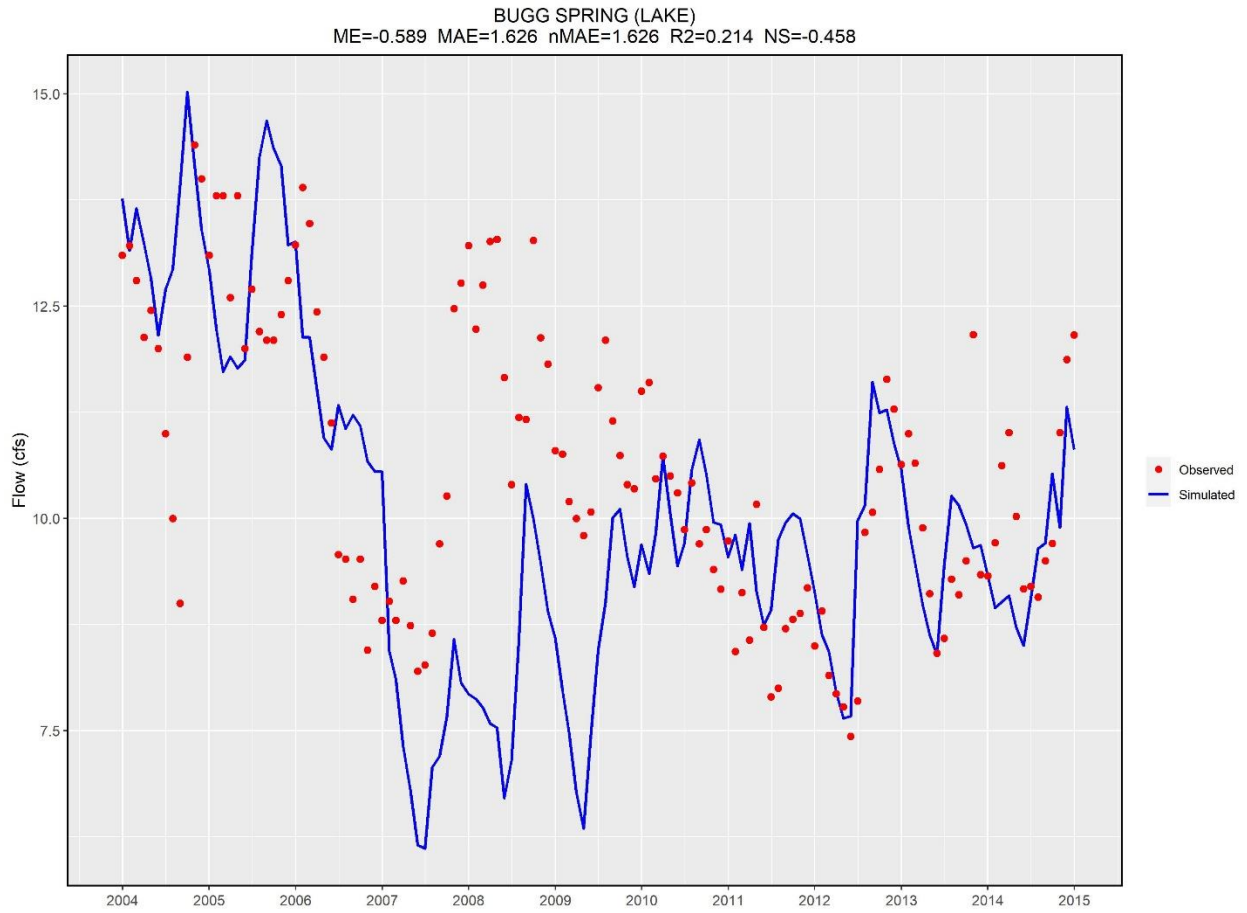


Figure 9. Simulated and observed flow at Chassahowitzka Spring for the model simulation period.

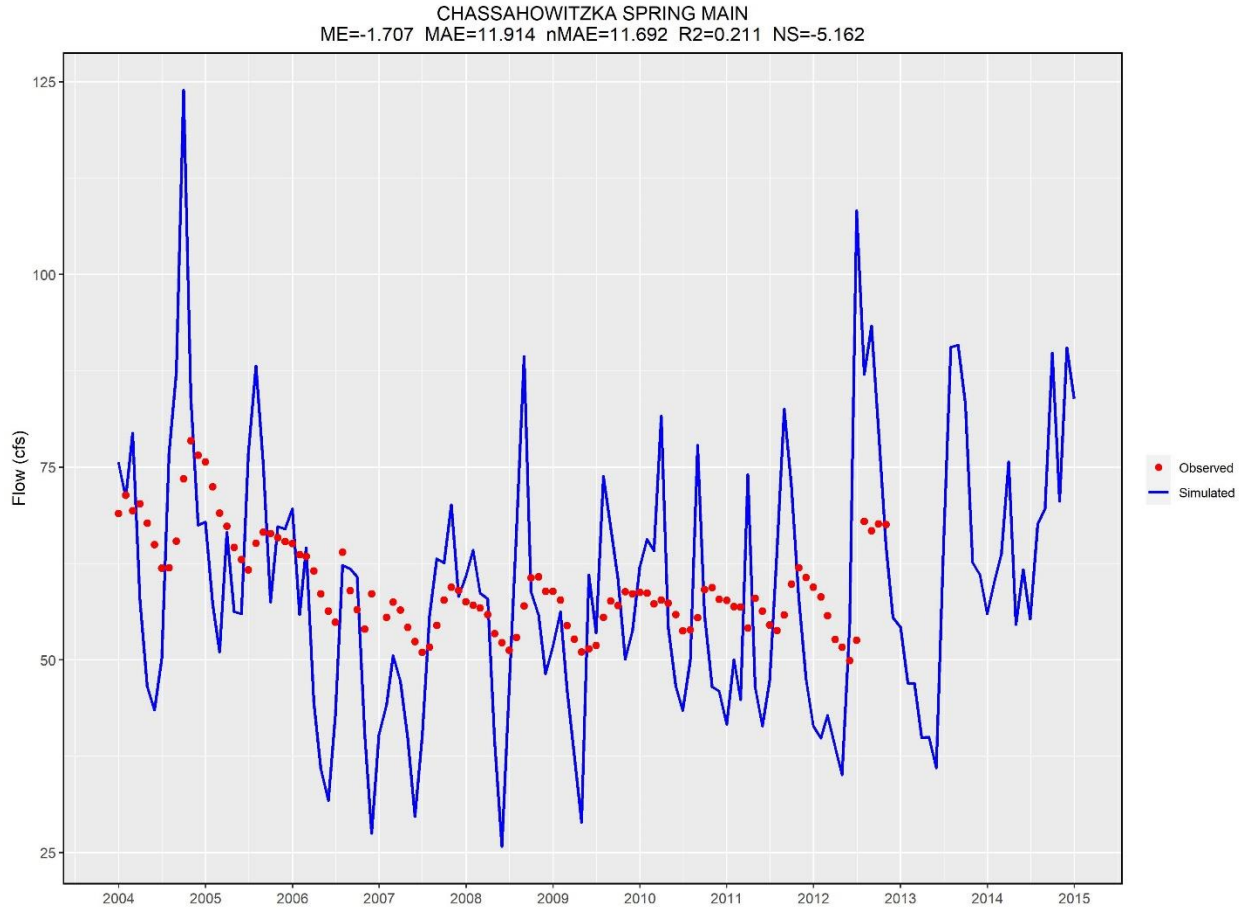


Figure 10. Simulated and observed flow at Crystal Main Spring for the model simulation period.

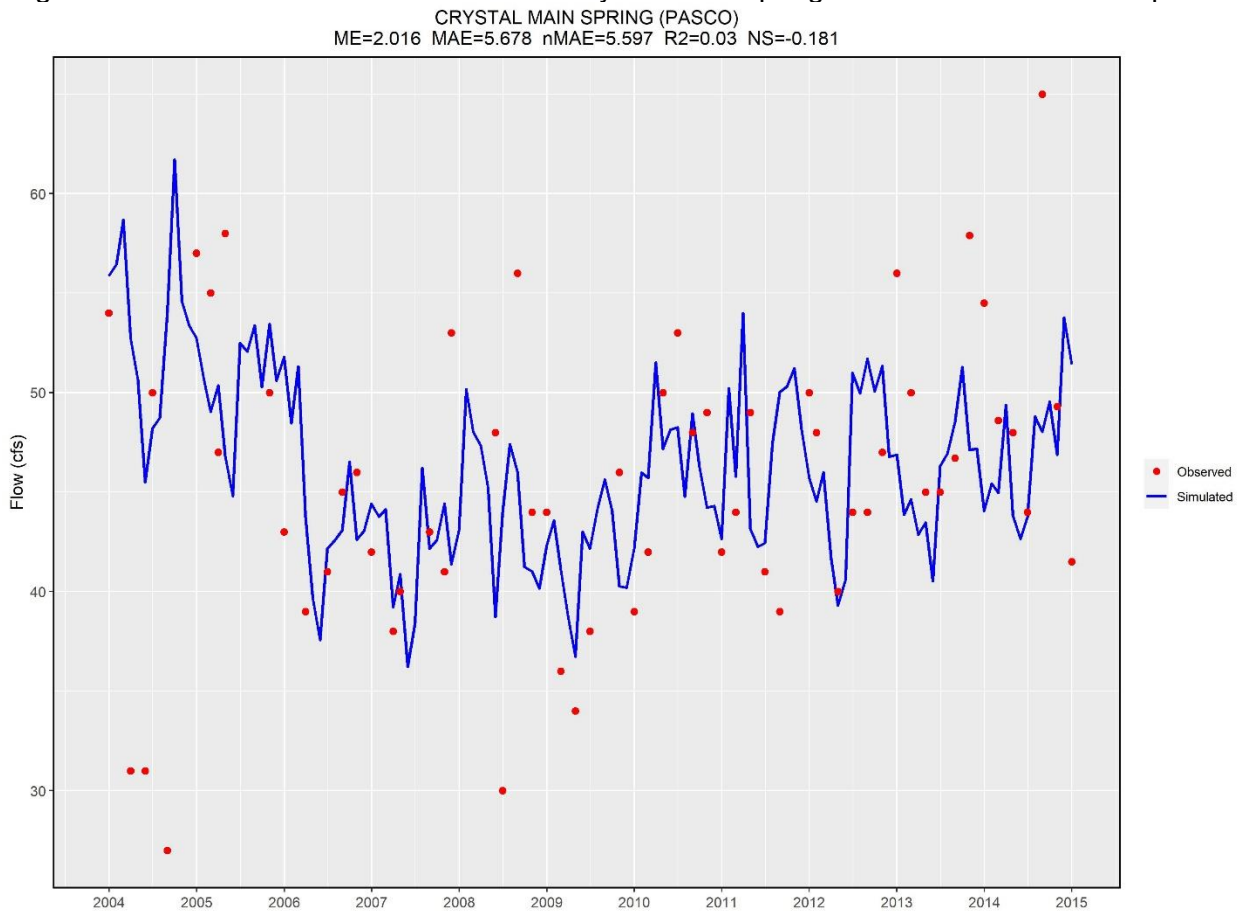


Figure 11. Simulated and observed flow at Gum Spring for the model simulation period.

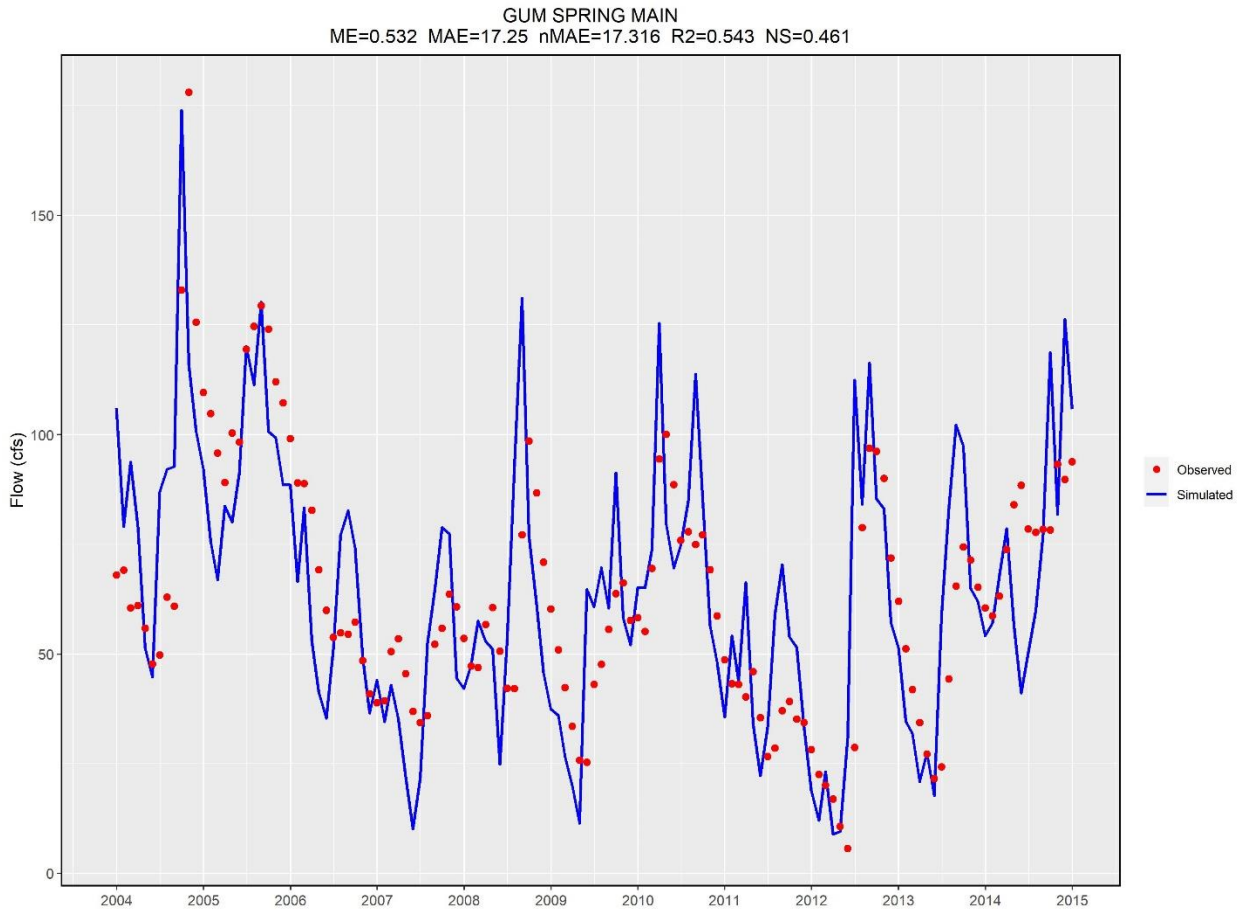


Figure 12. Simulated and observed flow at Homosassa Spring for the model simulation period.

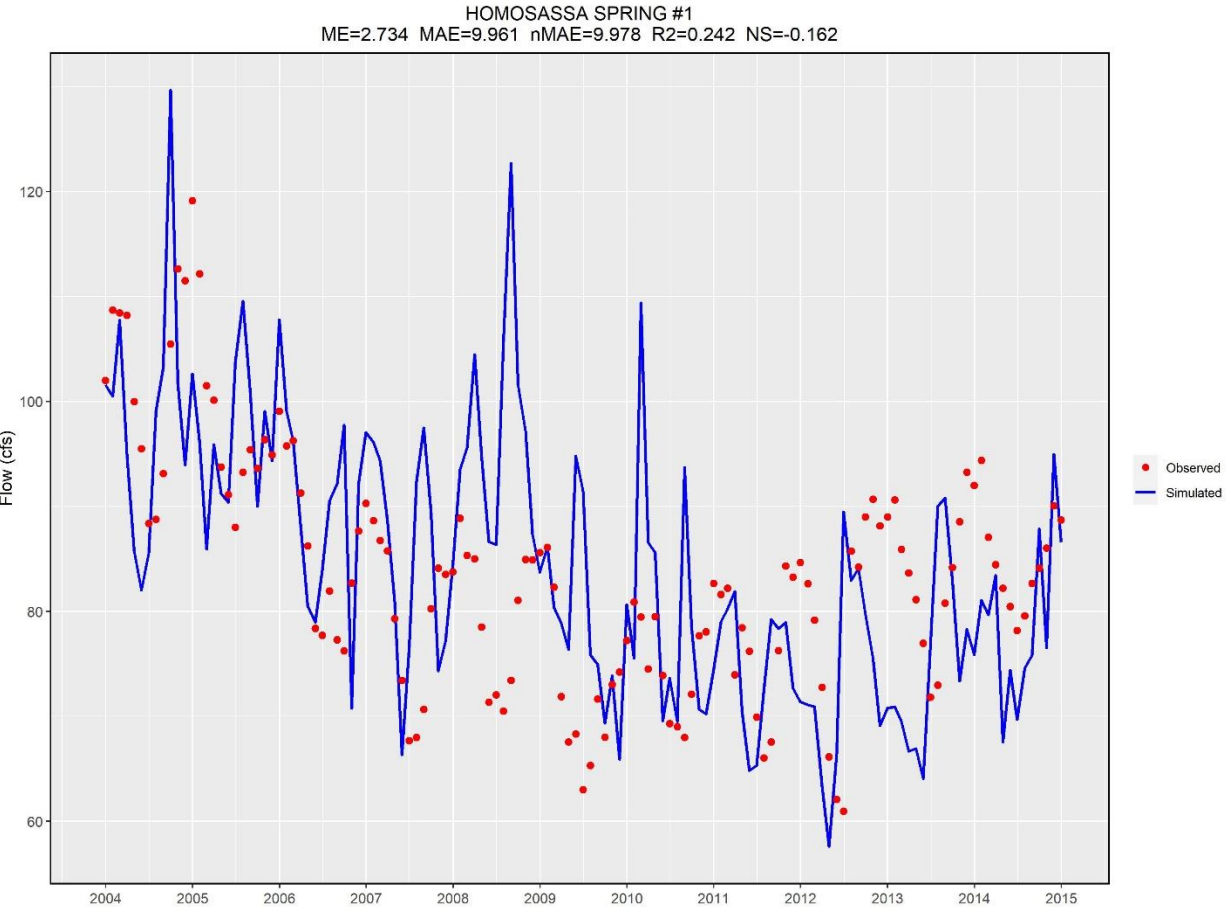


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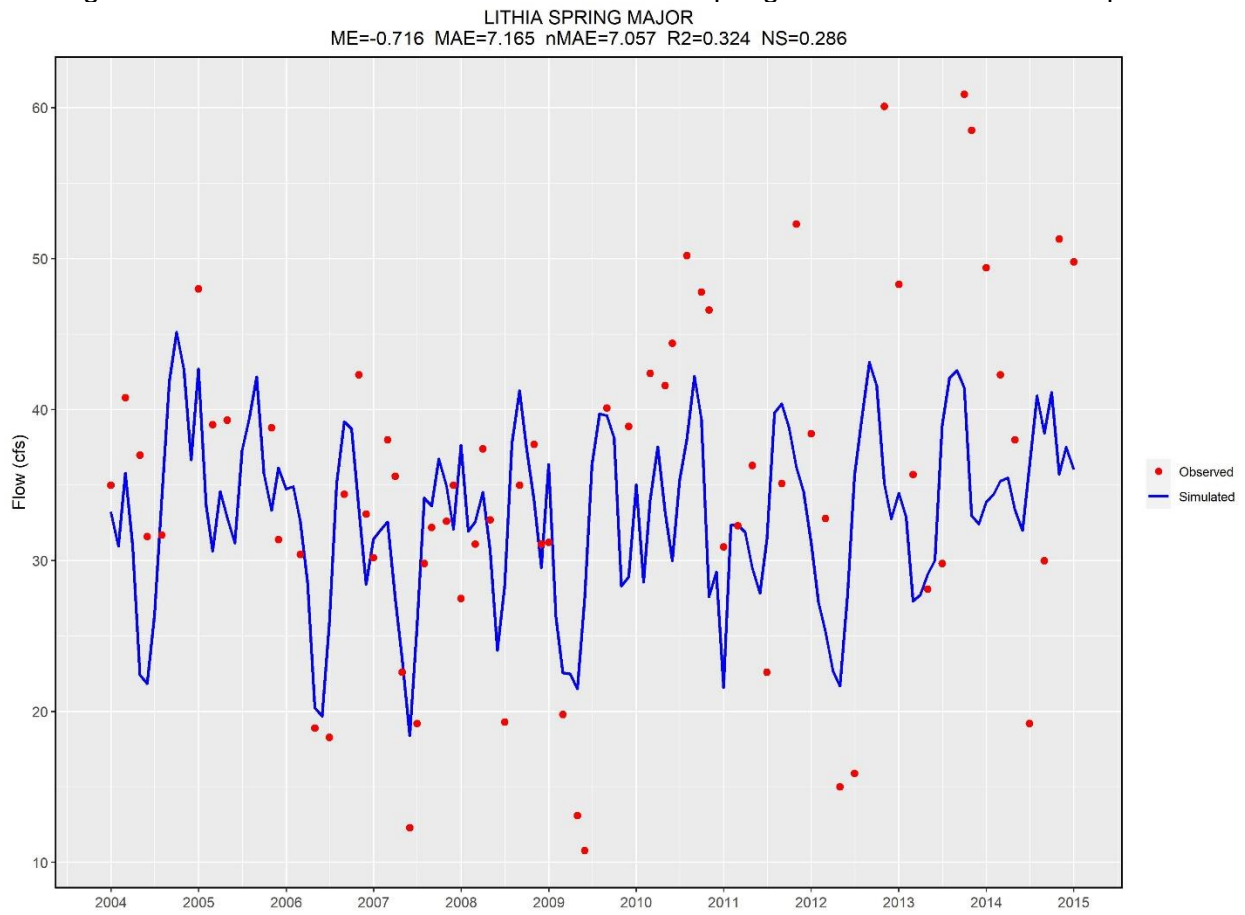


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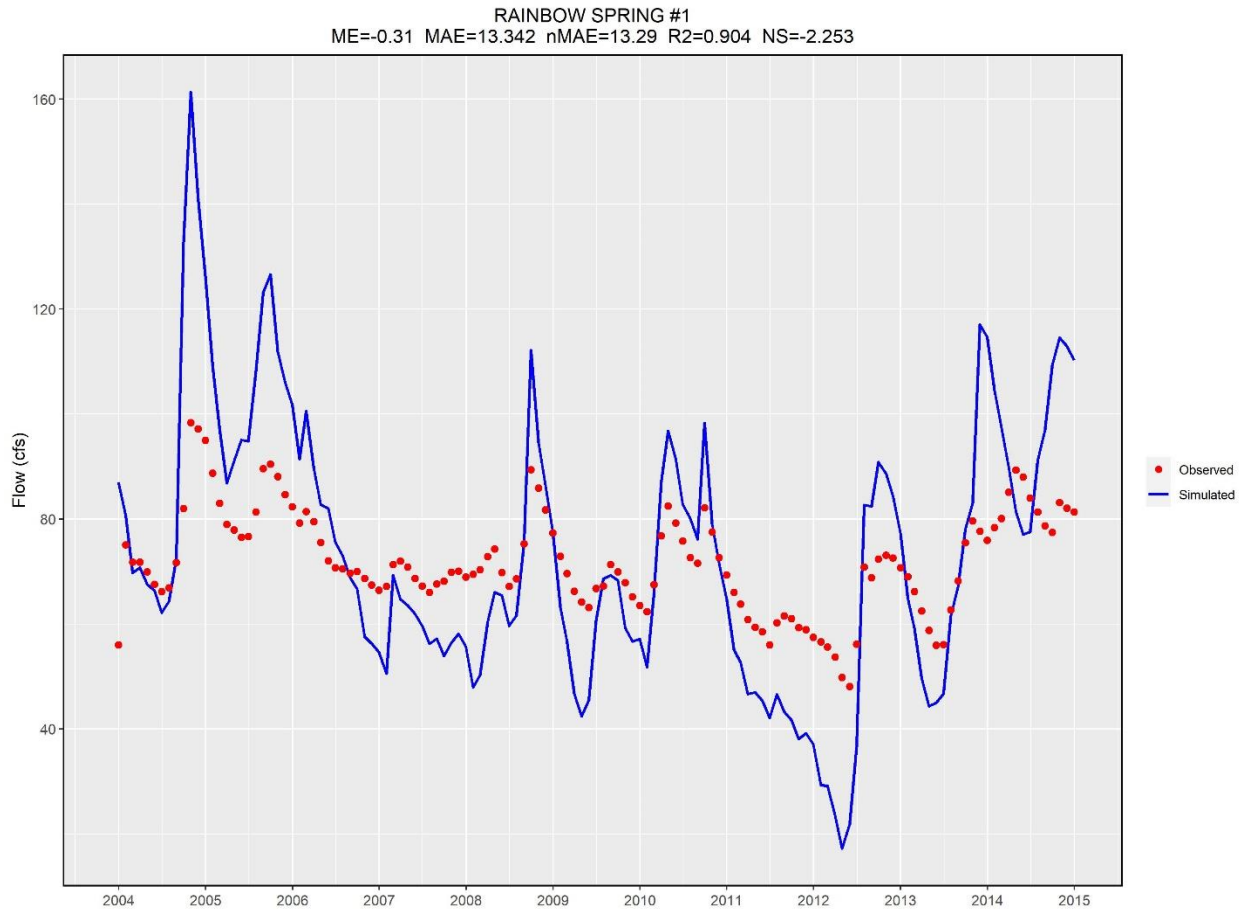


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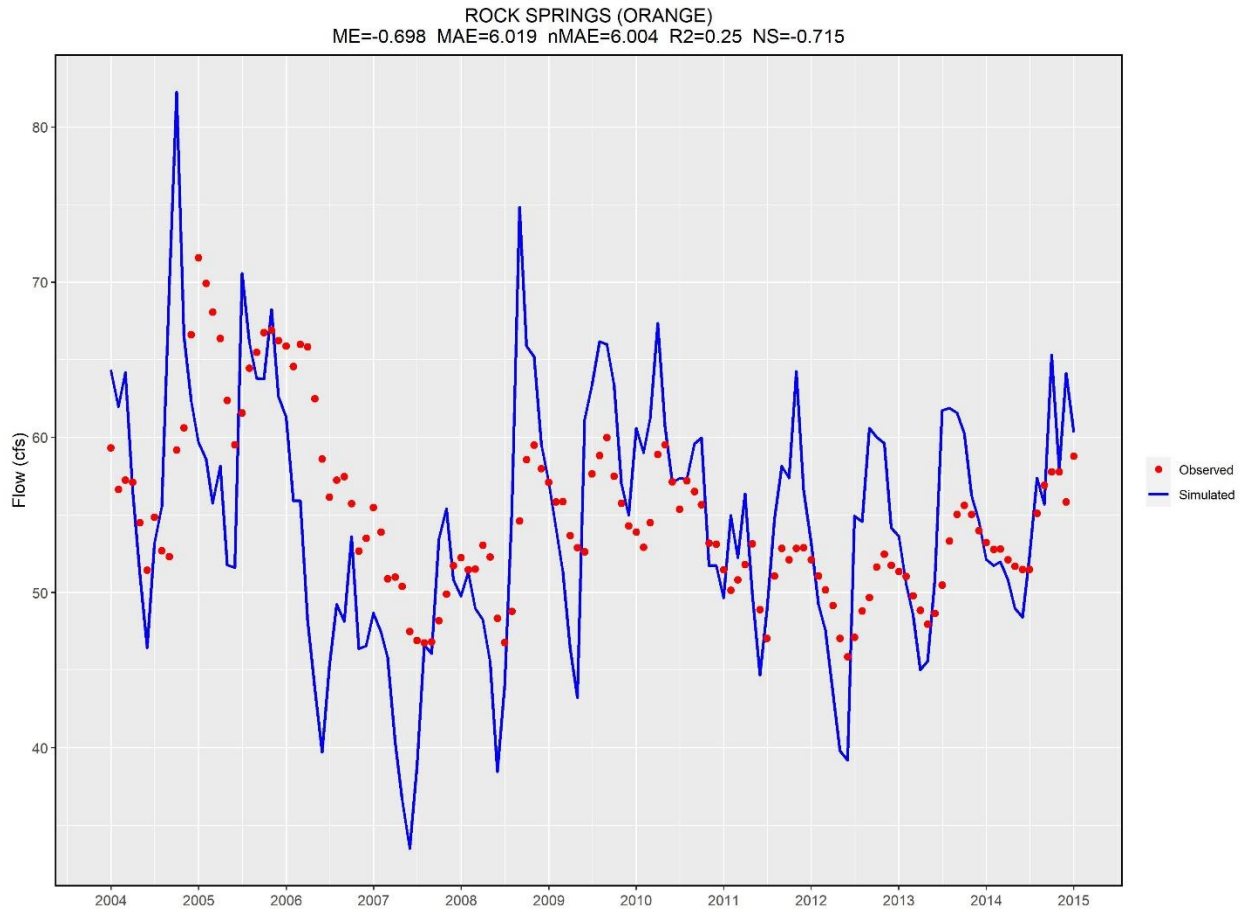


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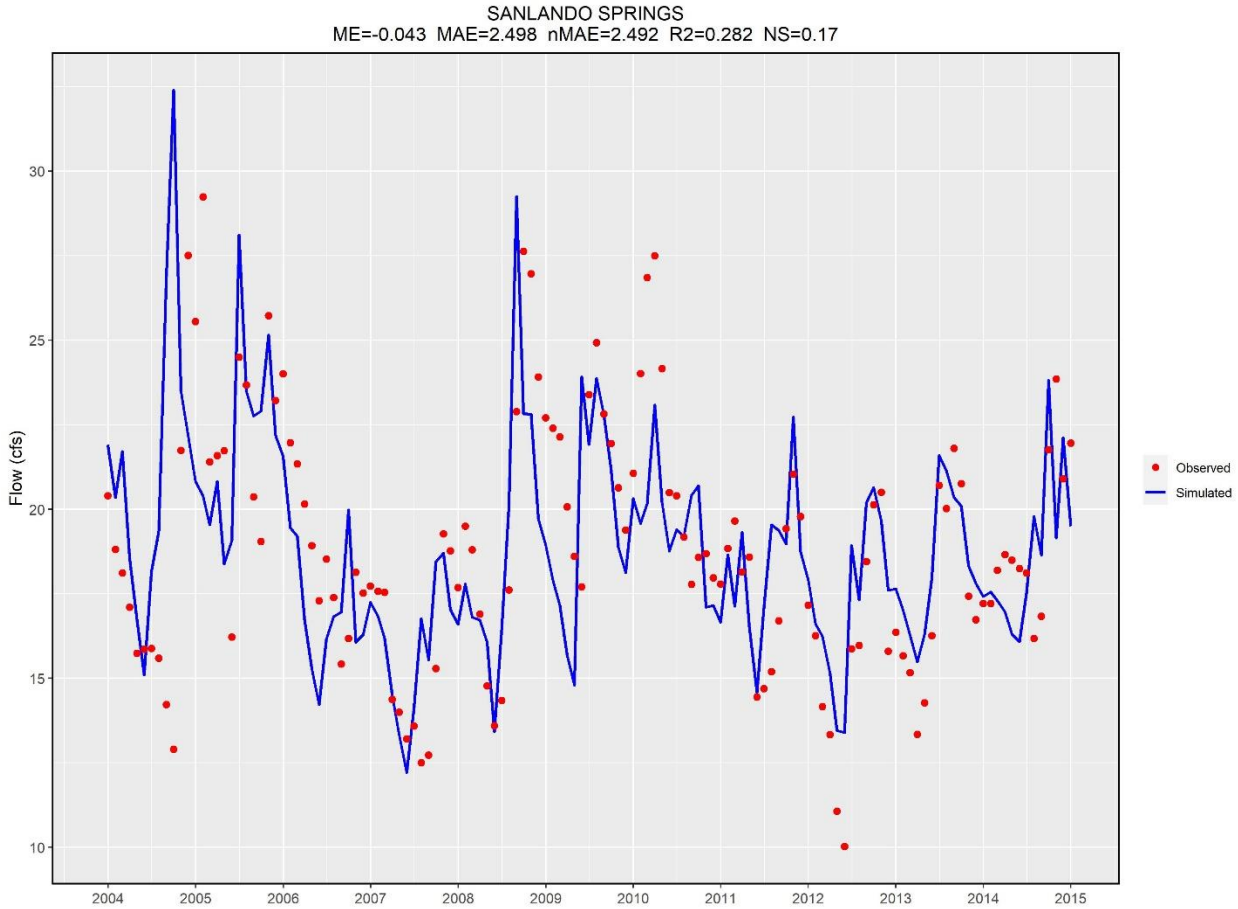


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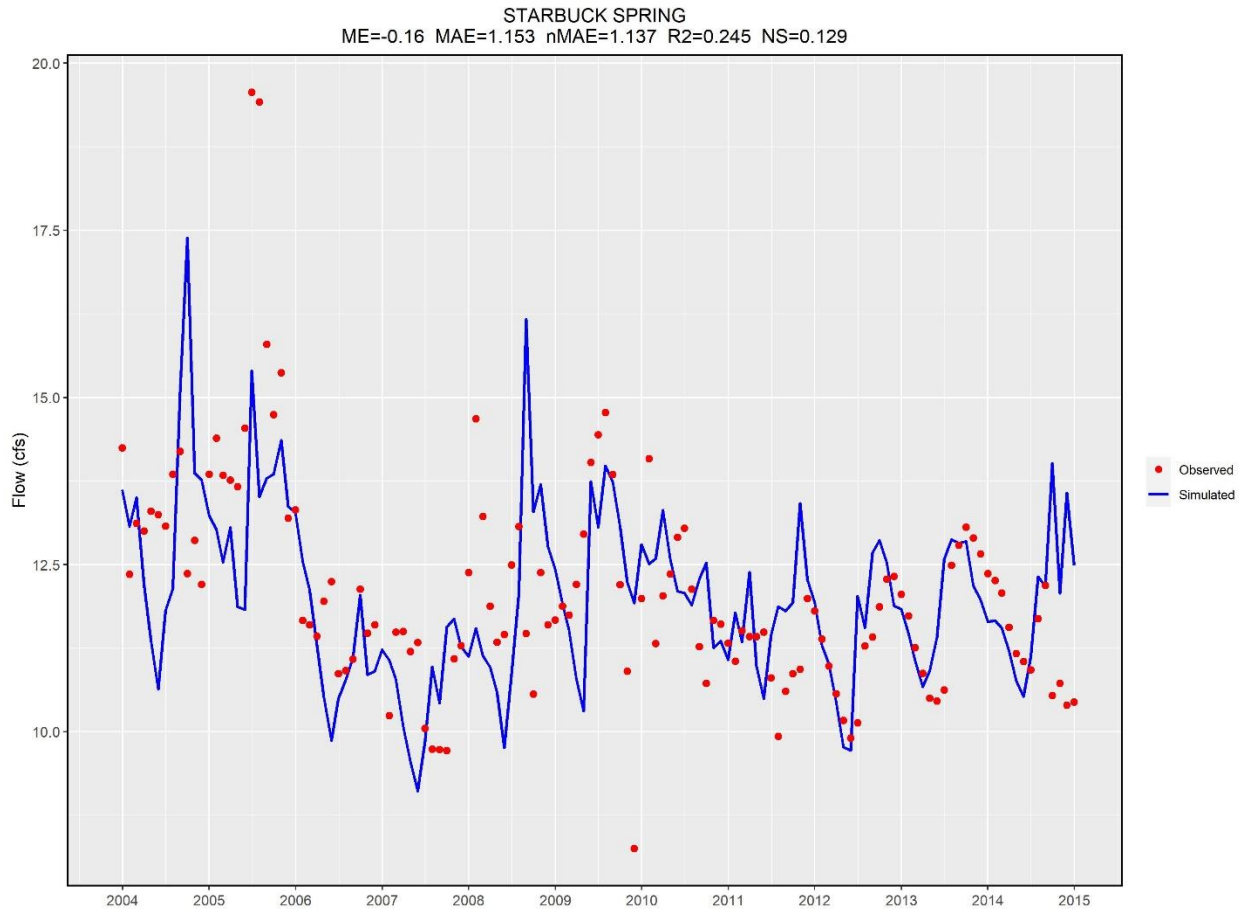


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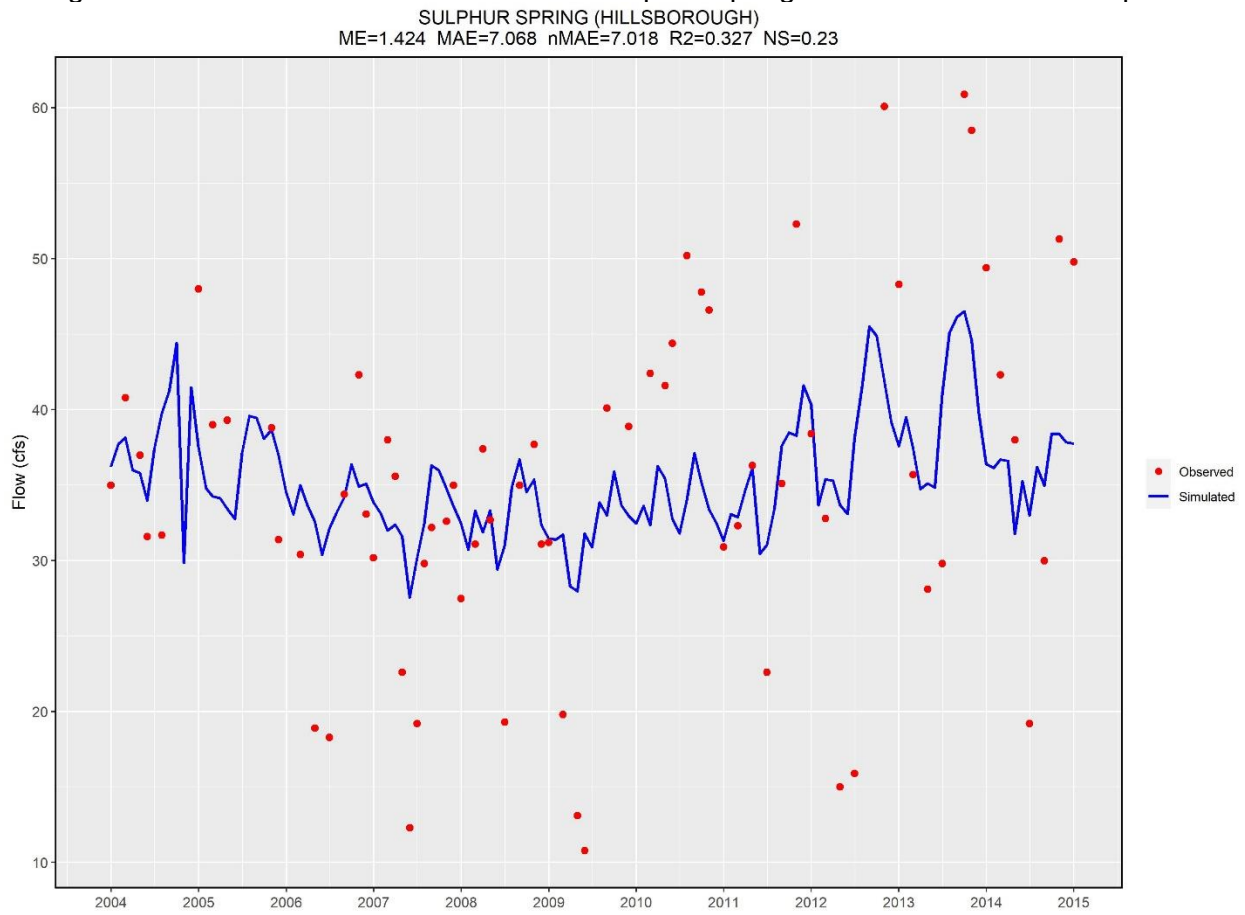


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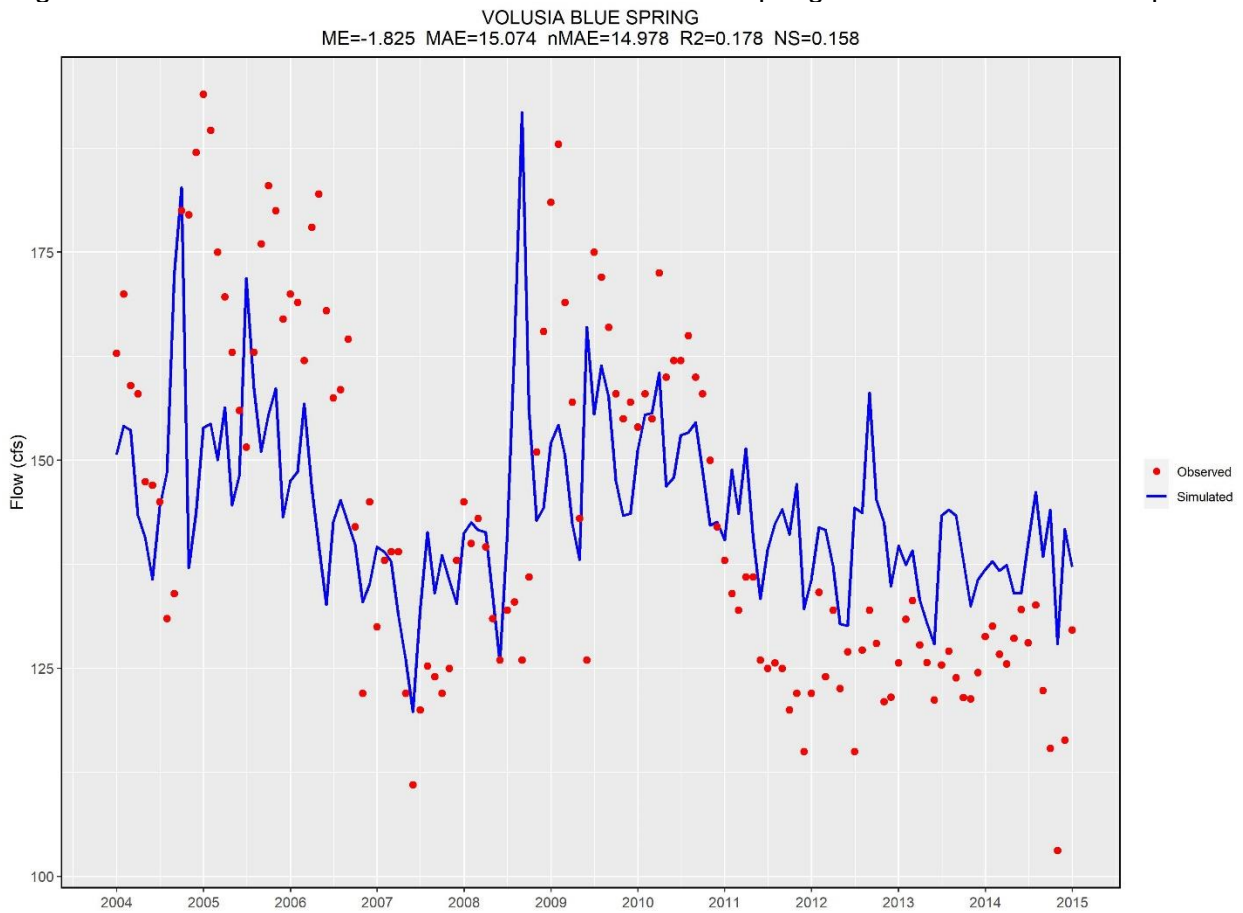


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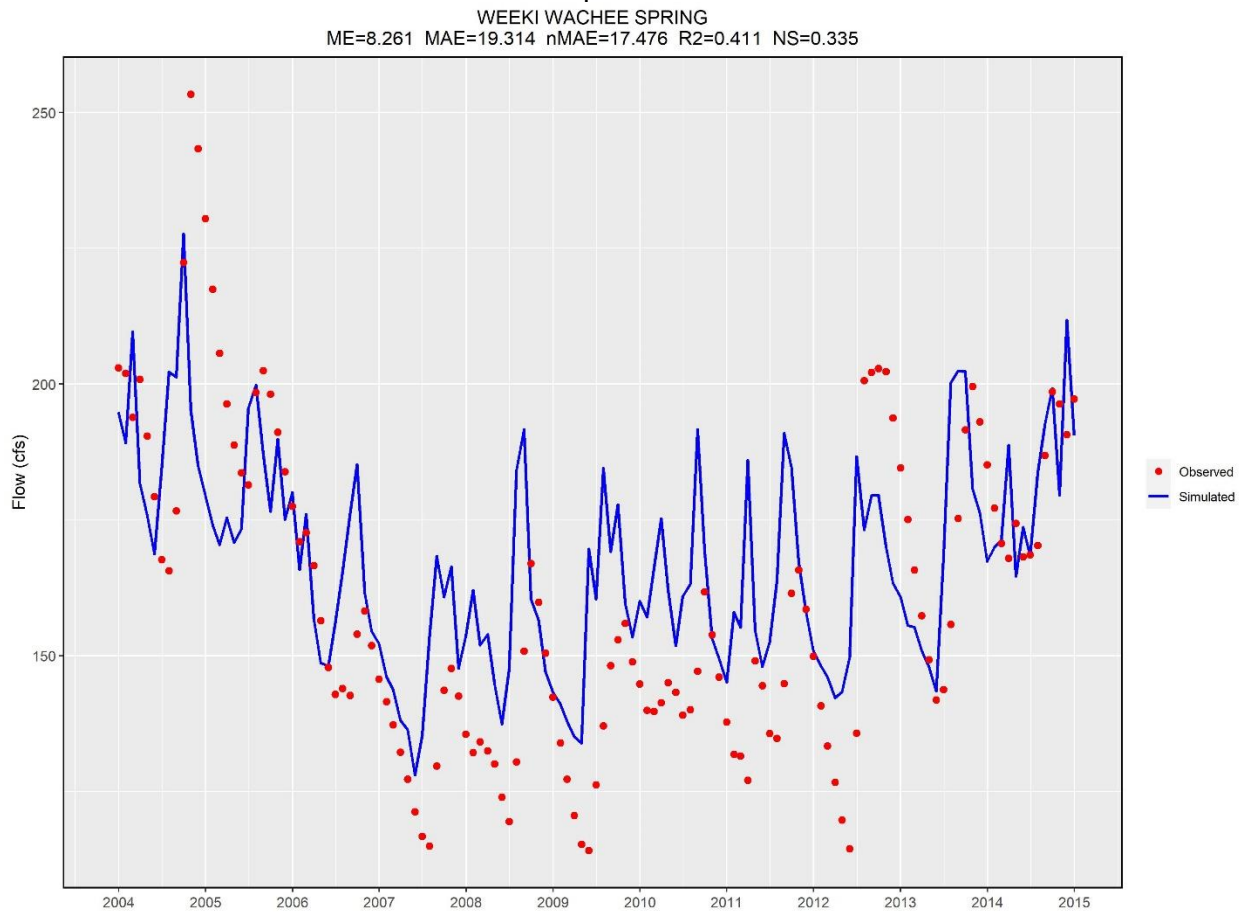


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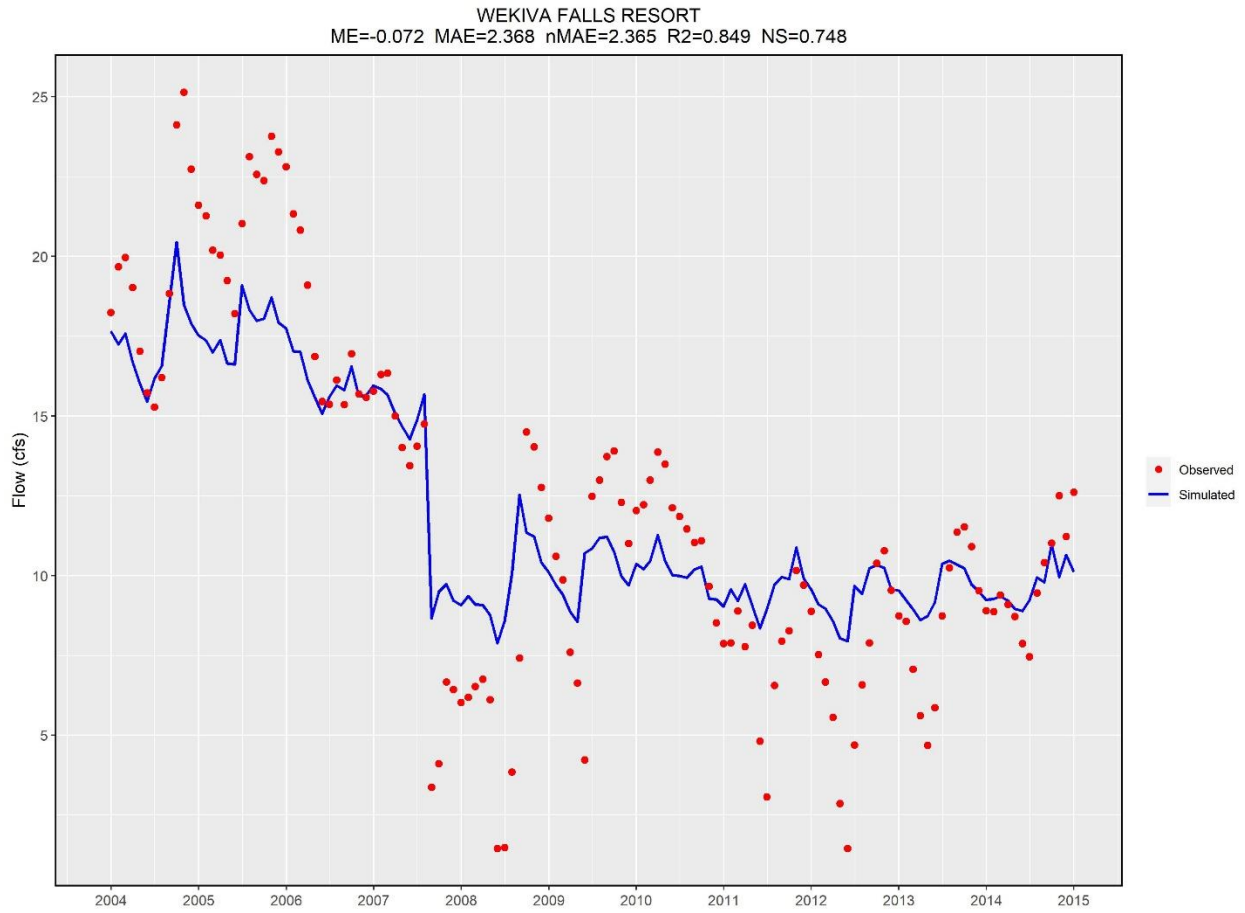
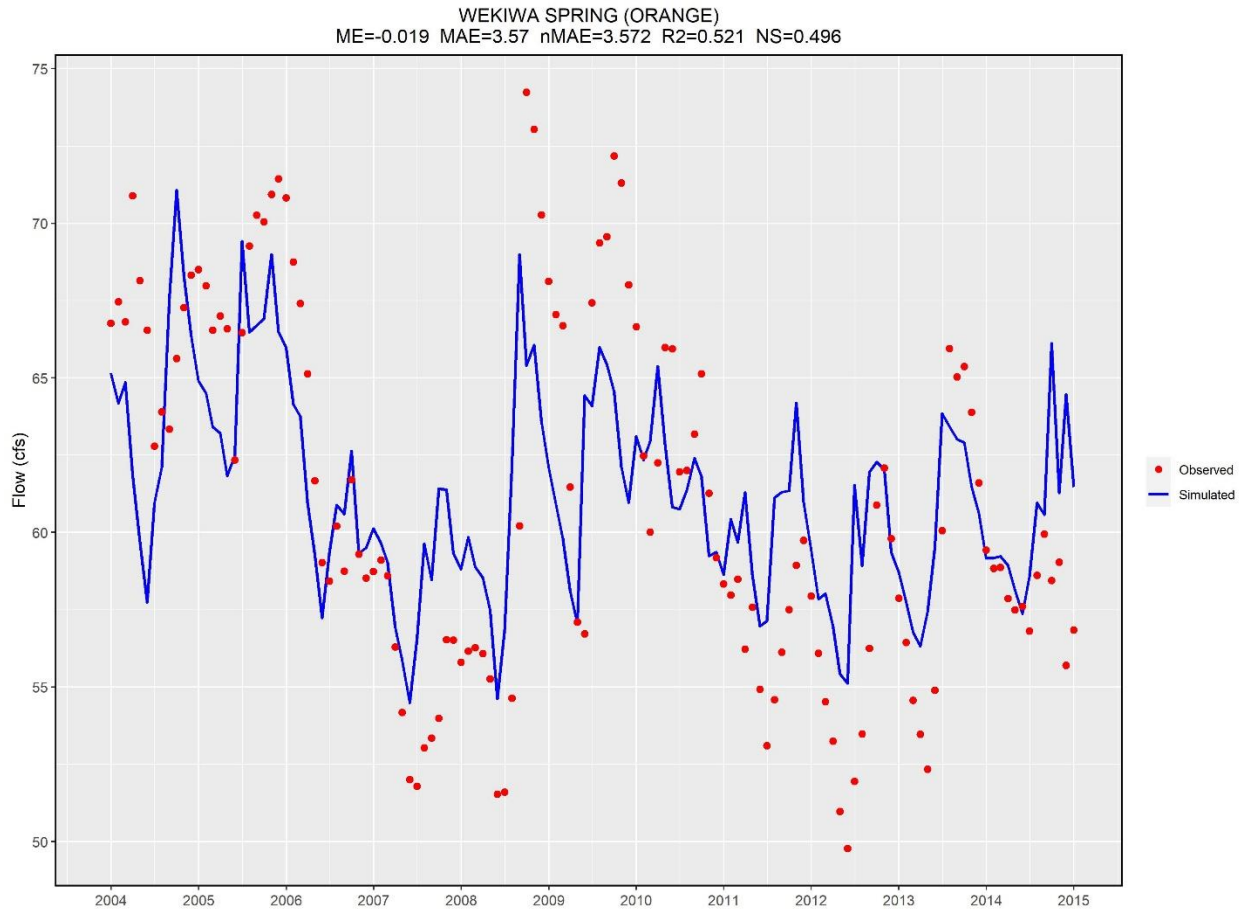


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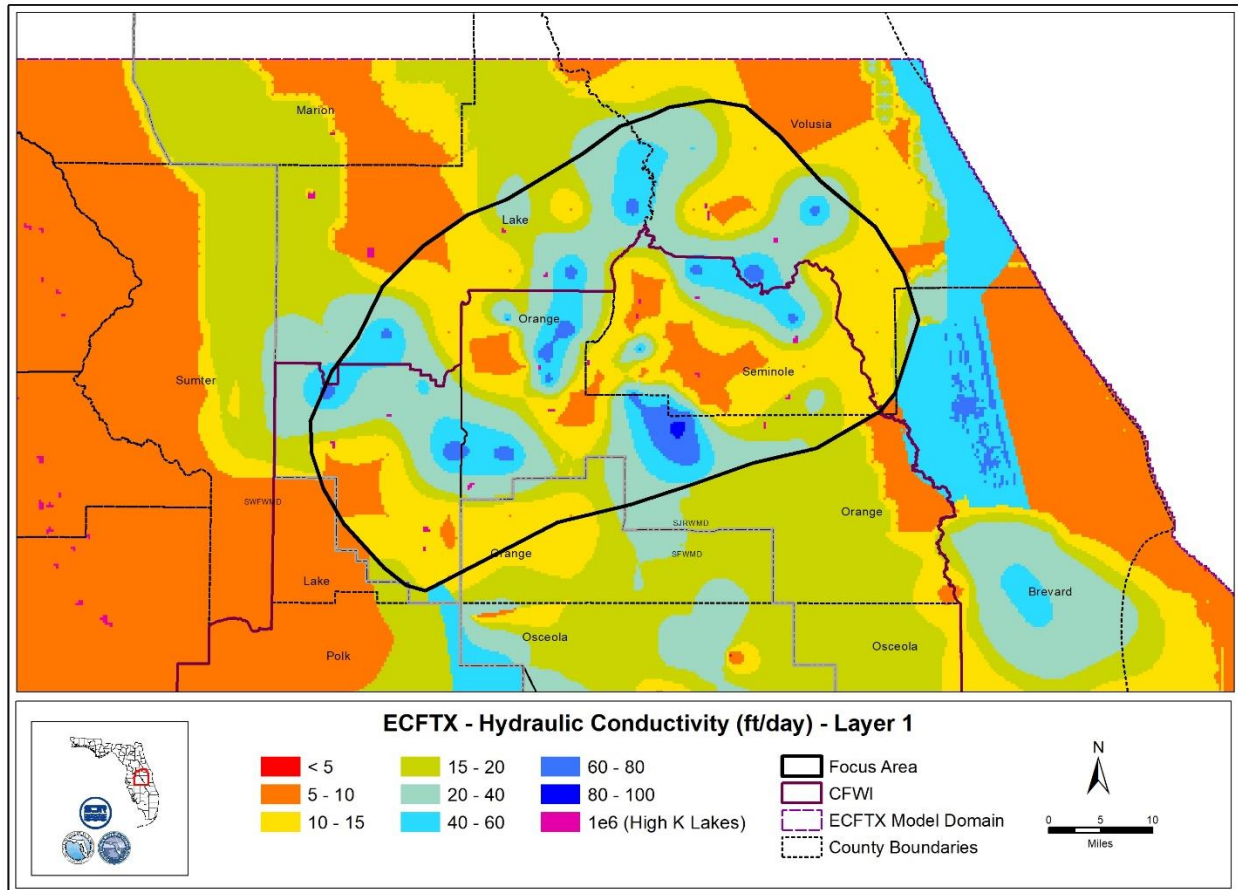


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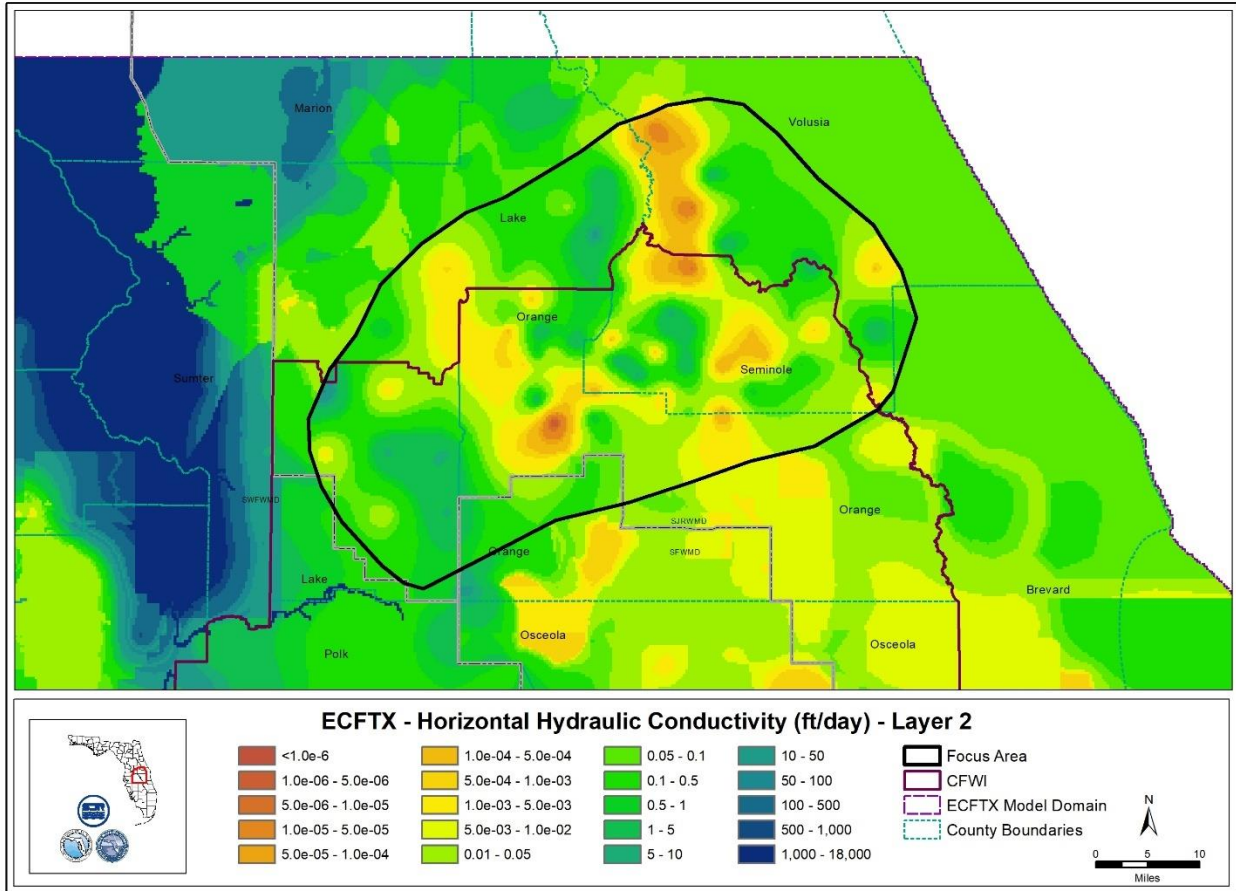


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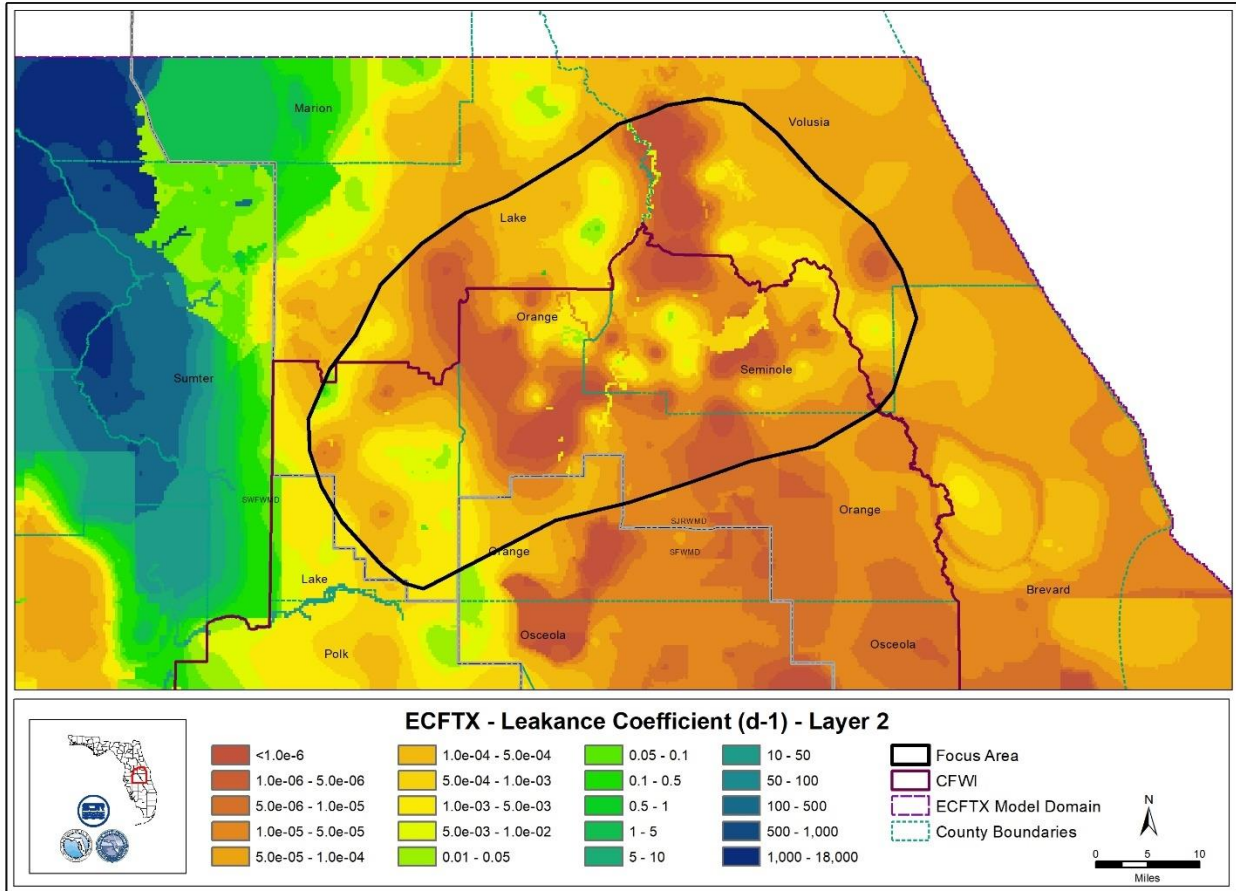


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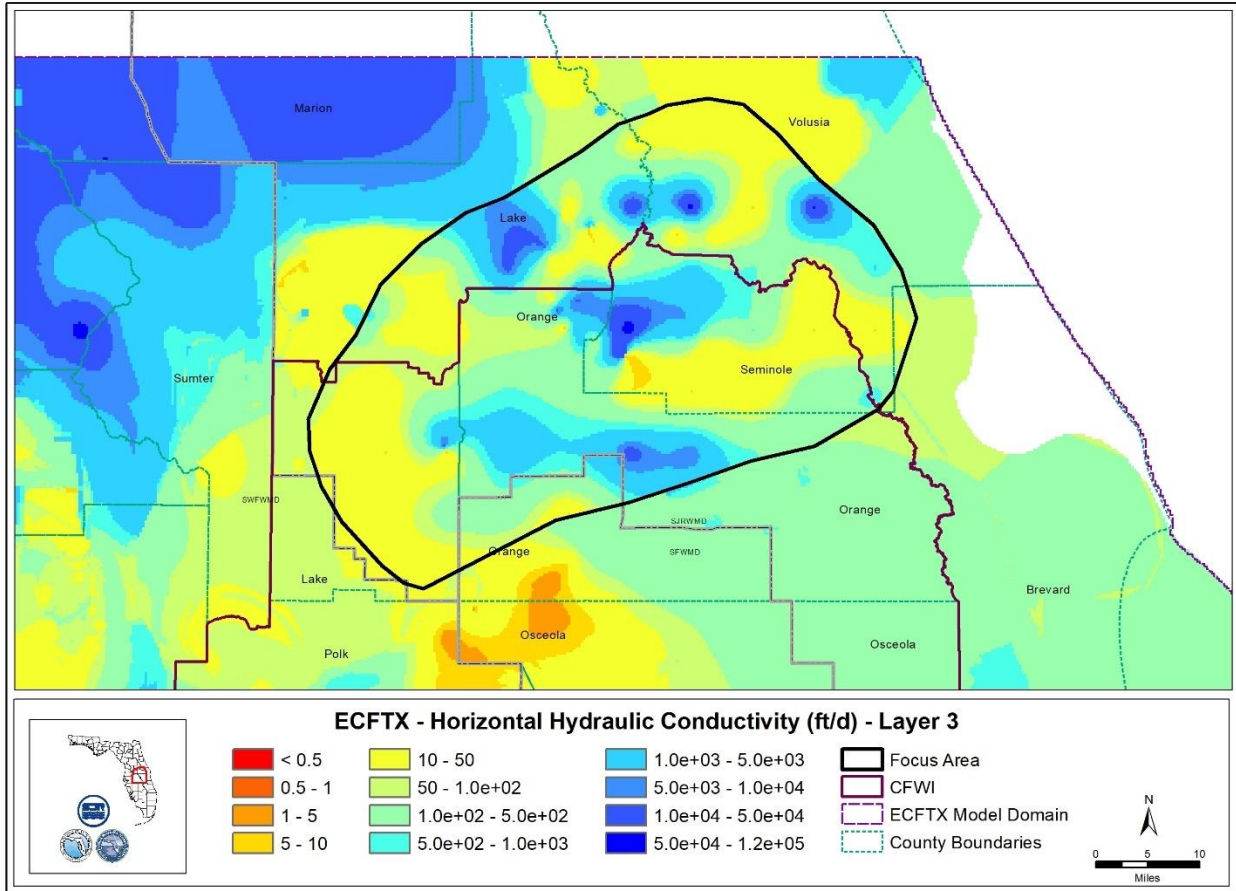


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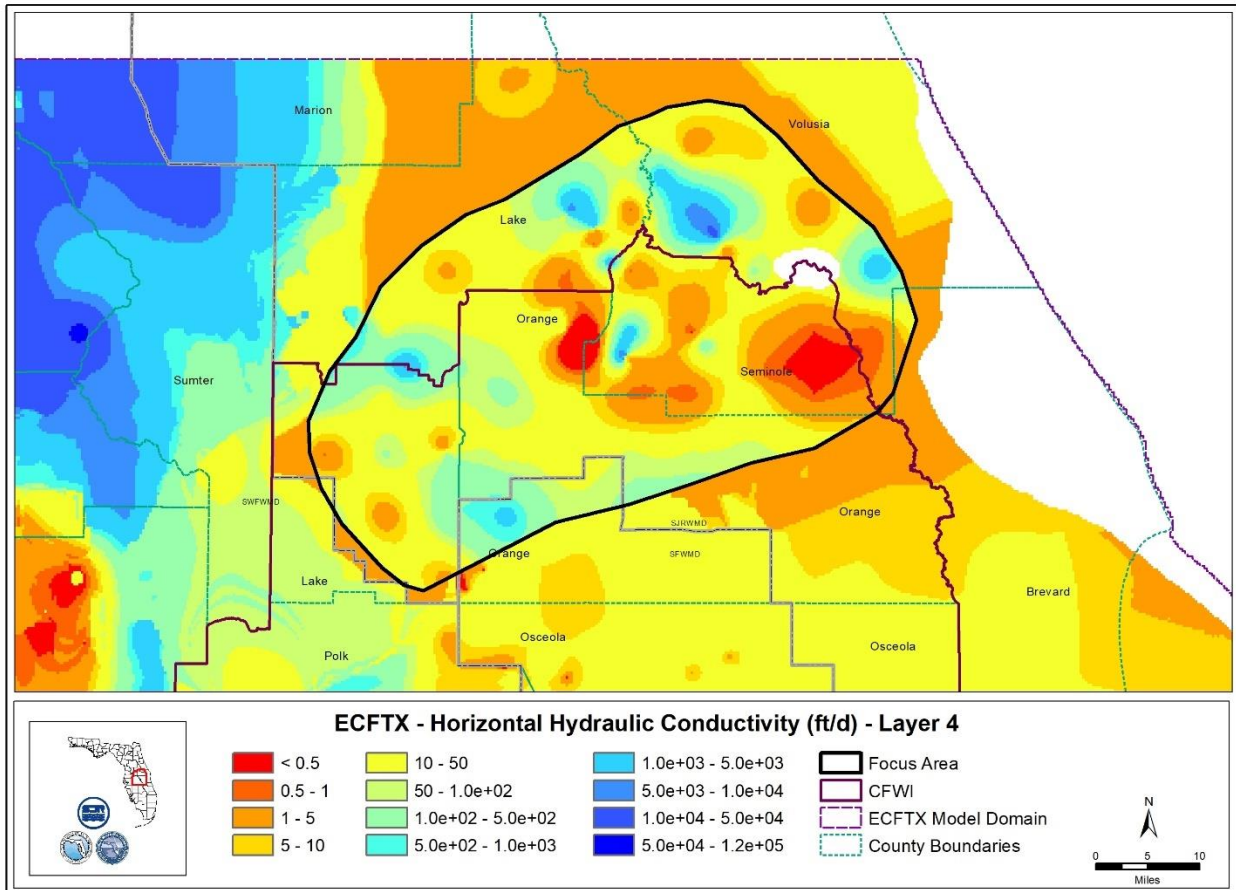


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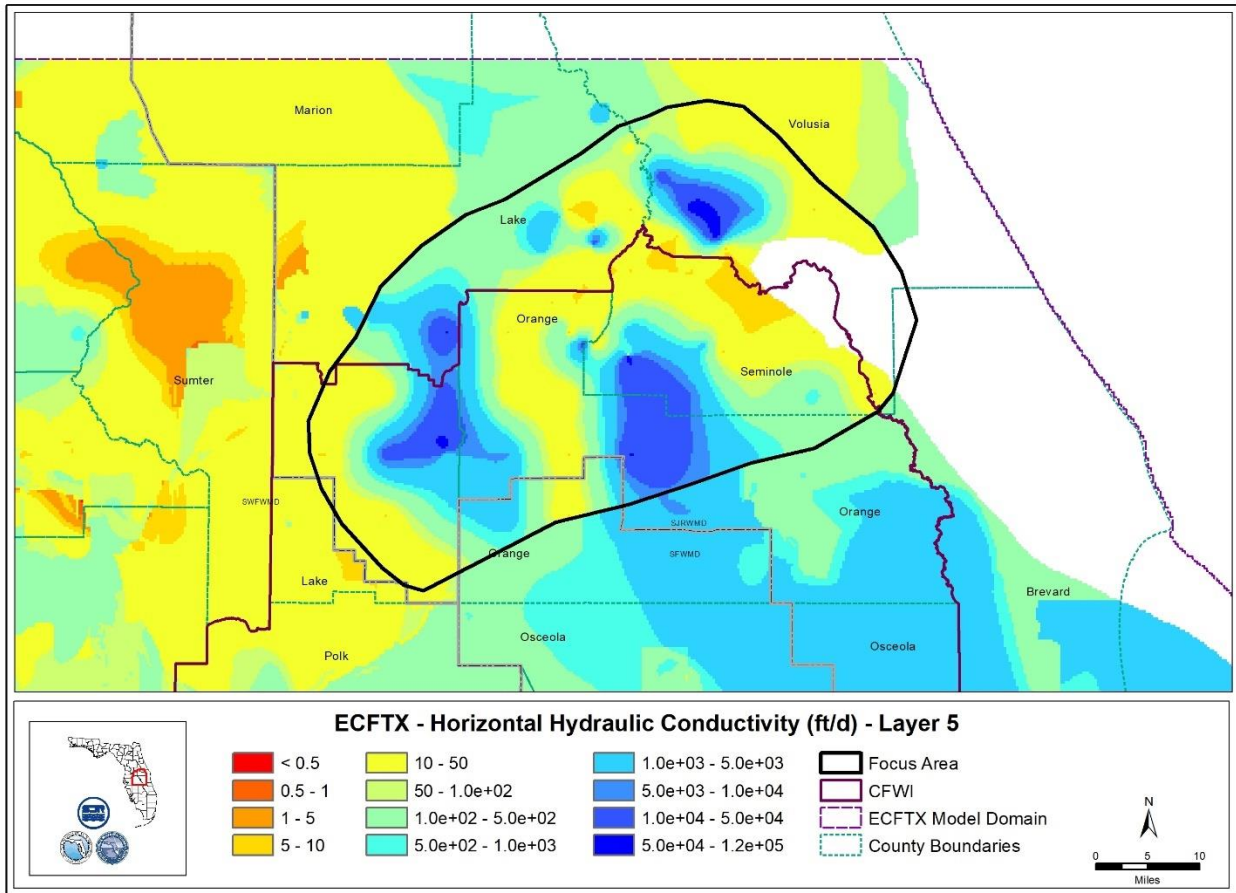


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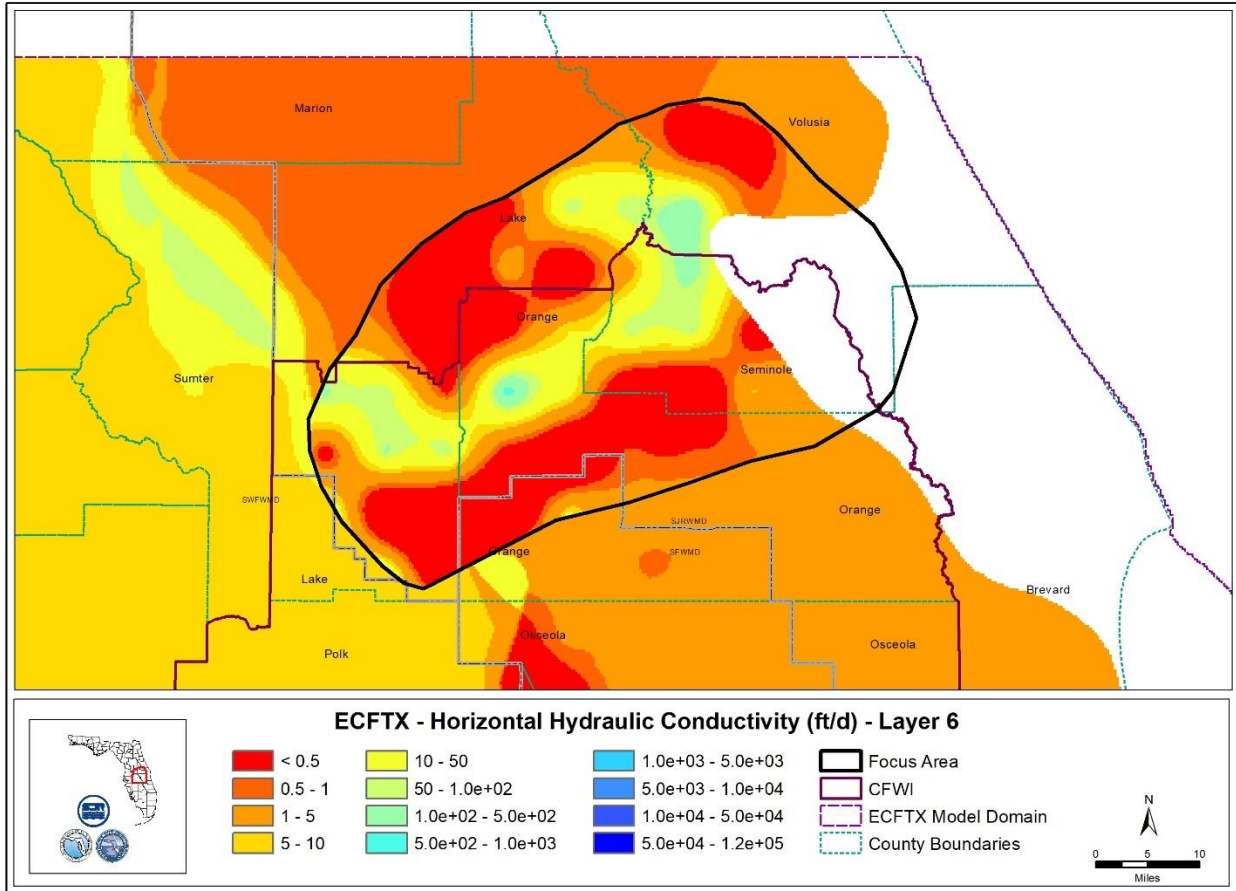


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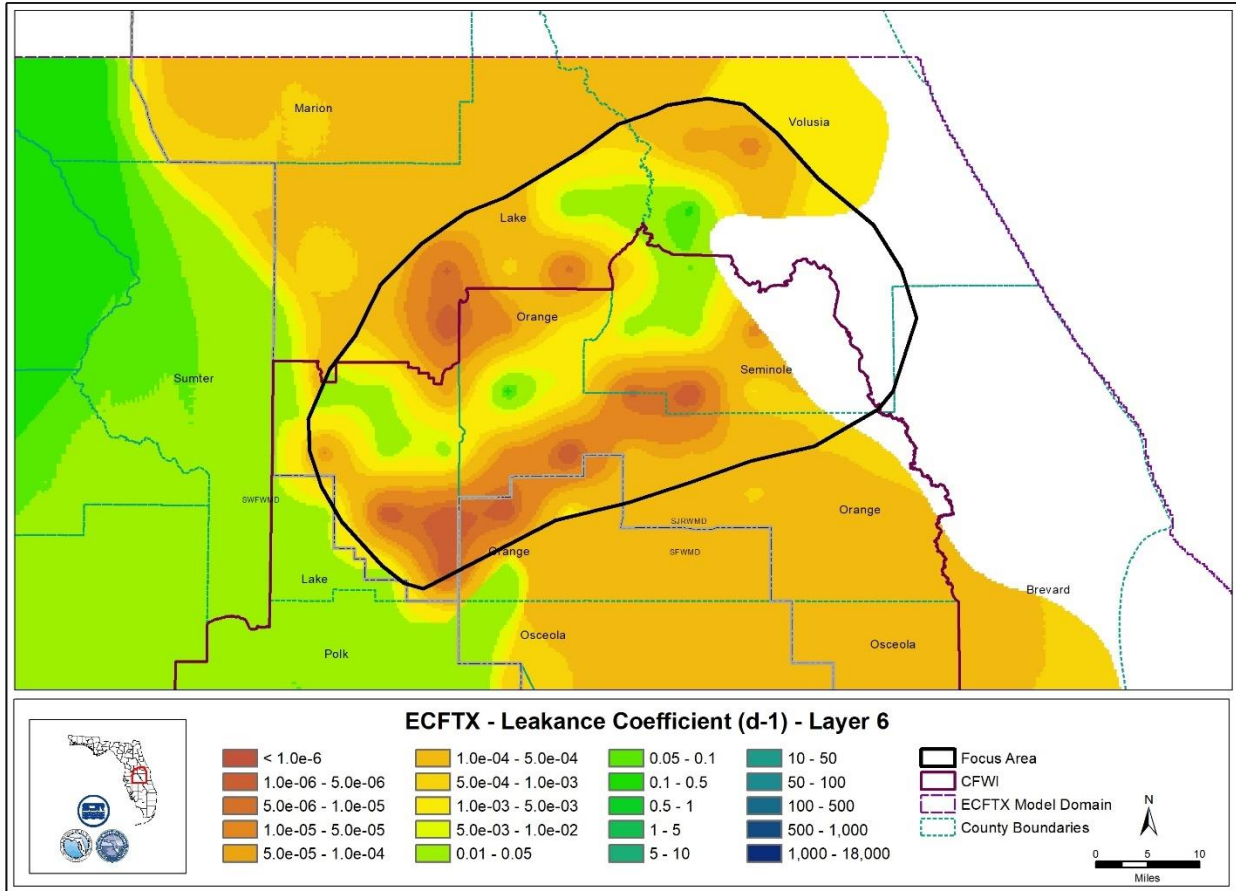


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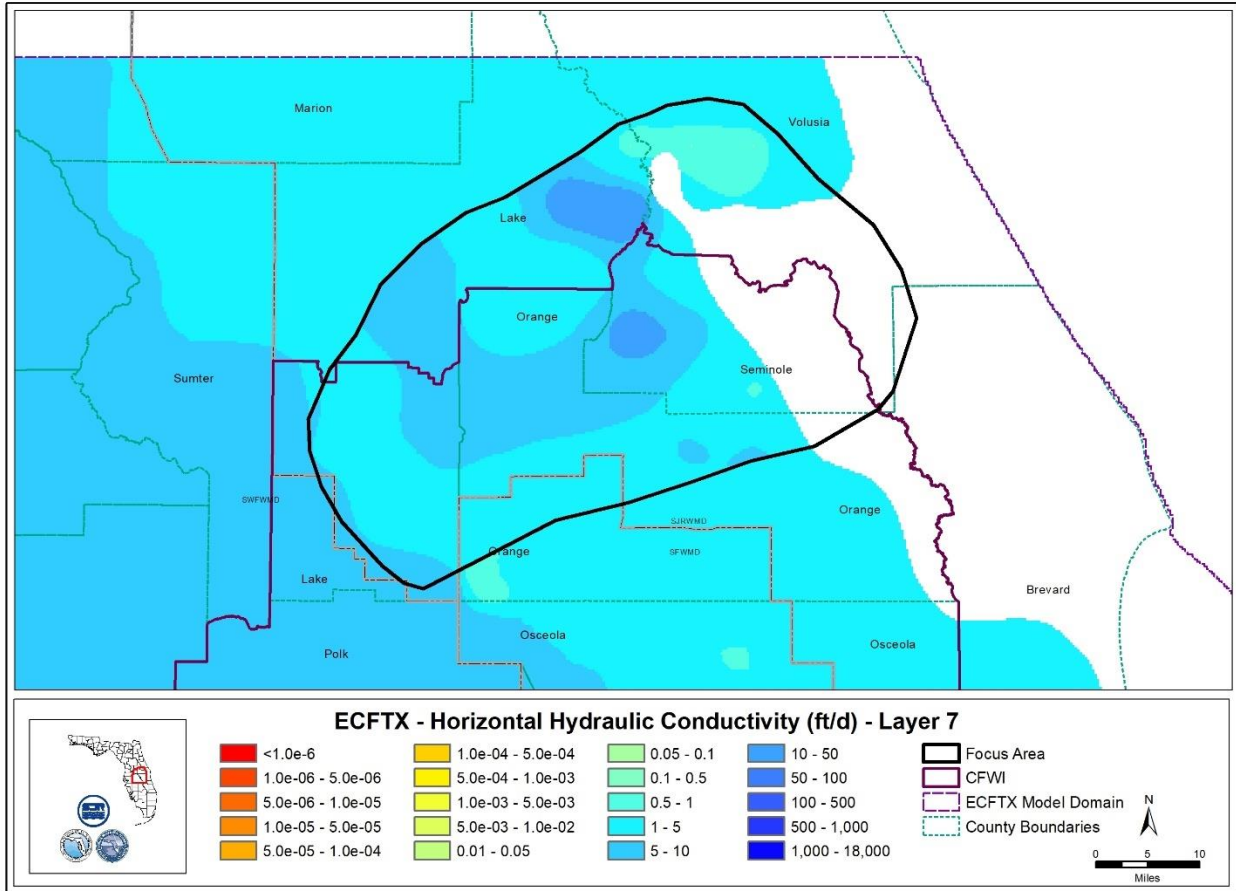


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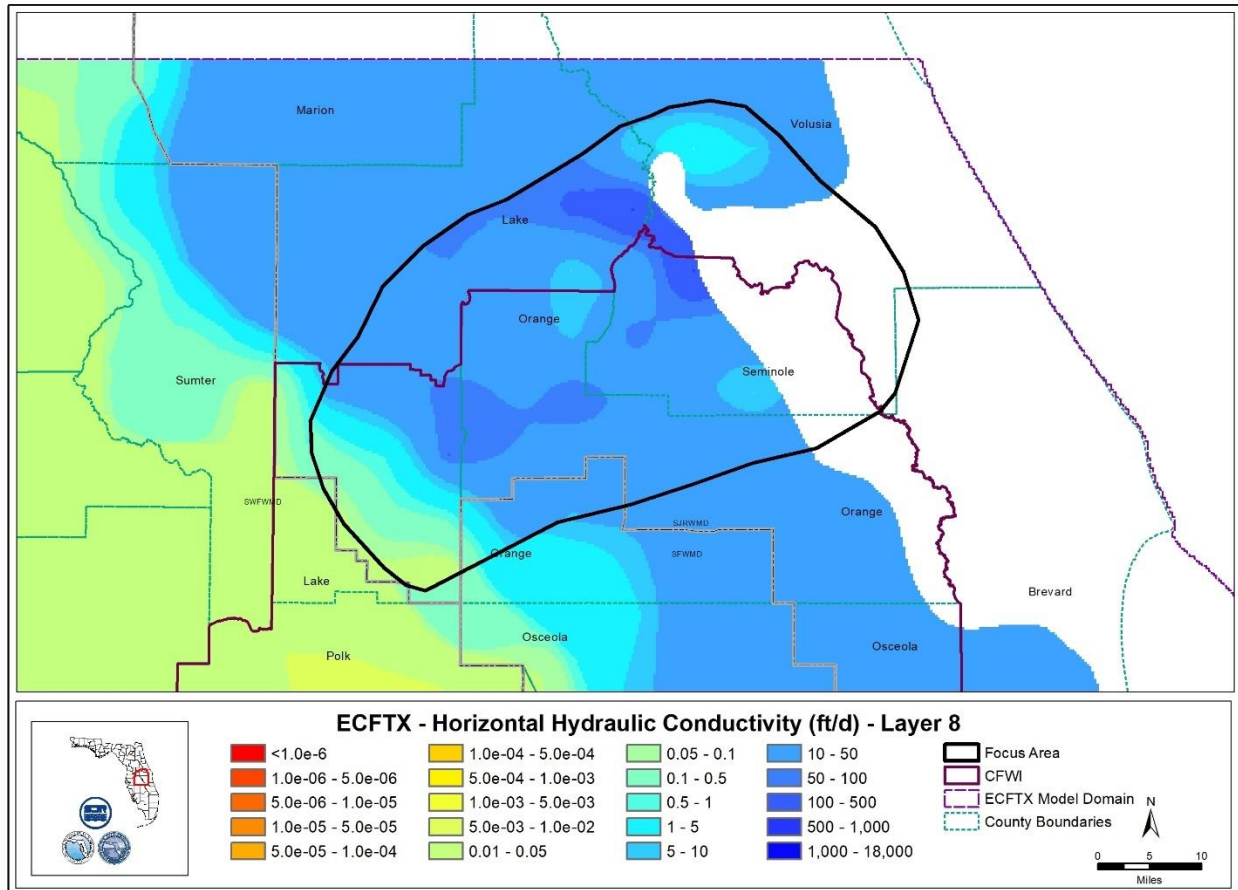


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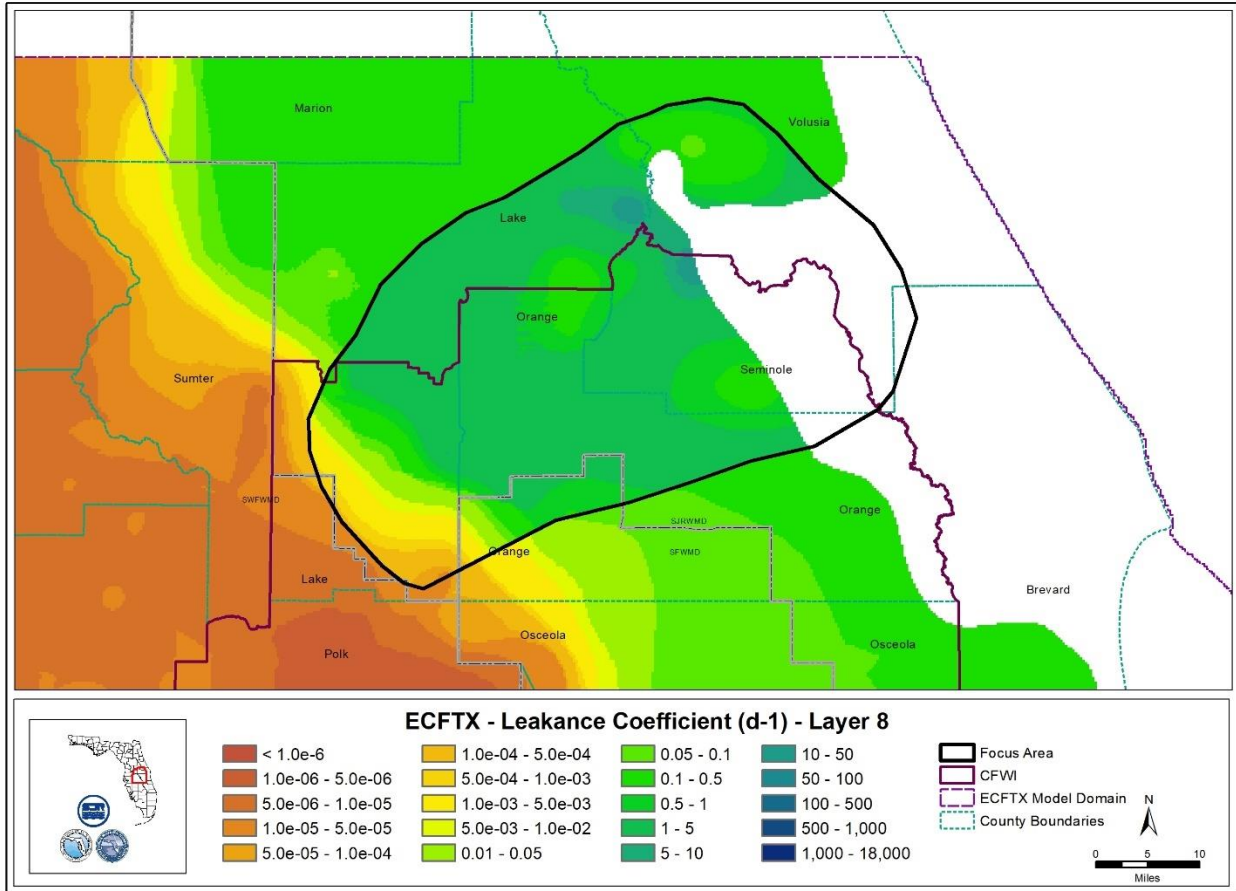


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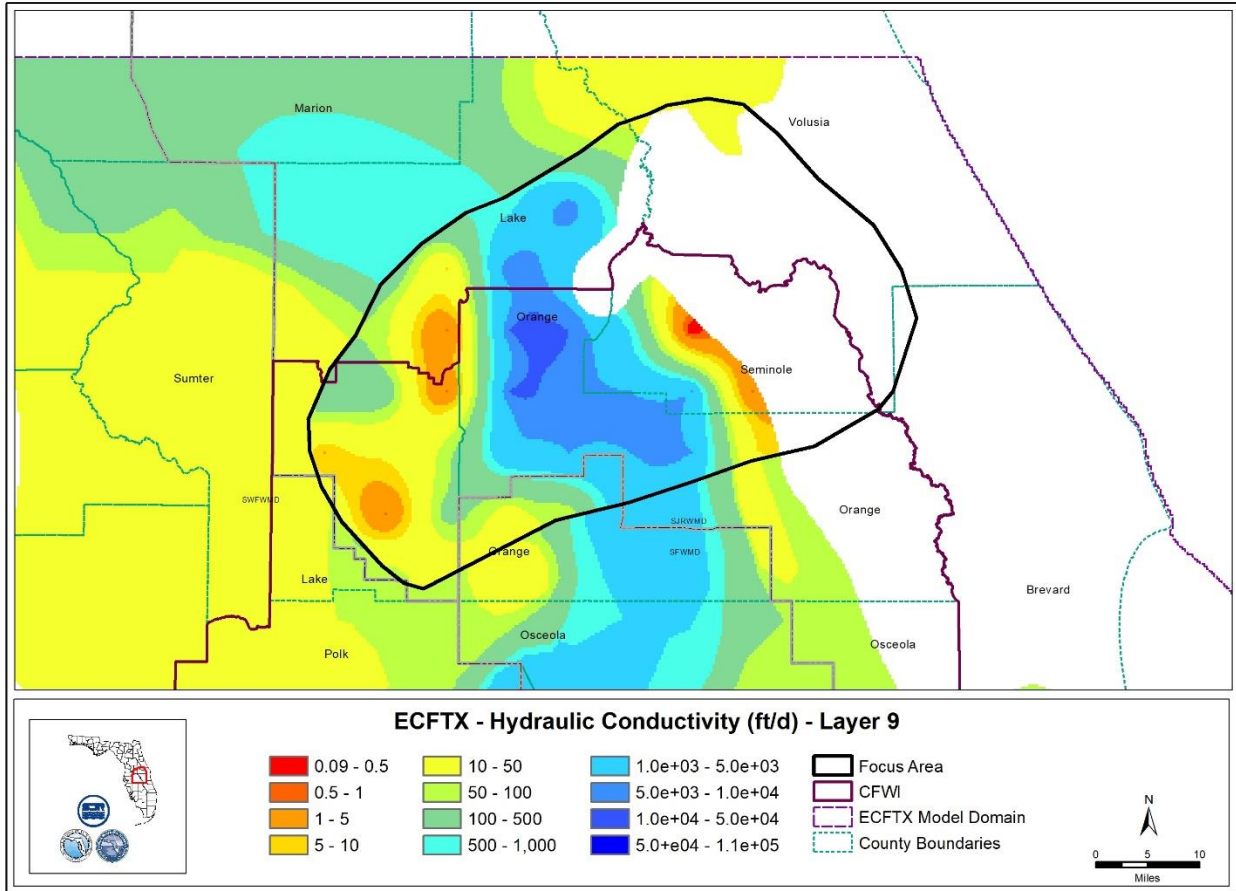


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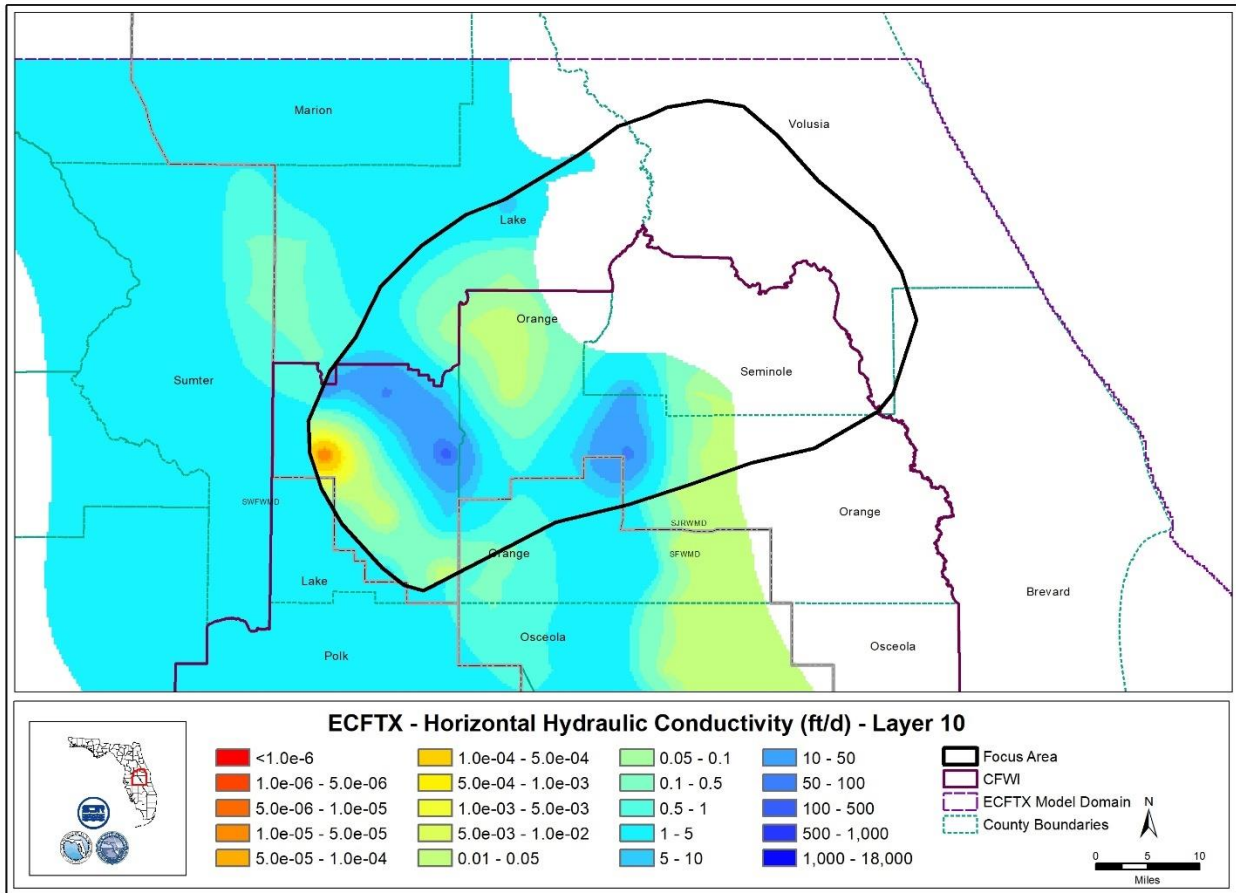


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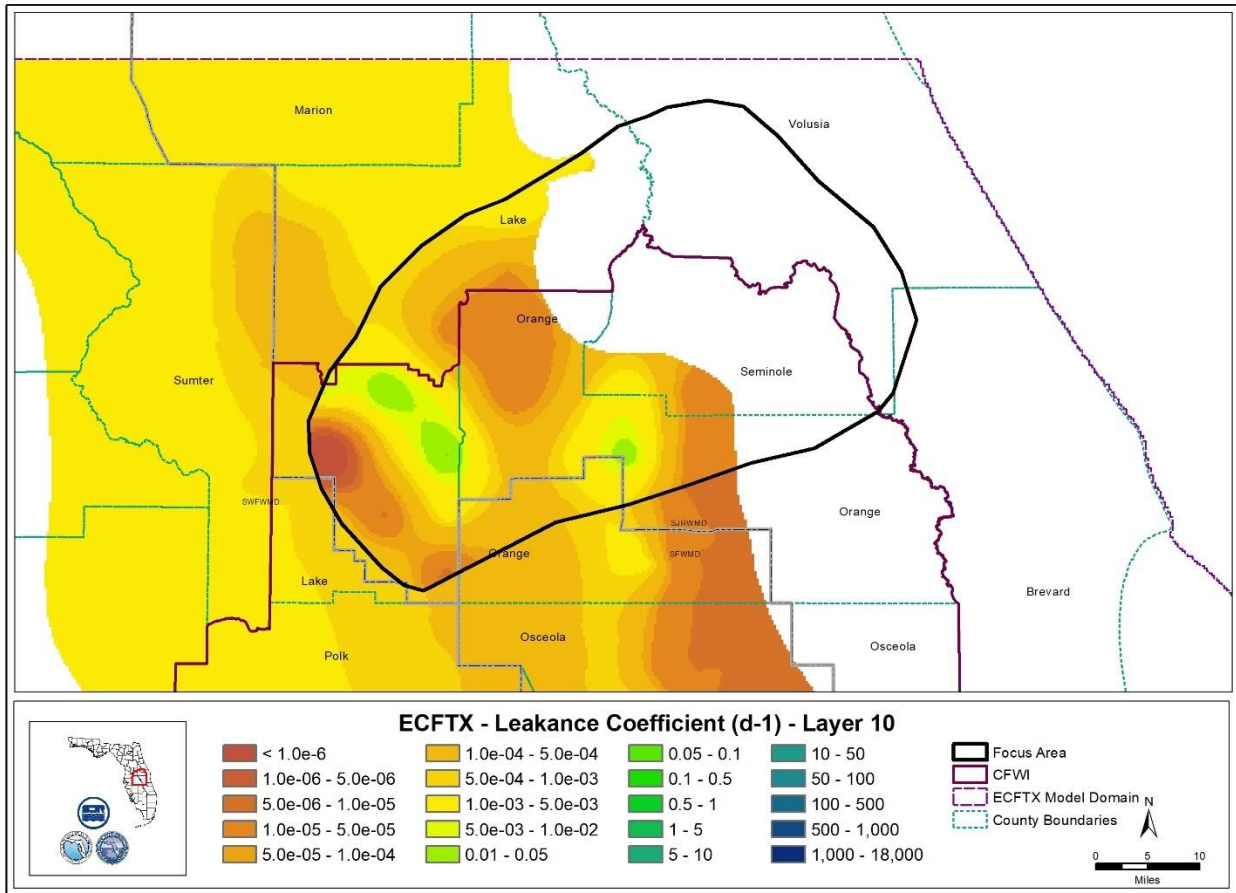
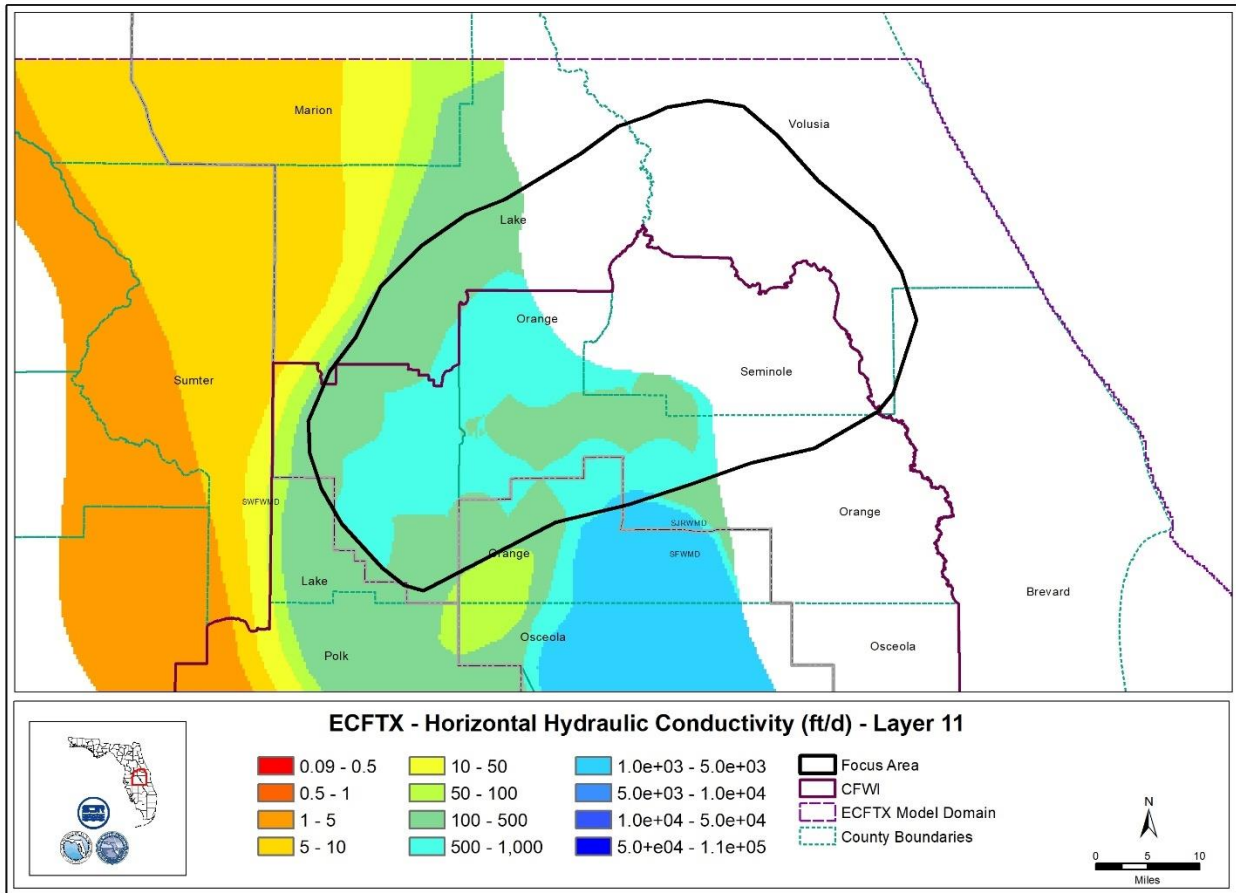


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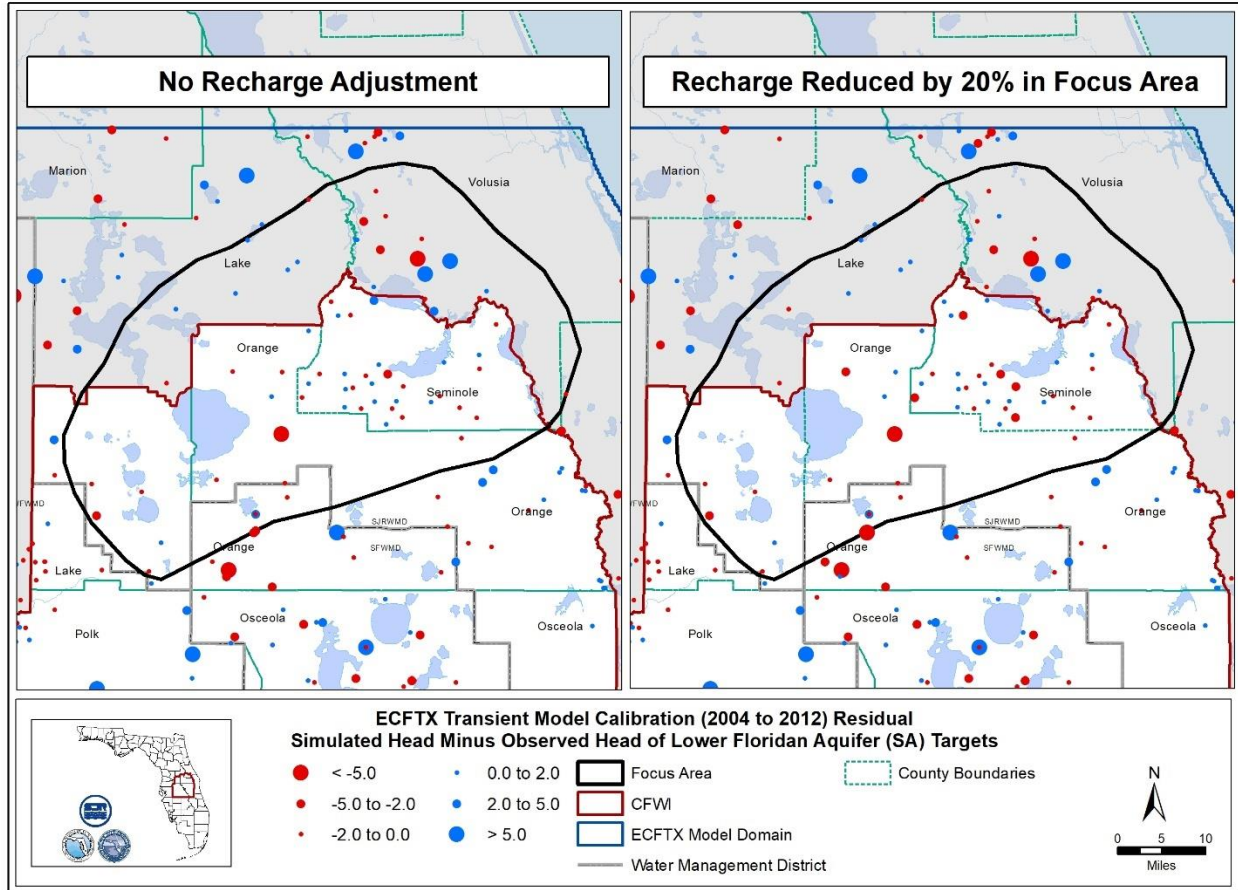


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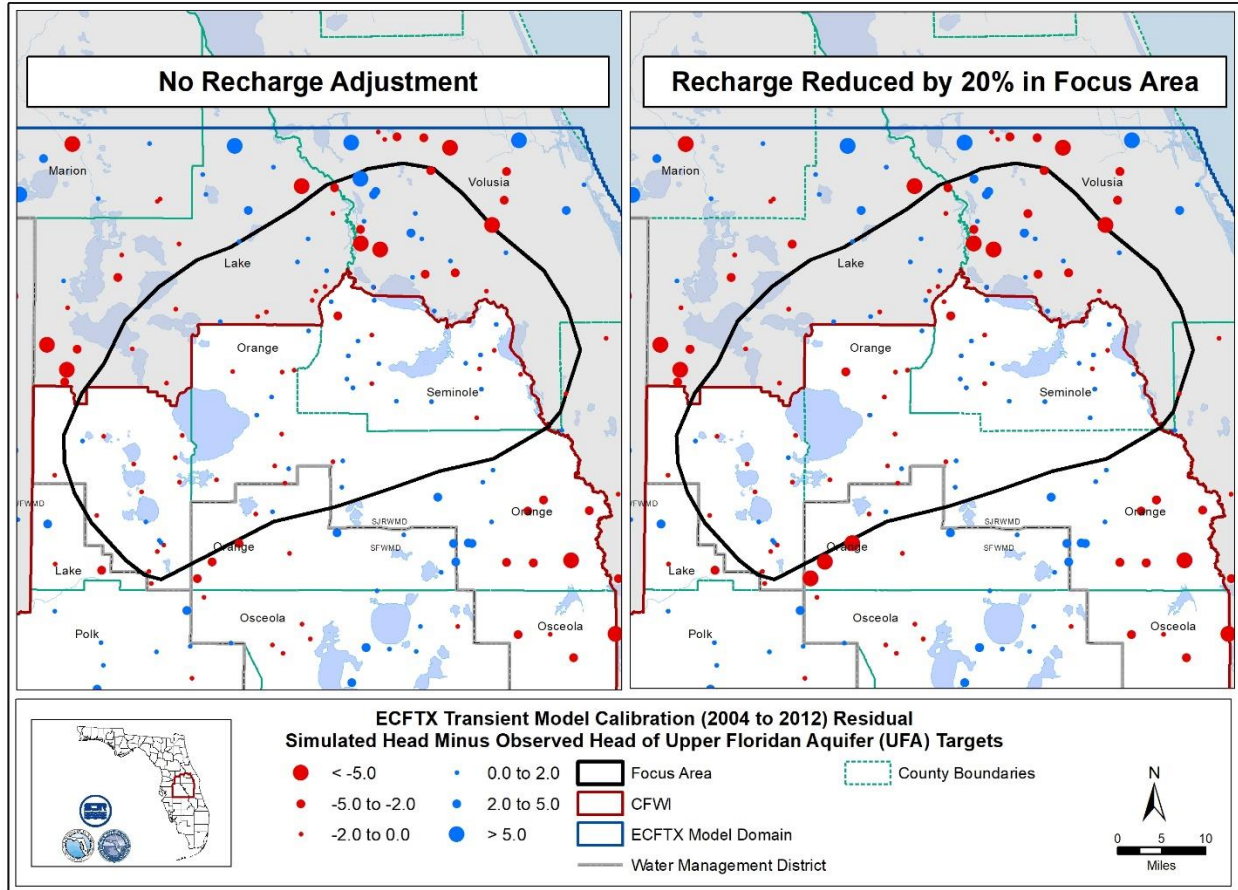


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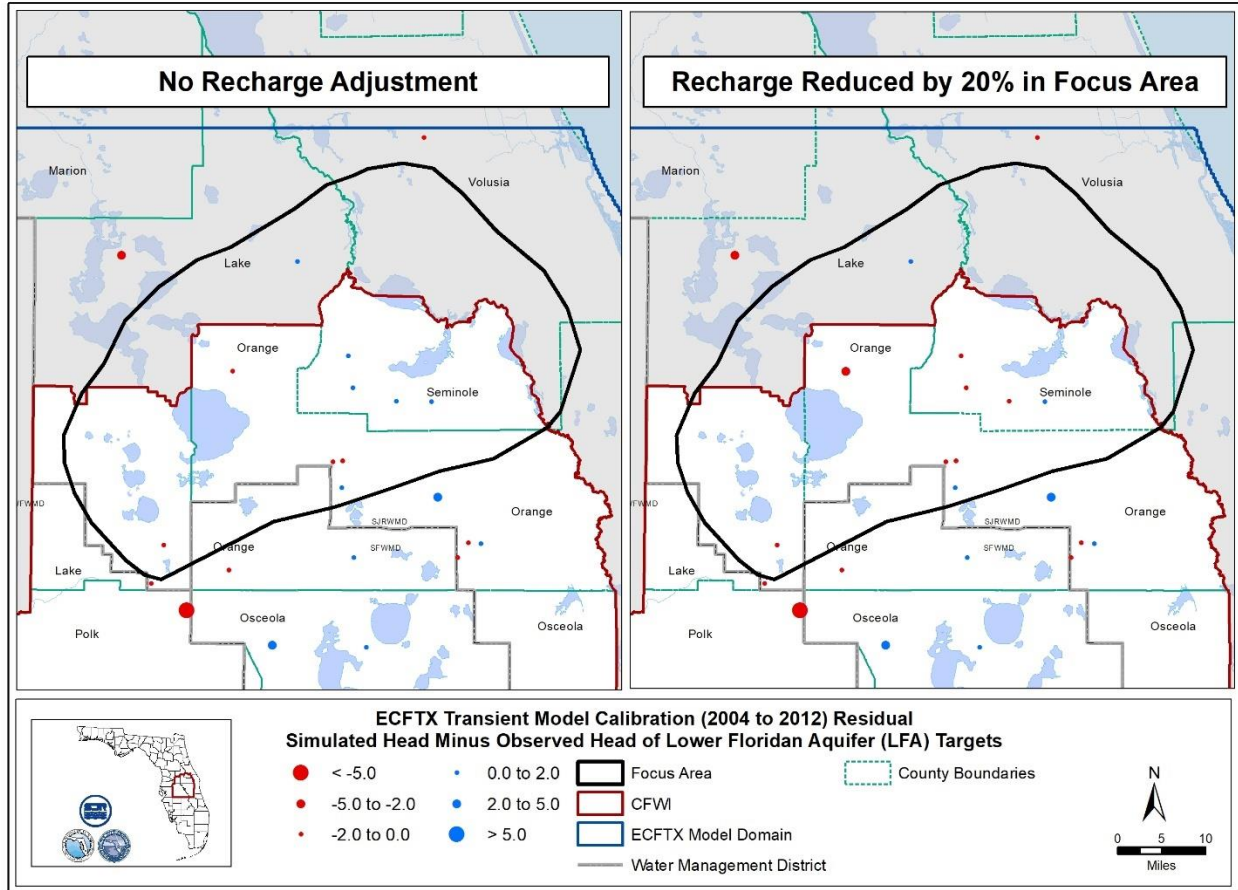


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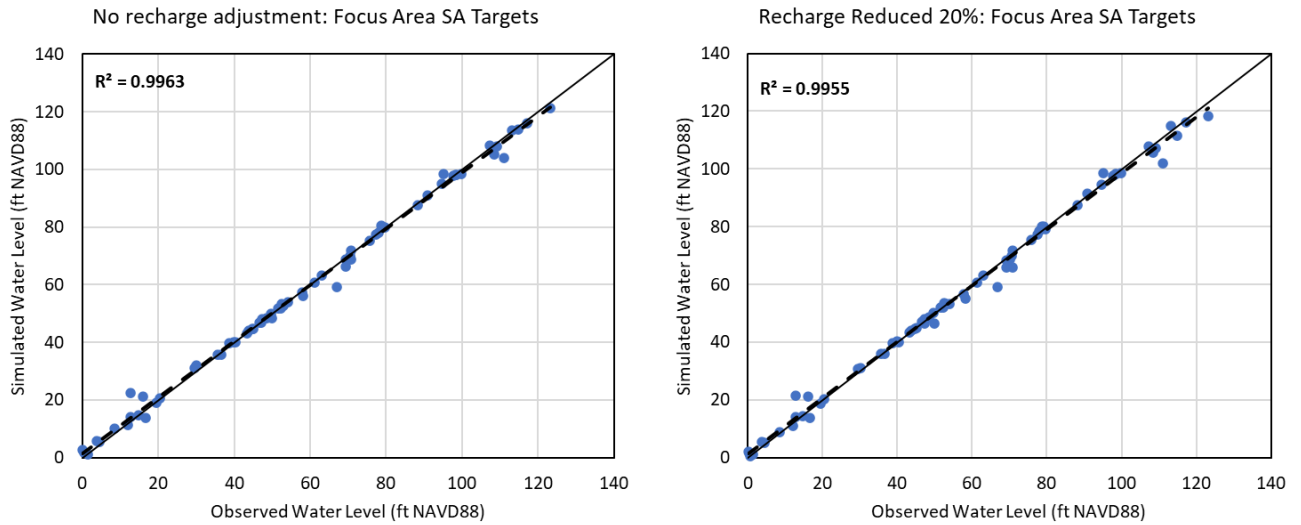


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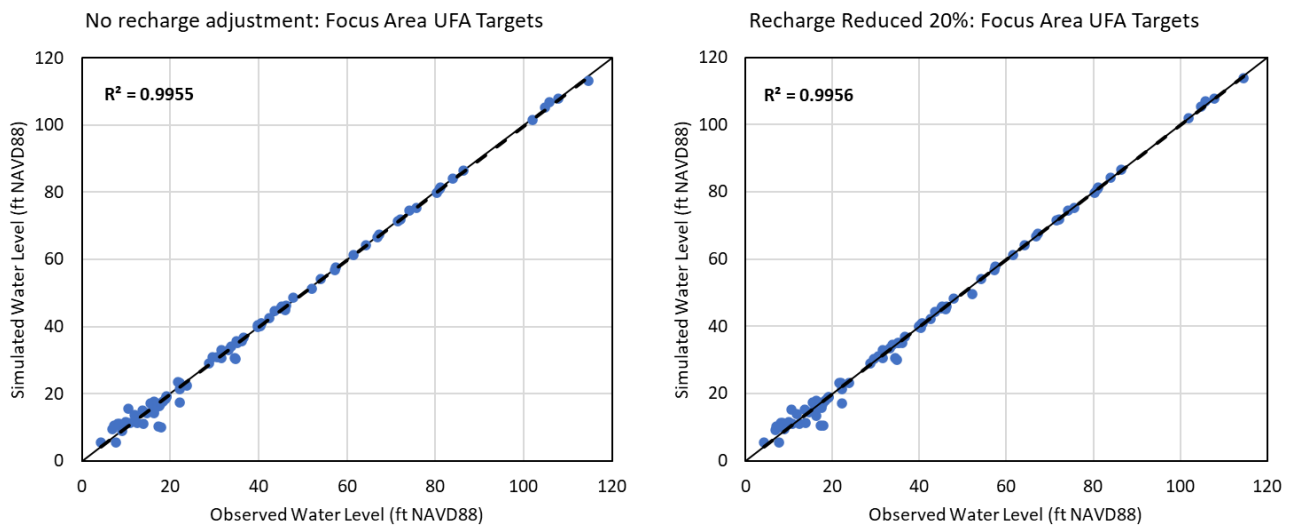


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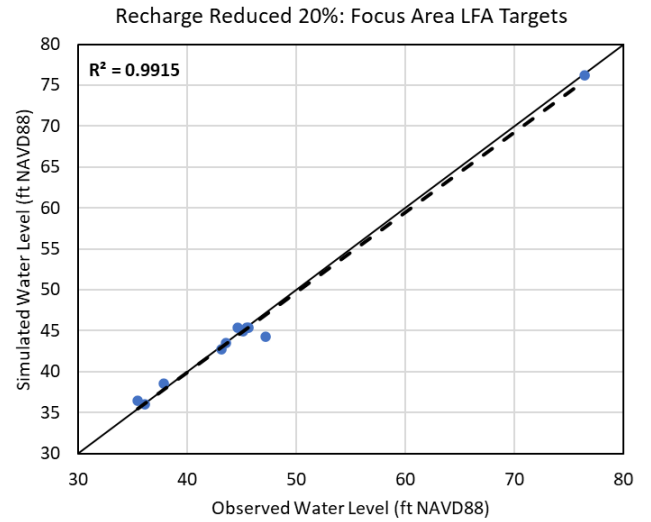
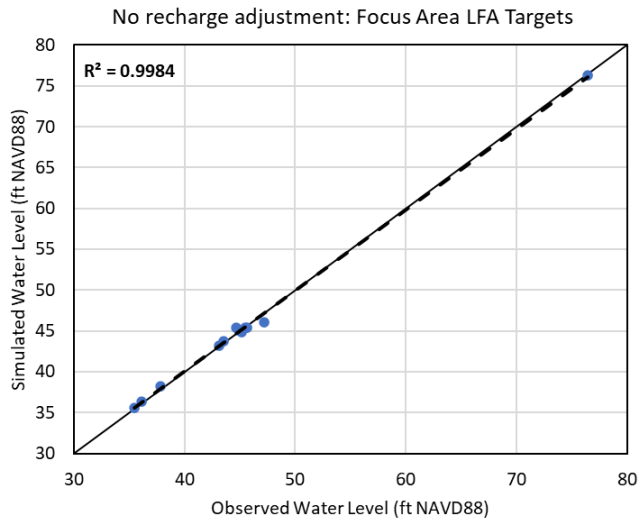
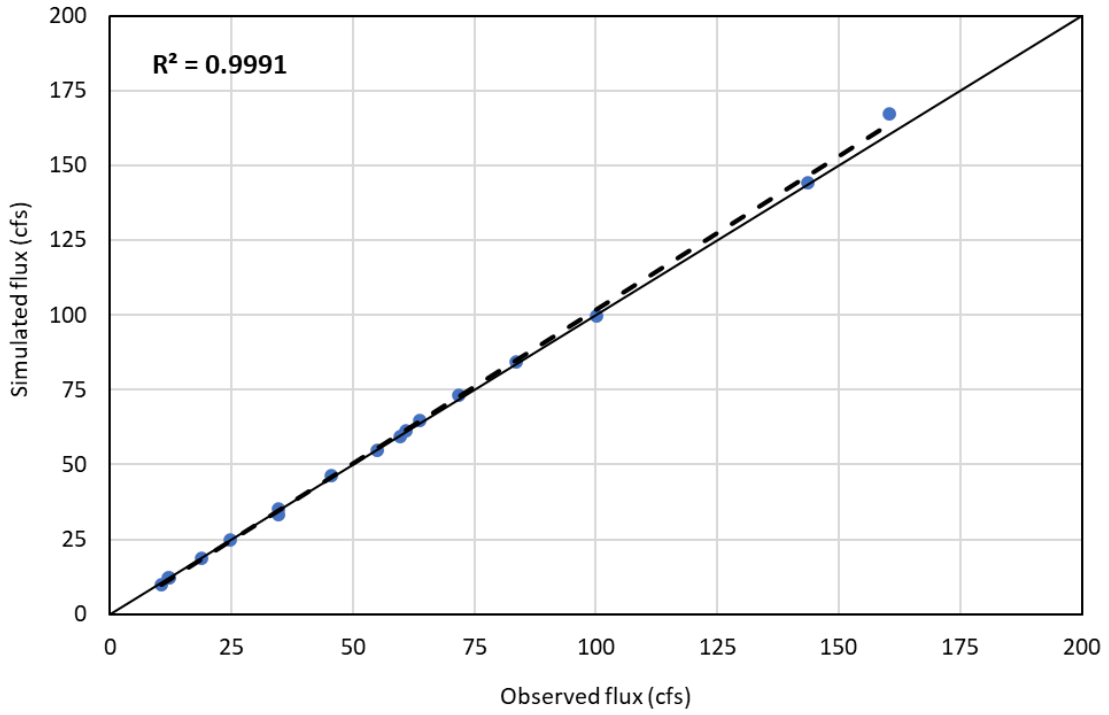


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No Recharge Adjustment: Spring Flow Observed vs. Simulated



Recharge Reduced 20%: Spring Flow Observed vs. Simulated

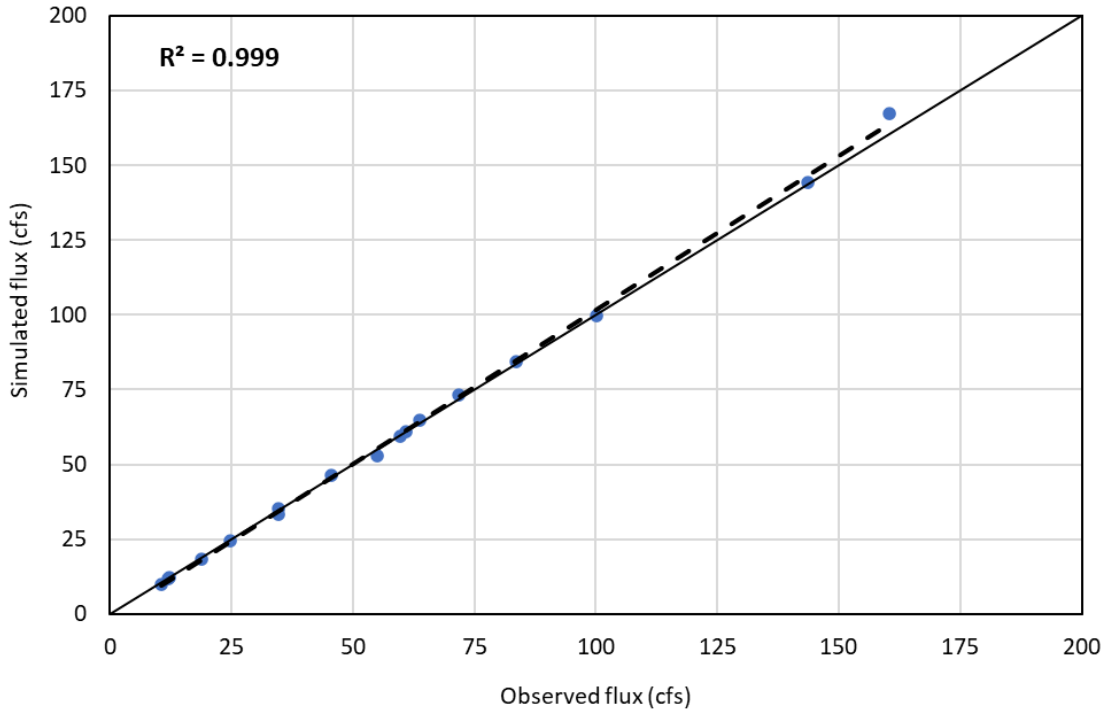


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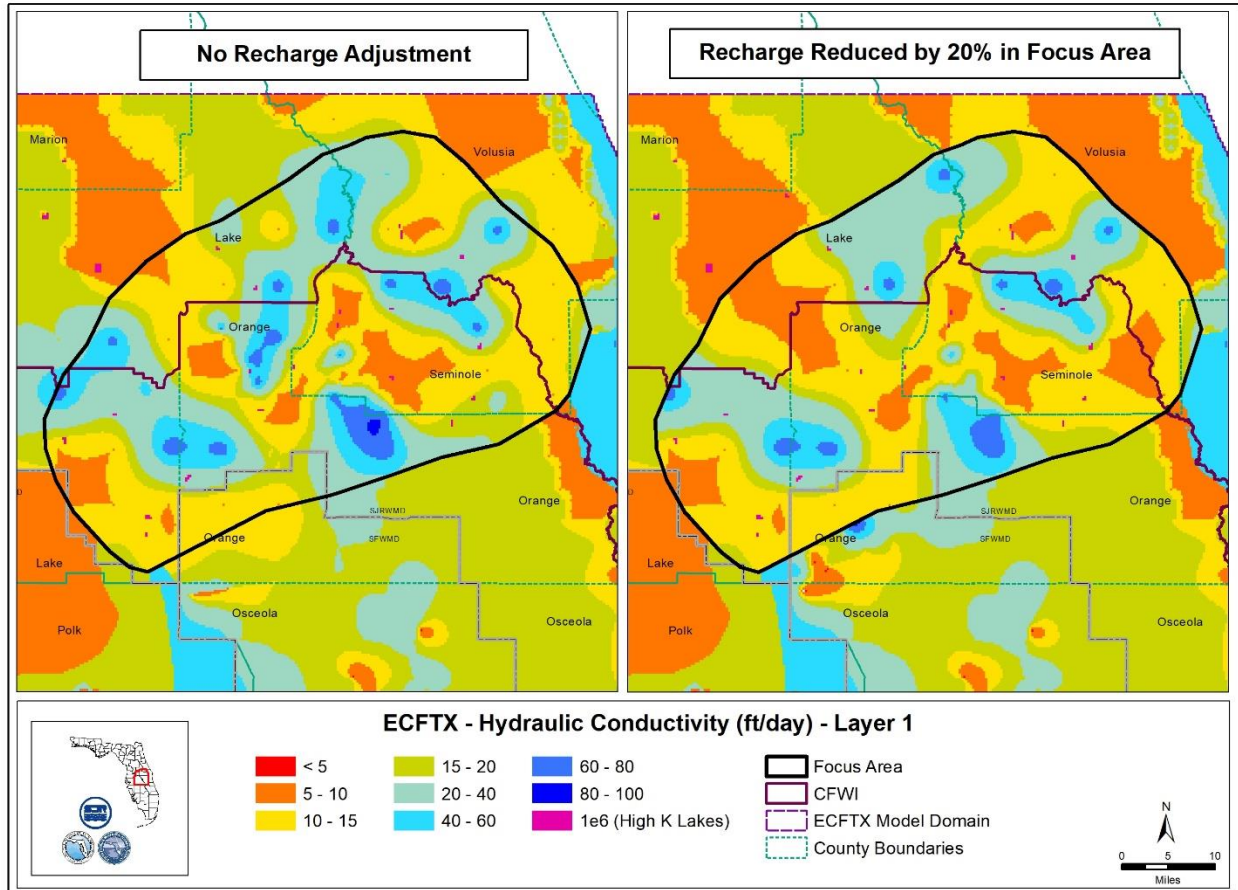


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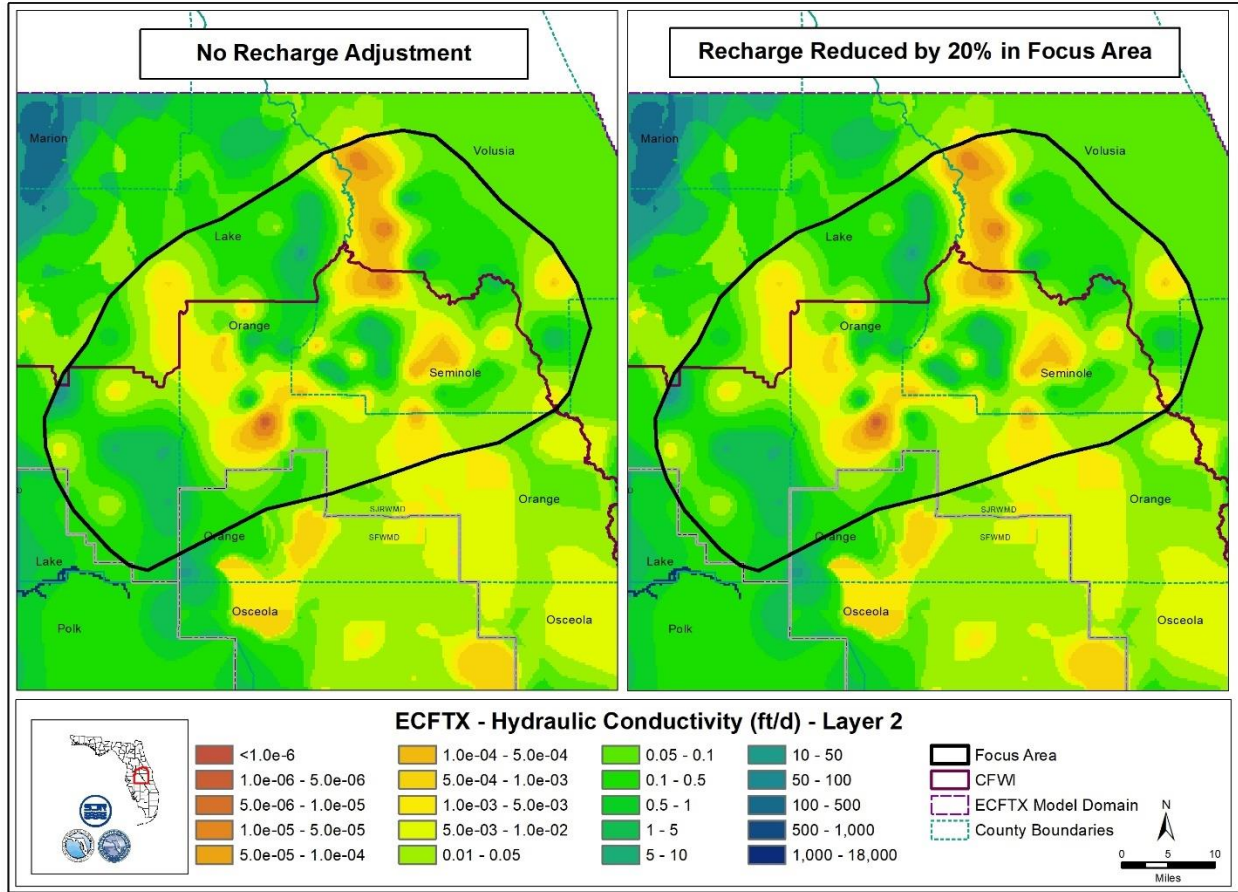


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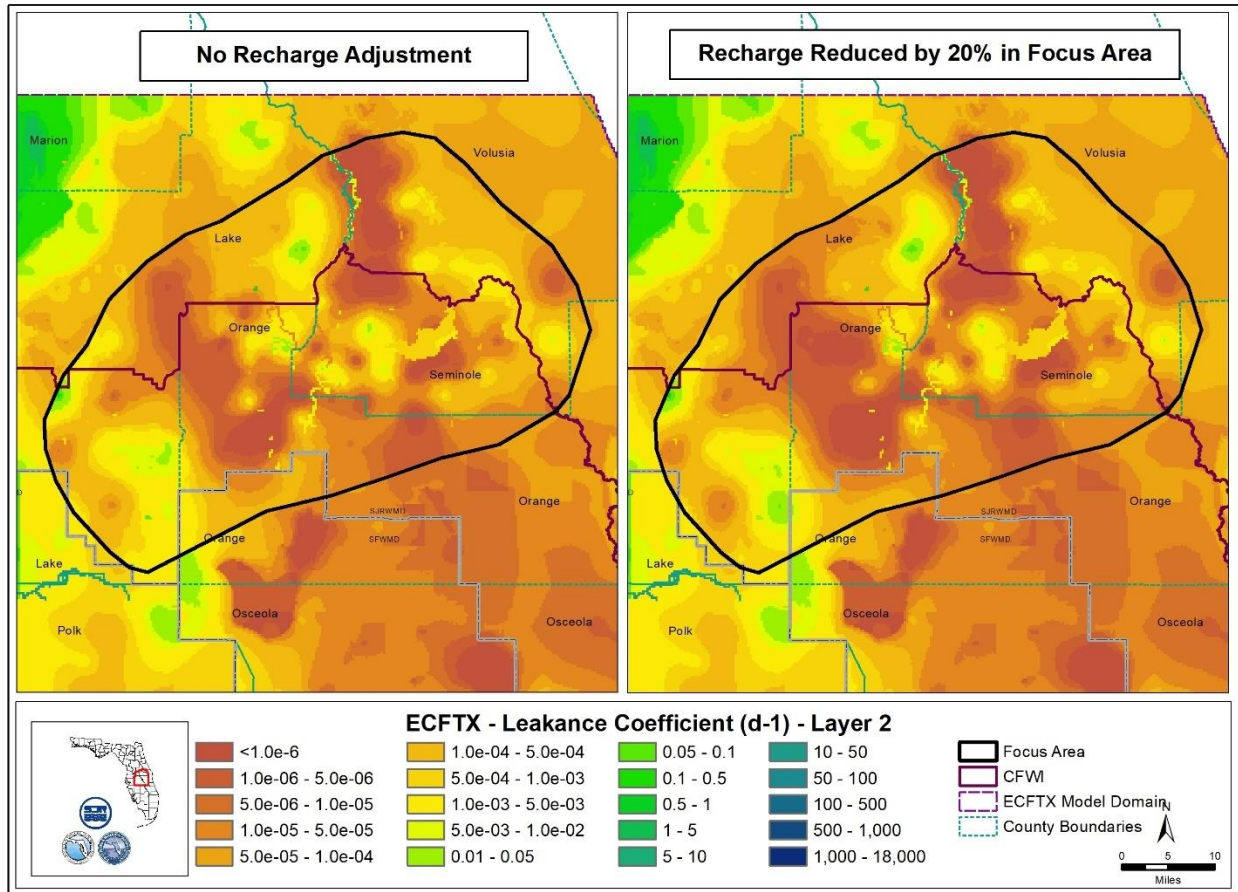


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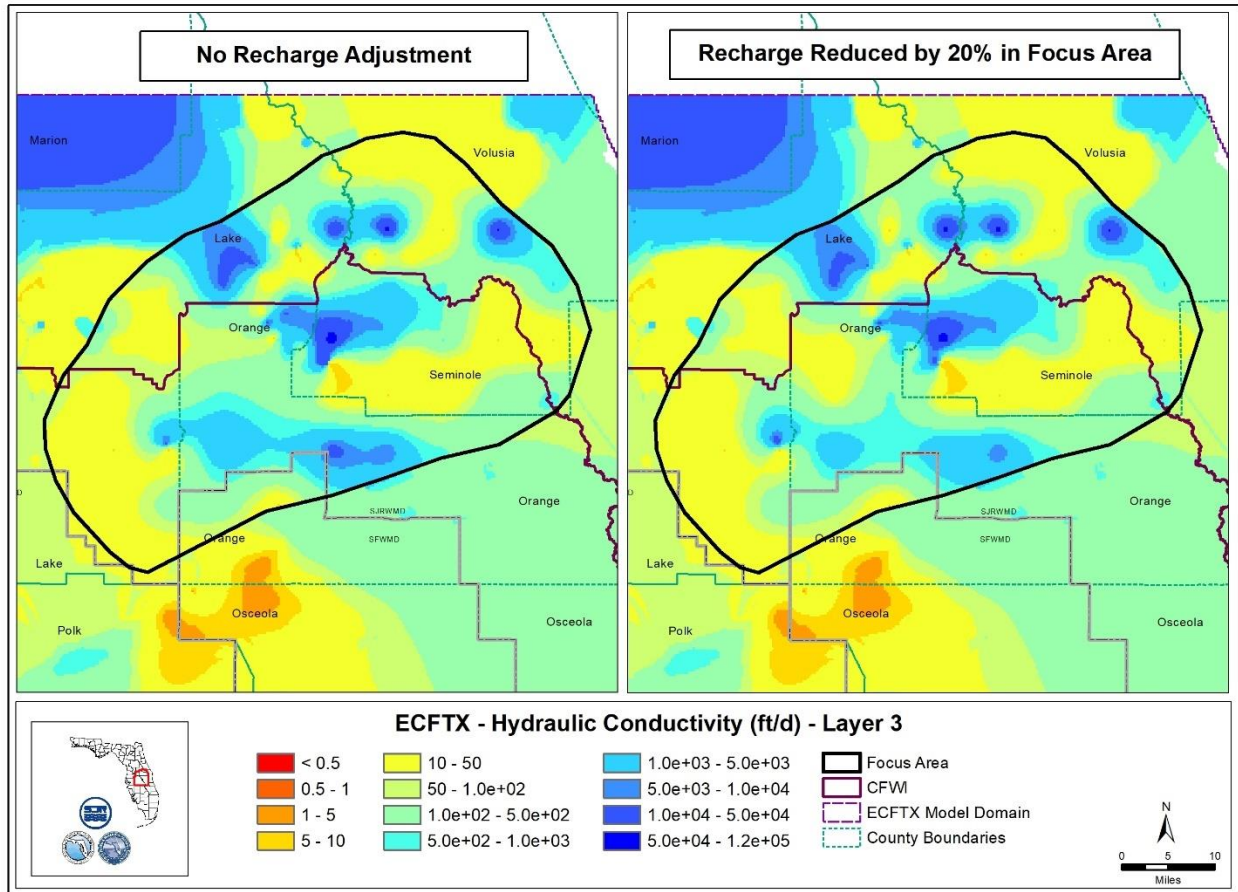


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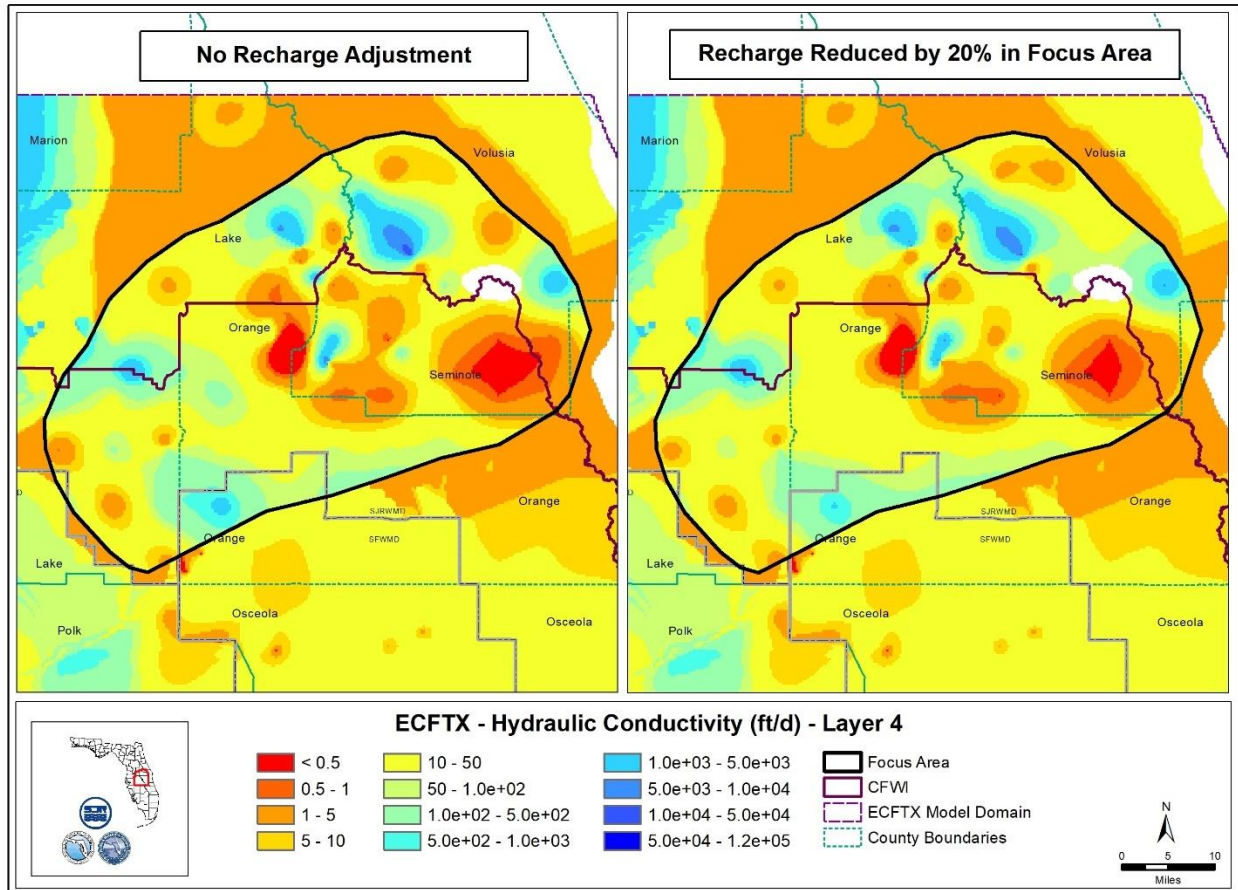


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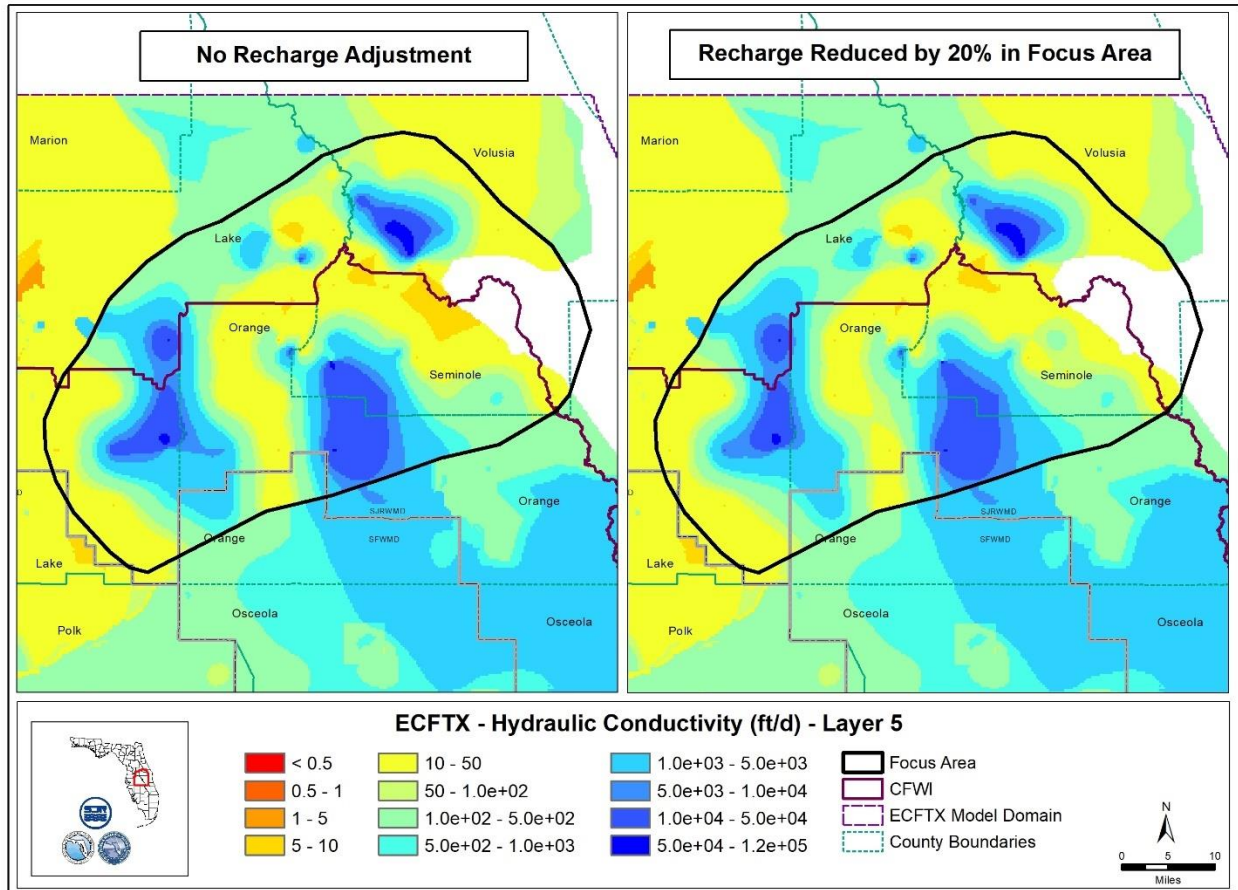


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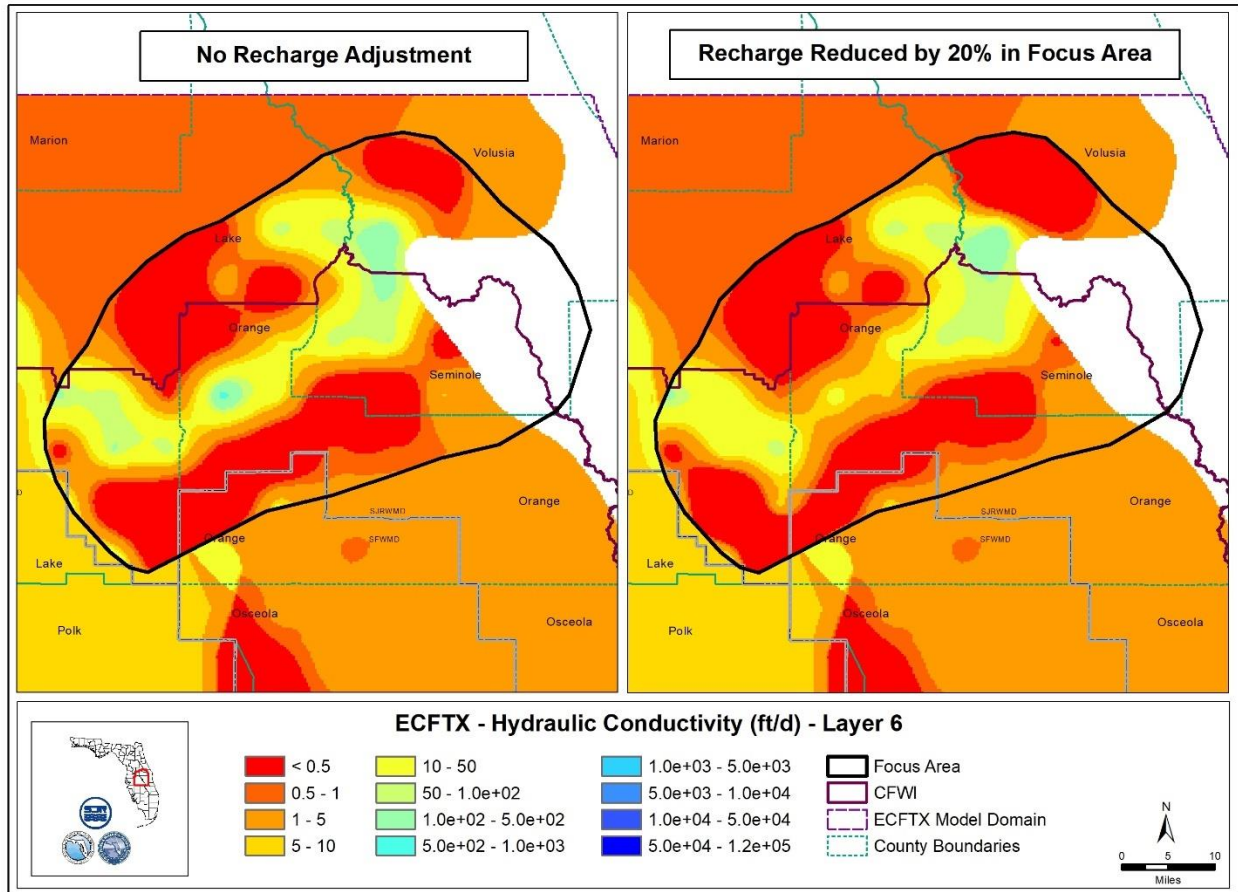


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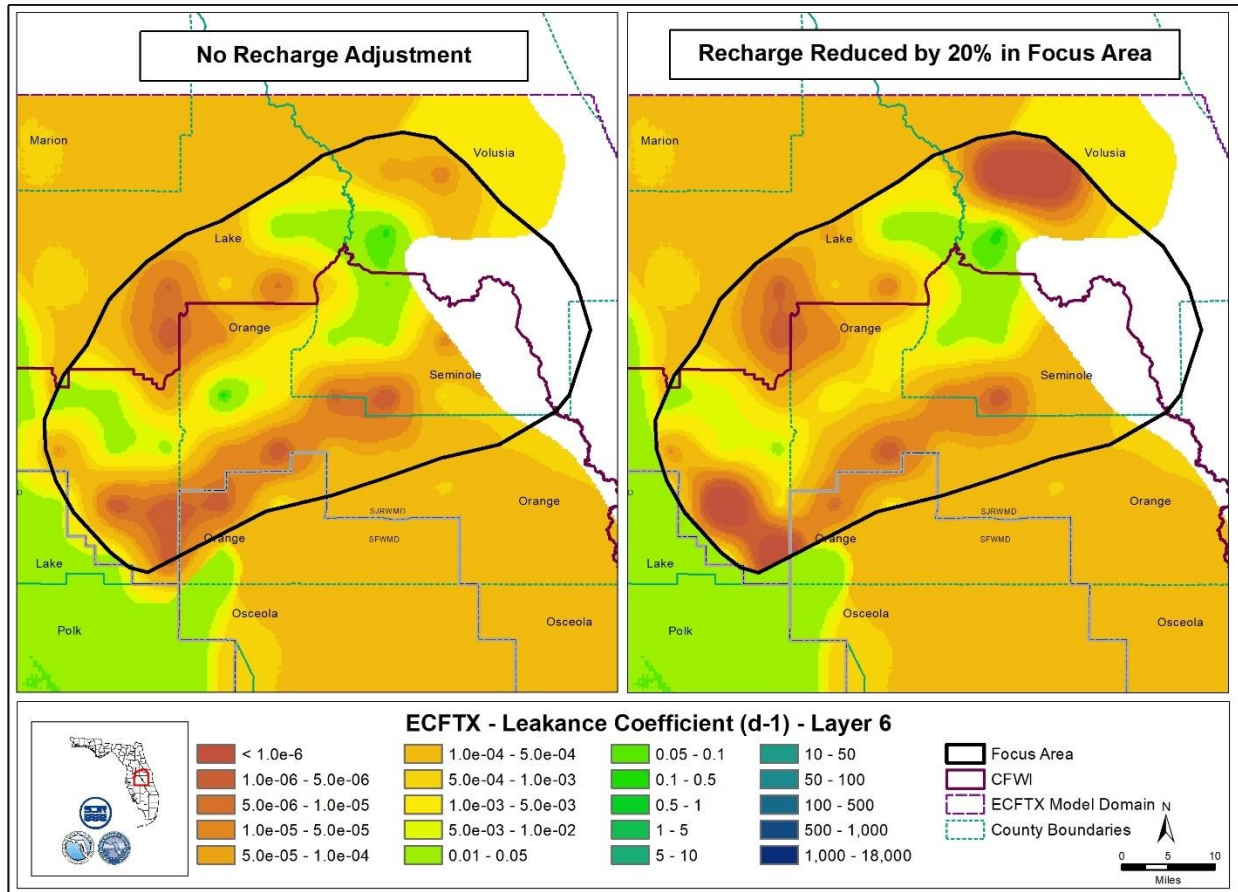


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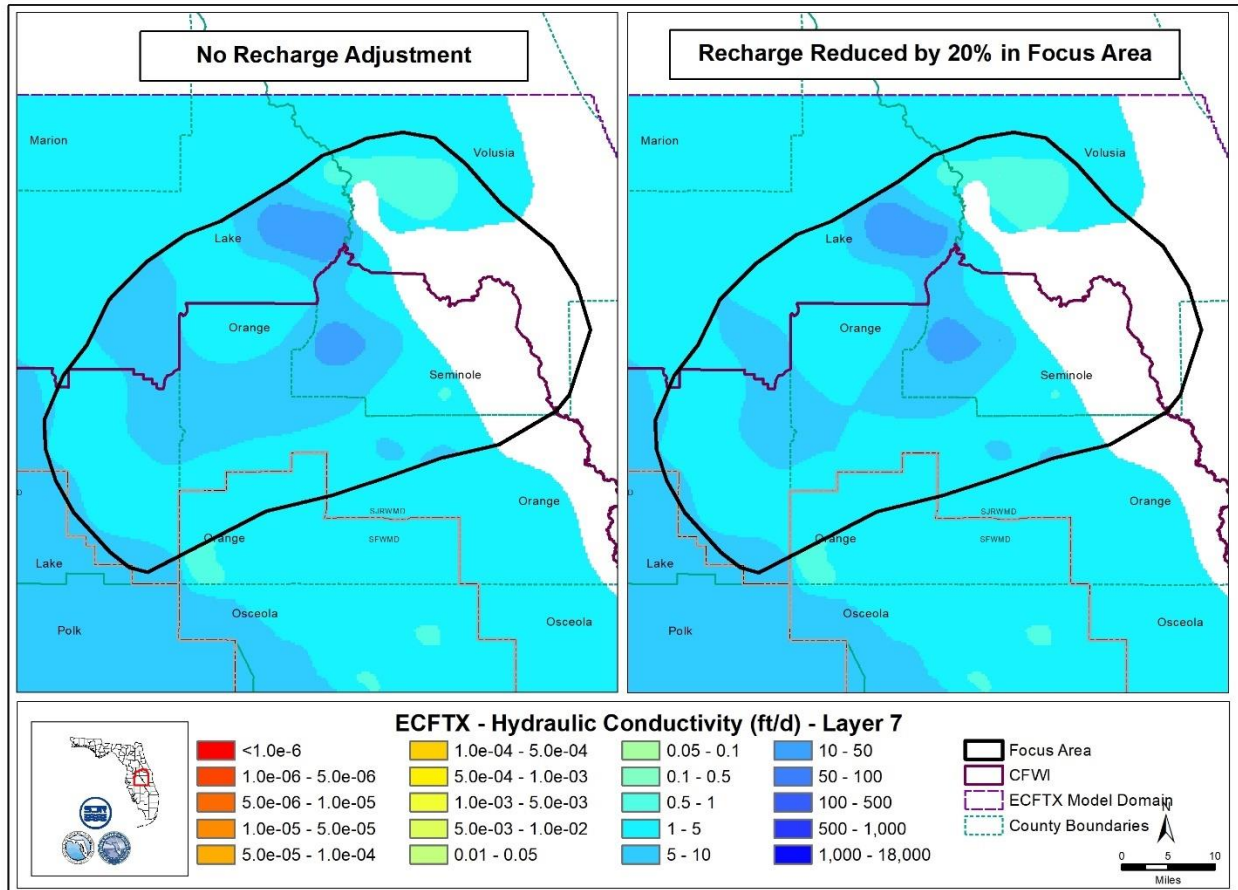


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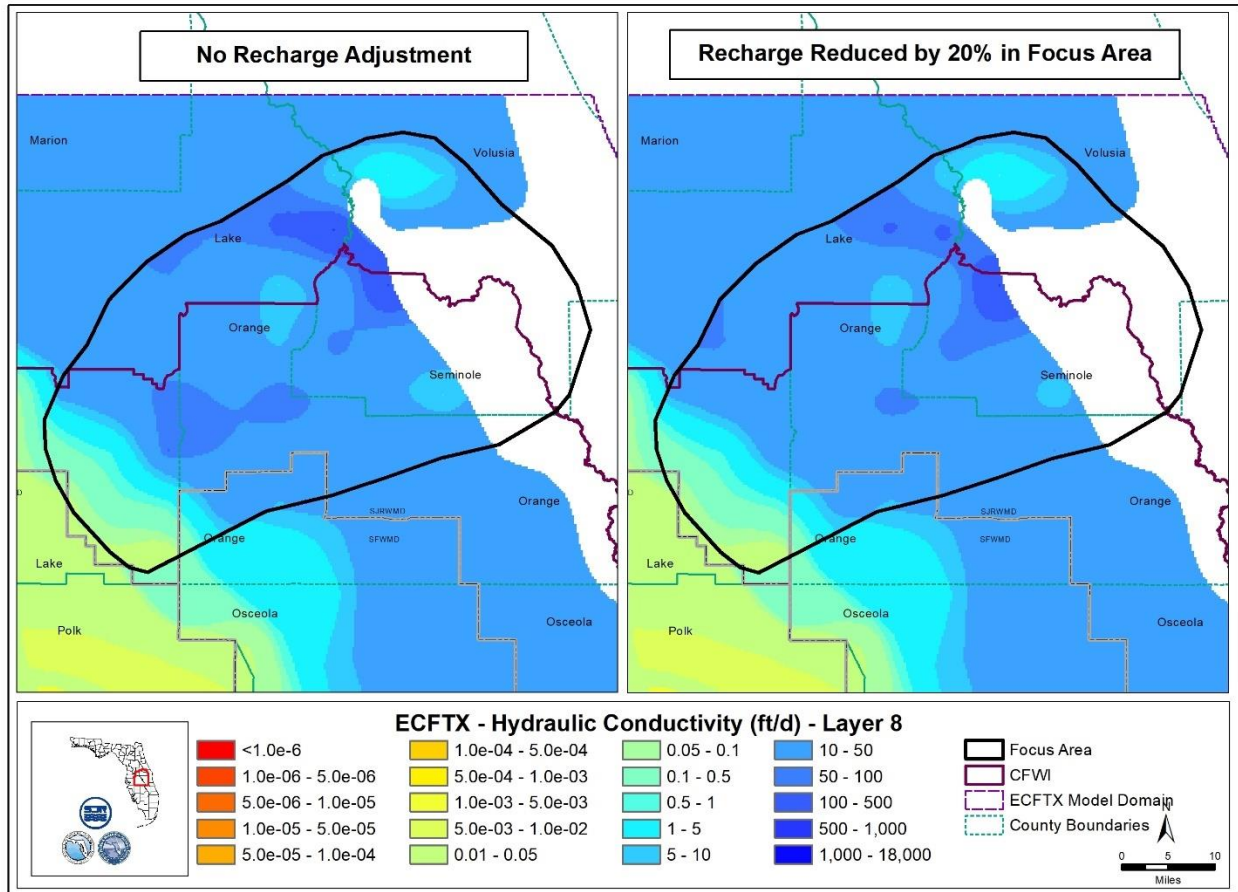


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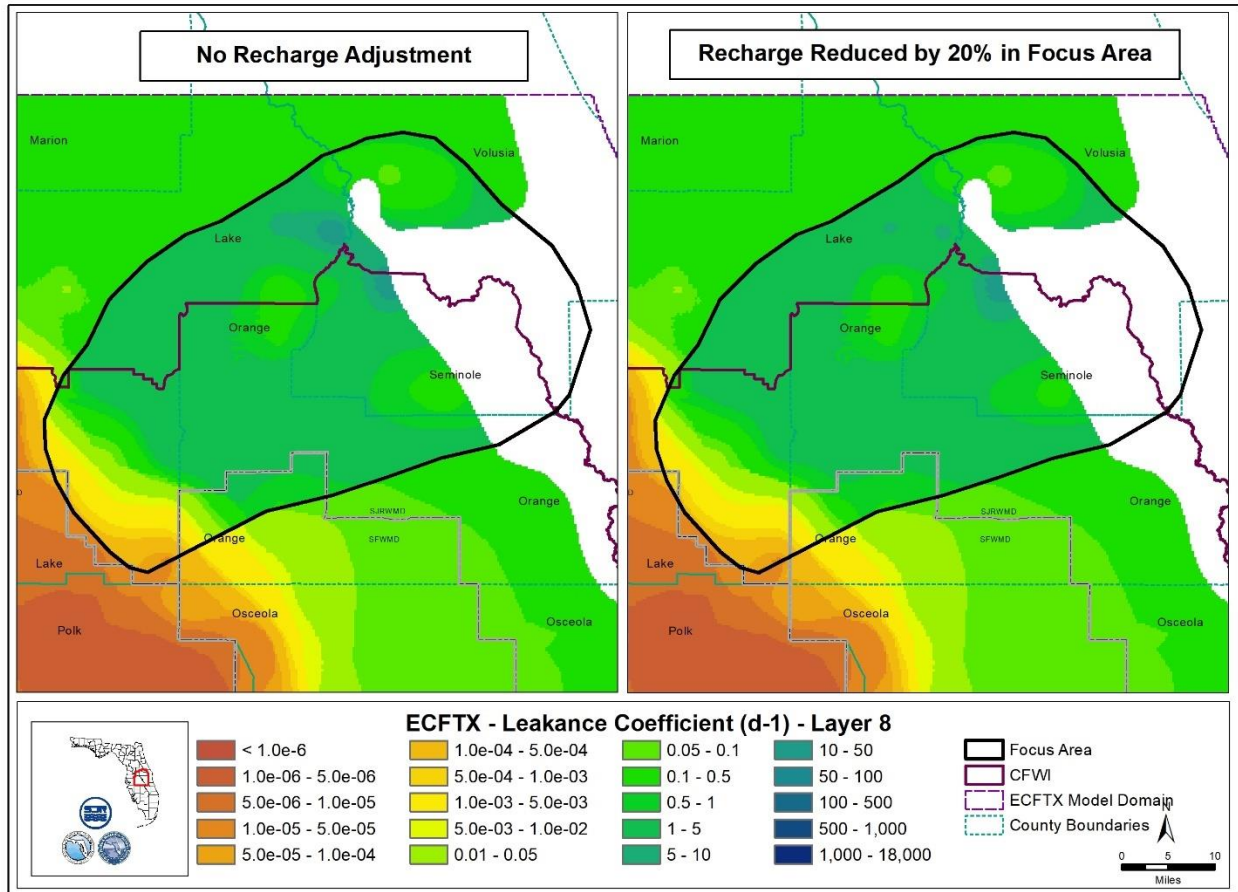


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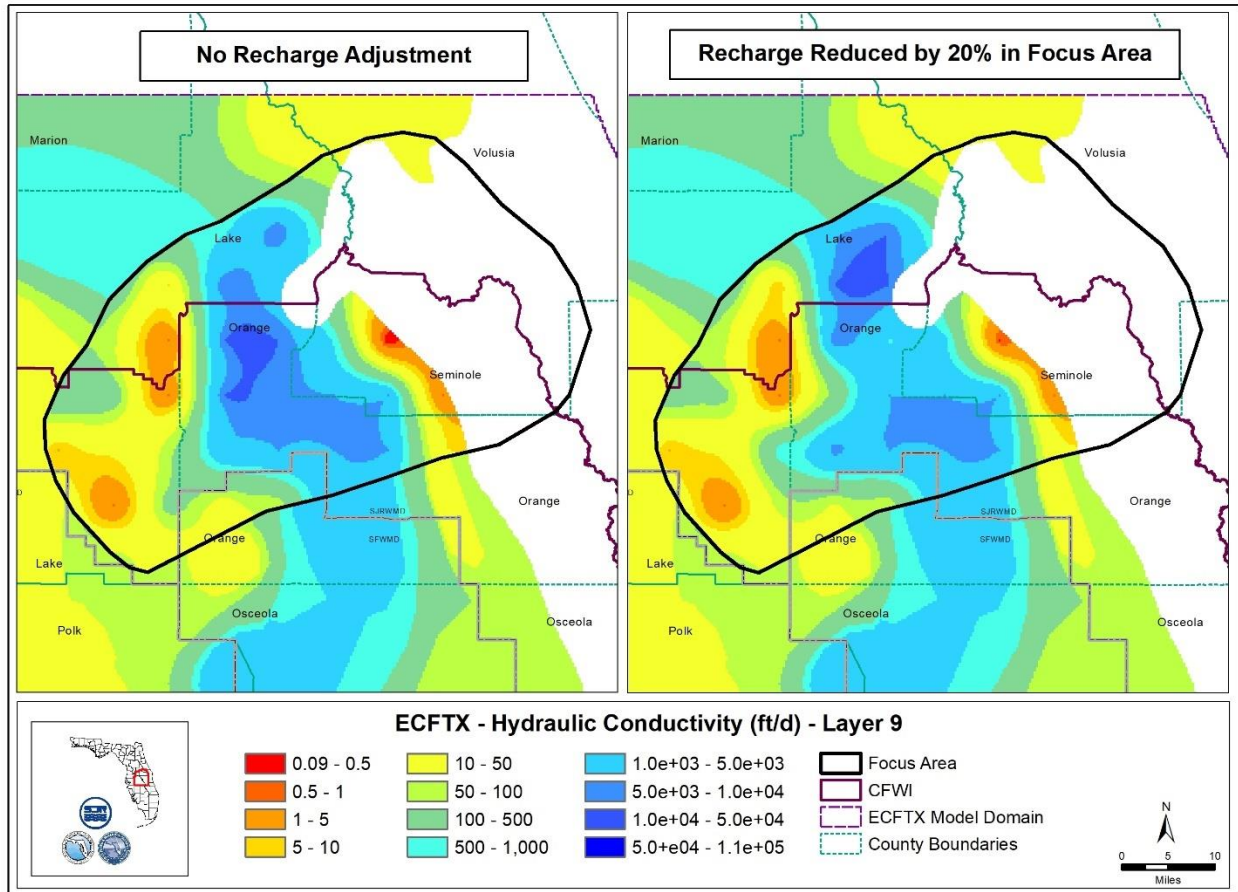


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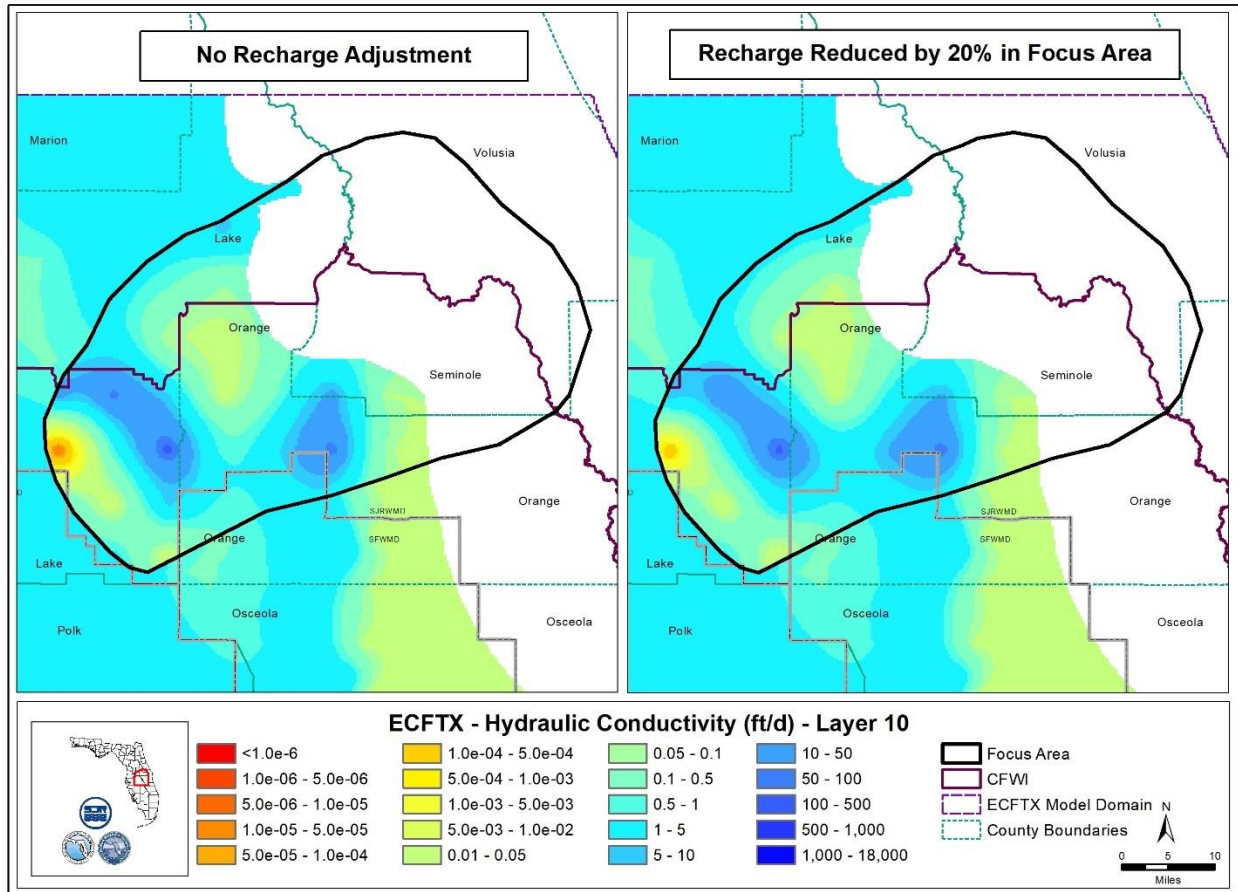


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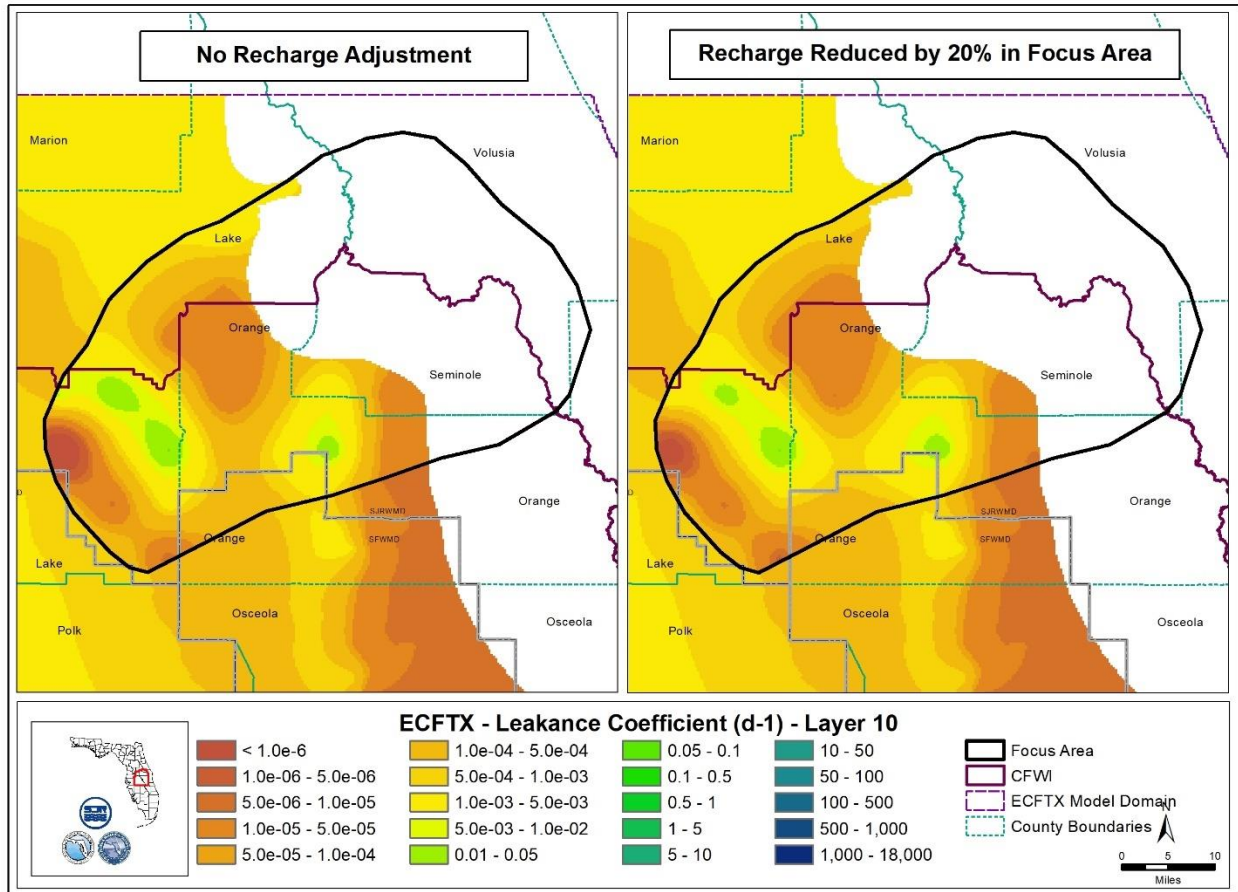


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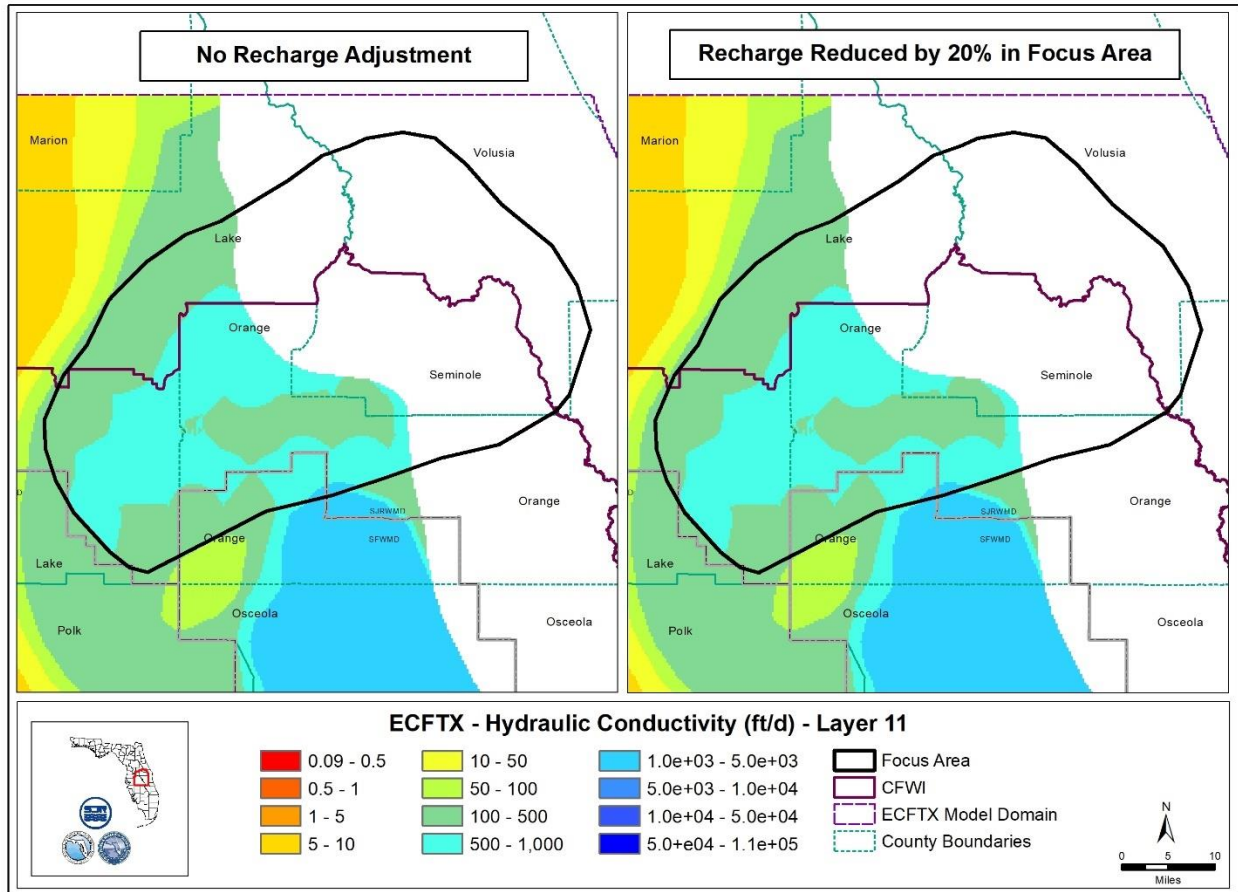


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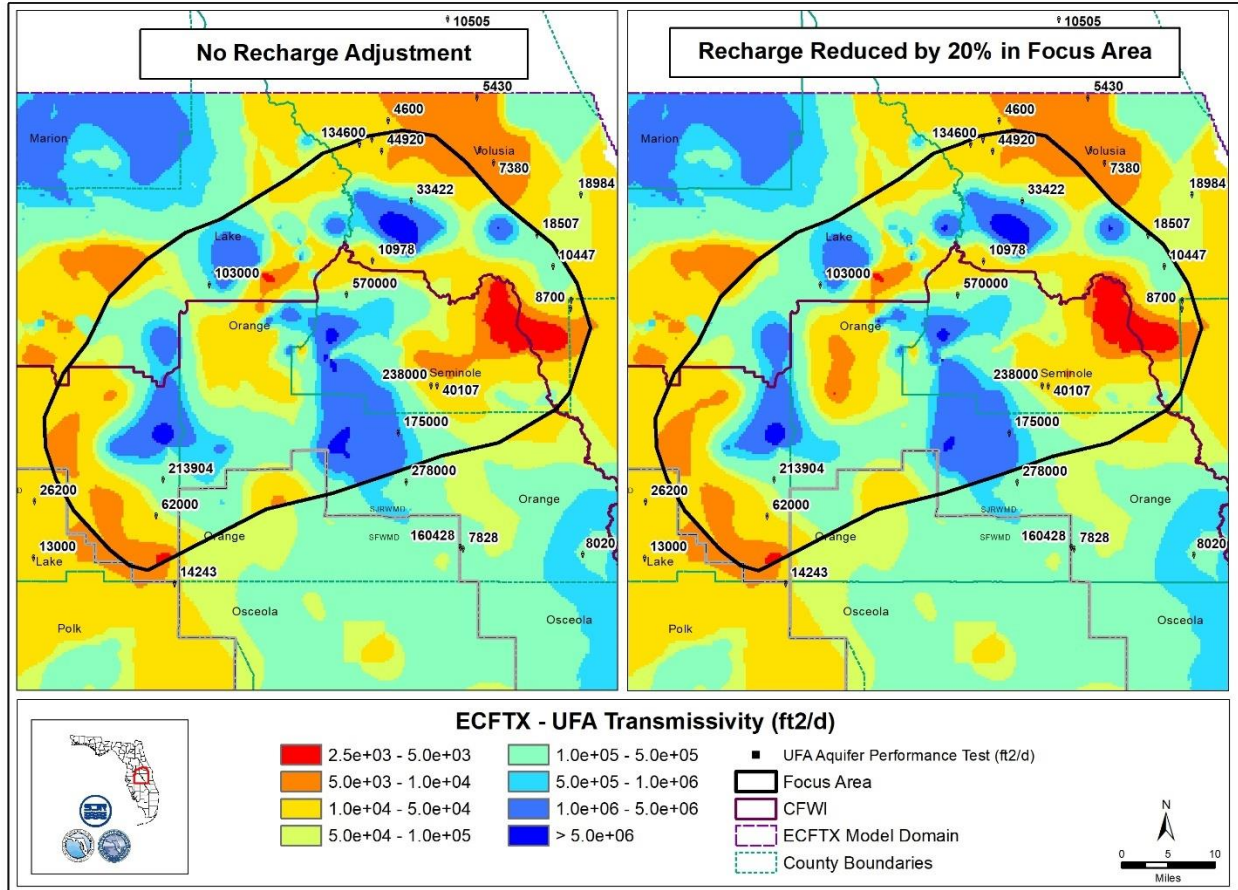


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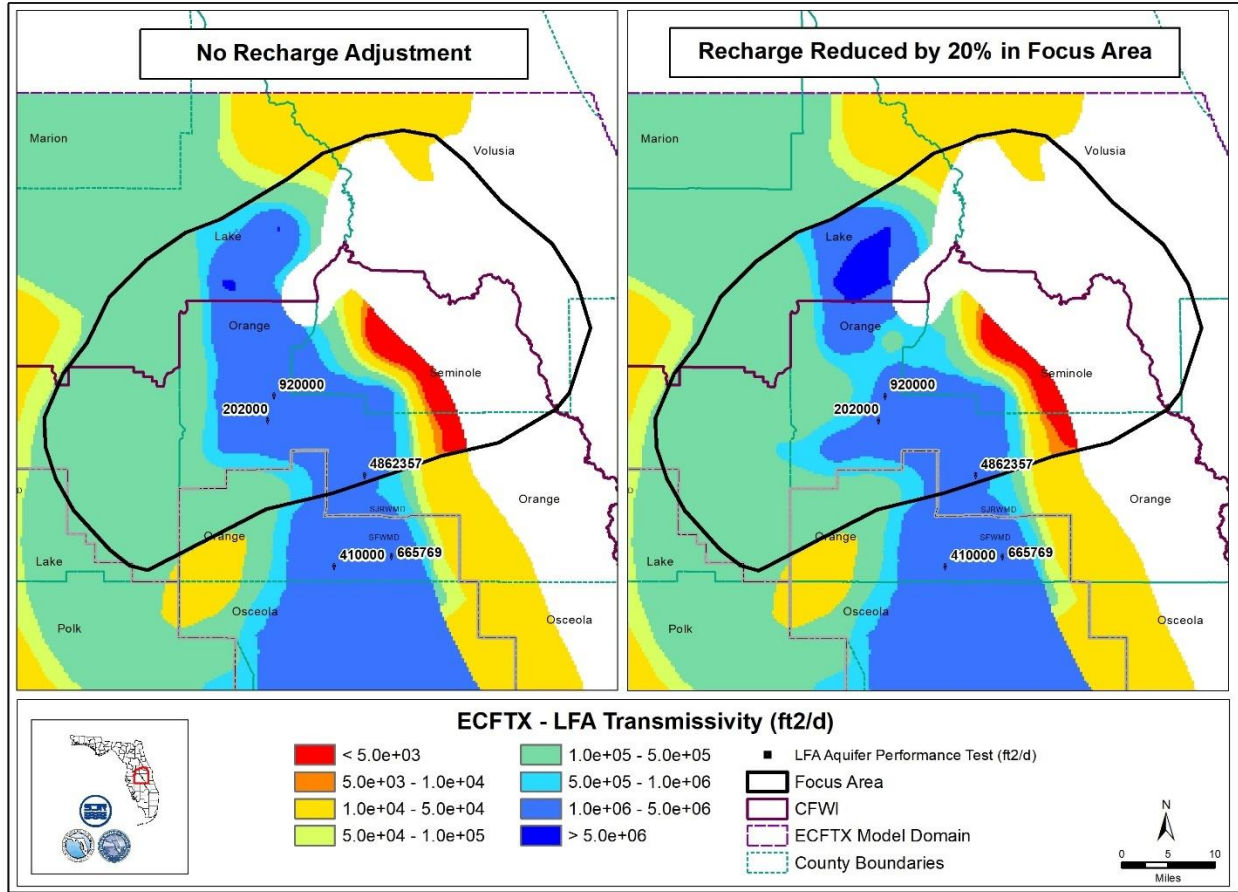


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